

Curriculum Vitae of Ralf Cuntze

comprising Career, essential Scientific Findings & Personal Pictures

Prof. Dr.-Ing. habil. Ralf Cuntze VDI, Ralf_Cuntze@t-online.de

Retired from industry, MAN-Technologie, Augsburg, and from Composites United



Engineer and hobby scientist:

application-oriented with a touch for material modelling and with the hope to be some bridge-builder between mechanical and civil engineering (construction).

Hobbies: exploring the world, nature photography, gardening, mountaineering, cyclamen breeding, etc.

This was my LIFE.



1939 born Sept 8 in Erfurt. Survived bombing at Erfurt and a machine gun fire at the war's end

1964: Dipl.-Ing. Civil Engineering CE (construction, *TU Hannover*). 1968: Dr.-Ing. in Structural Dynamics (CE). 1978: Dr.-Ing. habil. Venia Legendi in Mechanics of Lightweight Structures (TU-Munich)

1980-1983: Lecturer at Universität der Bundeswehr München: on 'Fracture Mechanics' in the construction faculty and 1990-2002 on 'Composite Lightweight Design' in aerospace faculty

1987: Full professorship 'Lightweight Construction', *not started in favor of industry*

1998: Honorary professorship at Universität der Bundeswehr München

1968-1970: FEA-programming (*DFVLR* at the airport Essen/Mühlheim)

1970-2004: MAN-Technologie (München, Augsburg). Headed the Main Department 'Struct. and Thermal Analysis'. 50 years of life with Fibers CarbonF, AramidF, GlassF, BorF, Bs(basalt)F.

*Theoretical fields of work: structural dynamics, finite element analysis, rotor dynamics, structural reliability, partial/deterministic safety concepts, material modeling and model validation, fatigue, fracture mechanics, design development 'philosophy' & design verification

*Mechanical Engineering applications at *MAN*: ARIANE 1-5 launcher family (design of different parts of the launcher stages, inclusively Booster) Cryogenic Tanks, High Pressure Vessels, Heat Exchanger in Solar Towers (GAST Almeria) and Solar Field, Wind Energy Rotors (GROWIAN Ø103 m, WKA 60, AEROMAN. Probably the first world-wide wind energy conferences organized in 1979, 1980 with Dr. Windheim), Space Antennas, Automated Transfer Vehicle (Jules Verne, supplying the space station ISS), Crew Rescue Vehicle (CMC application) for ISS, Carbon and Steel Gas-Ultra-Centrifuges for Uranium enrichment. Filament Winding theory. Material Databank etc.

*Civil Engineering applications: Supermarket statics, armoring plans, pile foundation, 5th German climbing garden (1980 designed, concreted and natural stone-bricked)

1971-2010: Co-author of *ESA/ESTEC*-Structural Materials Handbook, Co-author and first convener of the ESA-Buckling Handbook and co-author in Working Groups WGs for ESA-Standards 'Structural Analysis', 'High Pressure Vessels' (metals and composites) and 'Safety Factors'

1972-2015, *IASB*: Luftfahrt-Technisches Handbuch HSB 'Fundamentals and Methods for Aeronautical Design and Analyses'. Author and Co-author of numerous HSB sheets and about

2006-2008 co-transfer with co-translation of the HSB aerospace structural handbook into its present English version.

1980-2011: **Surveyor/Advisor** for German BMFT (MATFO, MATEC), BMBF (LuFo) and DFG

1980-2006: **VDI Guideline 2014**, co-author of Parts 1 and 2, Beuth Verlag ‘*Development of Fiber-reinforced Plastic Components*’; Part 3 ‘*Analysis*’, editor/convener/co-author

1986 and 1889: One week **FRP-lecture** on composite design in Pretoria, SA

2000-2013: World-Wide-Failure-Exercises **WWFE** on Uni-directional fiber-reinforced materials (UD) strength: WWFE-I (*2D stress states*) non-funded winner against institutes of the world, WWFE-II (*3D states*) top-ranked

2009-2021 linked to **Carbon Composites e.V.** at Augsburg, later **Composites United CU e.V.** and to TUDALIT Dresden. Since 2011 working on the light weight material Fiber-reinforced (polymer) Carbon Concrete. **Founded and headed the working groups:** (1) 2009: 'Engineering' linked to the WG Non-Destructive Testing and the WG Connection Technologies, mechanical engineering. (2) 2010: 'Composite Fatigue'. In 2010 the author held an event that was excellently attended by international speakers. (3) 2011: 'Design Dimensioning (*Auslegung, Bemessung*) and Design Verification (*Nachweis*)' mainly for carbon concrete. This working group was the foundation stone for the later specialist network **CU Construction**, aiming at “*Fiber-based lightweight construction*”. (4) 2017: 'Automated fabrication in construction including serial production' (“*3D-Print*”). (5) 2020, 2021: Forum ‘Carbon concrete for practice’ at ‘Ulm Concrete Days’

2010: Founder of the Germany-wide Working Group BeNa to base fatigue life prediction ‘embedded lamina-wise’ in order to become more general in future fatigue life design

2019: ***GLOSSAR.** “*Fachbegriffe für Kompositbauteile - technical terms for composite parts*“. Springer 2019. Edited at suggestion of carbon concrete colleagues to improve mutual understanding

2022: ***Life-Work Cuntze - a compilation from the author’s papers, presentations, published and non-published design sheets and project works in industry** (850 Pages, more design work-related)

2023: ***Design of Composites using Failure-Mode-Concept-based tools - from Failure Model Validation to Design Verification.** Mechanics of Composite Materials, Vol. 59, No. 2, May, 2023, pp. 263-282. ***Minimum Test Effort-based Derivation of Constant-Fatigue-Life curves, displayed for the brittle UD composite materials.** Mechanics of Composite Materials, Springer, Advanced Structured Materials, Vol.199, 107–146, draft. ***Cuntze R and Kappel E: Benefits, applying Tsai’s Ideas ‘Trace’, ‘Double-Double’ and ‘Omni Failure Envelope’ to Multiply UD-ply composed Laminates? * UD-Strength Failure criteria: Which one should I take? * Elaboration on Finite Fracture Mechanics.**

** Preprints, drafts are fully open for the public and downloadable from <https://www.carbon-connected.de/Group/Prof.Ralf.Cuntze> or from Research Gate*

- 1) *The presented novel scientific ideas invite for discussion.*
- 2) *The author’s research works were never funded.*
- 3) *The author asks for forgiveness in advance for inaccuracies, as he cannot get anyone to proofread.*

Of course, the text content in the scientific chapters would have deserved a revision and harmonization, but the author is becoming 86.

For me, this document collects all memories, which were relevant for me during life.

Me, Home, School and University - Time before contracted Working at University, MAN-Technologie and Composites United, Contracts ESA/ESTEC and Airbus

GUESS: “1939, *Birth of son Ralf* in the first week of World War-II was one of the few rays of hope”.

2024: The author
after the yearly
‘pilgrimage’,
26 miles.
To and from the
monasteries
Kloster Indersdorf (home)
↔
Kloster Scheyern.



STRENGTH is still his Life !

“Anyone who stops learning is old, whether at twenty or beyond eighty.

Anyone who keeps learning stays young.

The greatest thing in life is to keep your mind young”. *Henry Ford*

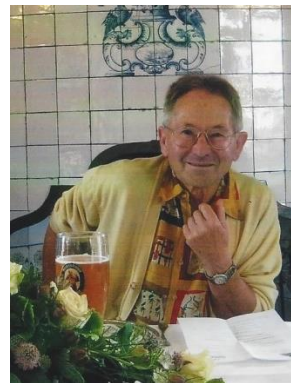
Movings

Erfurt →1945 Heimboldshausen (house of the Grandparents) →1953 Harnrode →1959 Hannover
→1968 Mühlheim-Ruhr → 1970 Dachau →1976 Markt Indersdorf.

1966 married Jutta Leonhardt (no children), 2012 wife Jutta moved to another place, 2013 meeting Maria Hasler, 2015 divorced from first wife and marriage of Maria.



Me: Becoming older and older



Family, at Home, School, Time before Working under Contract

The Ancestors:

A very old family.

A Cuntze was privy councilor to the Roman-German King Wenzel.

A captain Friedrich Cuntze took part in a crusade with Emperor Friedrich III (crowned in 1452). For this he received a coat of arms and the so-called letter nobility 'von'. (Wiener Wappensammlung, Folio 247, 37. Teil).

The two branches of the family have less than 100 members worldwide.



Mother of father.
*GrandDad passed away
before my birth
Hofgeismar*



Father



Mother , 1936



**Parents of mother
Heimboldshausen**



**After the war
Happy with her two sons
(3 girls and 2 sons)
See pictures later**

**During the war
Happy with her daughter.
The second daughter terribly died from
pulmonary tuberculosis (at that time a
person got nothing to drink!)**



Impressed statement of
Terrible World War I
Mother's father was saved from freezing to
death in the Carpathians.
He said to me
"My feeling was inner peace and warmth".

After 25 years the same happened again.



Penny farthing 1929



***On a motor bike with his
brother 1930***





The father, celebrating a festivity in his small town of Hofgeismar



Semester trip 1932 of civil engineer students

No engineering work after studies, 1931 → 1935 !

During this time, he worked again as a bricklayer for a relative and for other small companies, at maximum at the technician level



1935-1945, now continually working as a Civil Engineer at "Eisenbetonbau Erfurt".

Before 1939 it became 'normal' for getting jobs in construction industry.

Living at Erfurt during World War II.

When the bombers had planted their 'Christmas trees' in the sky, it was time to run to the bunker as quickly as possible, if there was a bunker at all ('father's' one).



Because of his eyes, Papa was not fit for war, he built industrial plants and bunkers

The first independent steps:



Comment of the relatives: "He's going to be a professor one day." This requires Strength $R +$ Effort *Eff* ! see later



Potatoe collecting at Cuntze's acre , 1941



On dad's motor bike



Traveling in Dad's company car, 1942





Traditional yearly Cuntze-Family Festivities



Grandma 90 Years birthday



Family festivity in the fifties

Burnt Memories:

1944: Dreams

- * Finally getting a ball as a gift. Realized 3 years later at Christmas
- * A dream with a net descending over me. (symbolically became true): Descending plane over the house crashing behind the house in Erfurt on a free field in front of a hospital.
- * Running to the nearest bunker when the so-called 'Christmas trees' enlightened the sky. We were allowed to take this bunker, which had not yet been released, because dad happened to be the construction responsible here)
- * Relocation of the prisoners of war towards the Americans. March of these unfortunates over the Werra bridge and then through Heimboldshausen further west.

1945:

US Army advances over the Werra border into Thuringia: Target shooting at individual buildings from a mountain. This causes us to flee to the opposite and supposed safe side.

After the German Werwolf blew up the Werra bridge directly before the arrival of a US reconnaissance tank, the civilians lined up on the hemp were covered with a machine gun fire. Aunt shot in the head, grandmother shot in the stomach, *see memorial plaque on the right.*

Concerning me: I ran 200 m further to my parents into the forest and we were not shot dead on March 30, 1945, because 'our' machine gun salvo was 20 cm too high. We then surrendered to a US officer with a white flag. The fact that there were still 4 people in the forest was probably reported by the same tank gunner who had previously shot other civilians dead.

Despite all the bitterness: *We were all happy to leave that time behind us.*

- * Refugees from Sudetenland: In 1945 and 1946 two families = 8 persons had to be quartered. After repairing the damage caused by shelling and a huge explosion of a train.
- * During war 2 urban bomb victims had been quartered in the countryside, which means into grandpa's house. March 30: The mother was shot death like my grandma (below), the daughter survived and went directly back.
- * The smell of an extinguished candle still immediately creates this memory: In the basement, candles are burning, I'm lying on a potato sack, at a distance of 5 m from the air one US tank

after the other rattles by, upstairs the grandma lies bleeding to death after her shot in the stomach

- * On April 3, 1945, a German Me Bf109 G-8 fighter attacked a German train with tankers. This was in Heimbaldshausen, now behind the US border line, and was to be destroyed according to the German command. However, the American forces that had occupied Heimbaldshausen on March 31 had already set up shop near Heimbaldshausen station. The German fighter plane had no bombs on board and attacked the train only with on-board weapons. Parts of the exploding tanks or hit by US anti-aircraft machine guns then crashed the plane with the pilot at the first houses of Röhrigshof, the engine landed in the entrance of a classroom. At that time, my father was sitting on top of the roof repairing the grenade impact of March 31st. He was lucky, he didn't fly off the roof.

- * Each pupil had to take a piece of wood with him on his footpath to school so that the elementary school in the neighboring village, which had not been destroyed, could be heated.

1946:

- * The desire to finally get a pair of long pants. This was done years later with trousers made of the fabric of a military coat. This in order to finally no longer have to wear the long stockings, which were held with elastic bands.

1948:

Changing Reichsmark (RM) into Deutschmark (DM): I received 3000 RM from my Grandpa and proudly brought him back 30 DM with which one could buy something for.

March 30, 1945
Most impressive life
experience



After the war, Heimbaldshausen was still the crossing point of a line from Russian-occupied Thuringia to Hesse and then back to Thuringia and further from the route of the inner-Hessian district railway to the district town of Bad Hersfeld. In my grandfather's general store, the train drivers from Thuringia shopped. This offered the opportunity to have the machines of my uncle's paint factory in Erfurt transported to his new workplace, namely the town Hanau, which had been completely bombed. You 'only' had to load a wagon in Erfurt, attach it and decouple it in Heimbaldshausen. The new beginning in the West was secured and successful.

Despite everything, I had a very nice childhood

Supply of Clothing & Food: Situation during war and after

Ausstellende Behörde: **Gemeinde Feimboldshausen**

Bezugschein I

Nr. 7507

für ein Paar Straßenschuhe für Frauen

An den die *Helma Bauer*
(Vor- und Zuname)

wohnhaft in *Feimboldshausen* (Ort) Nr. *95* (Straße)

von Beruf , geboren am *1922* in *Feimboldshausen* (Geburtsort)

darf innerhalb von drei Monaten vom Tage der Ausfertigung dieses Bezugscheins an ein Paar Straßenschuhe abgegeben werden.

Die Ware darf nur unter gleichzeitiger Aushändigung dieses auf der Rückseite mit einer Empfangsbescheinigung des Bezugsberechtigten versehenen Scheins ausgegeben werden. Wird die Ware durch einen Vertreter oder einen Beauftragten bezogen, so hat dieser die Empfangsbescheinigung zu vollziehen. Andere als die obenbezeichnete Ware darf nicht abgegeben werden. Der Bezugschein ist nicht übertragbar. Jeder Mißbrauch ist strafbar.

Für die Entwertung und Registrierung des Bezugscheins nach der Warenausgabe gelten besondere Vorschriften.

(Dienststempel) **Gemeinde Feimboldshausen**

Ausgabe März 1944

Druck: A. Bernecker, Meisingen

Im Auftrage: *[Signature]*

Cloth Card 1944

(within 3 months of the issuance of this voucher, a pair of street shoes is available)

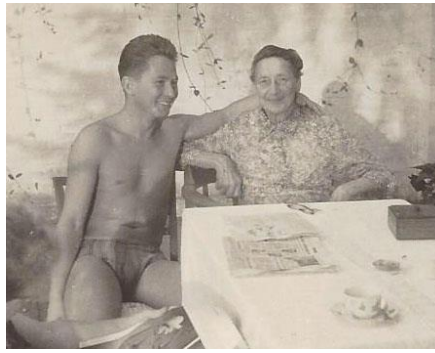
| 5 g Fett | | | | | E 90 10 | E 90 H | E 90 G | Brot D | Brot C | Brot B | Brot A |
|---------------------|--|--|--|--|---|---------------|---------------|----------------|----------------|----------------|----------------|
| 90 | | | | | EA | LEA Würt. | LEA Würt. | US-Zone | US-Zone | US-Zone | US-Zone |
| 5 g Fett | | | | | <p>U. S. Besatzungszone LEA WÜRTTEMBERG EA</p> <p>Lebensmittelkarte für Erwachsene über 18 Jahre</p> <p>E 90 Gültig vom 24. 6. bis 21. 7. 1946</p> <p>Für Sie von Brot für die Welt</p> <p>Anstelle von 500 g Brot können 275 g Mehl bezogen werden bei Verlust der Karte kein Ersatz</p> | | | | | | |
| 5 g Fett | | | | | | | | | | | |
| 5 g Fett | | | | | | | | | | | |
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| 5 g Fett | | | | | | | | | | | |
| 5 g Fett | | | | | | | | | | | |
| 50% Brot | | | | | 1000 g Brot | 500 g Brot | 1000 g Brot | 500 g Brot | Brot | | |
| 50% Brot | | | | | US-Zone | US-Zone | US-Zone | US-Zone | US-Zone | | |
| 50% Brot | | | | | E IV 90 | E III 90 | E II 90 | E I 90 | E I 90 | | |
| 50% Brot | | | | | 100 g Fleisch | 100 g Fleisch | 100 g Fleisch | 100 g Fleisch | Fleisch | | |
| 50% Brot | | | | | US-Zone | US-Zone | US-Zone | US-Zone | US-Zone | | |
| 50% Brot | | | | | E IV B 90 | E III B 90 | E II B 90 | E I B 90 | E I B 90 | | |
| 50% Brot | | | | | 50 g Fleisch | 50 g Fleisch | 50 g Fleisch | 50 g Fleisch | Fleisch | | |
| 50% Brot | | | | | US-Zone | US-Zone | US-Zone | US-Zone | US-Zone | | |
| 50% Brot | | | | | E IVC 90 | E IIC 90 | E IIC 90 | E IC 90 | E IC 90 | | |
| 50% Brot | | | | | 62,5 g Fett | 62,5 g Fett | 62,5 g Fett | 62,5 g Fett | FETT | | |
| 50% Brot | | | | | US-Zone | US-Zone | US-Zone | US-Zone | US-Zone | | |
| 50% Brot | | | | | E IV 90 | E III 90 | E II 90 | E I 90 | E I 90 | | |
| 50% Brot | | | | | Fett | Fett | Fett | Fett | ZUCKER | | |
| 50% Brot | | | | | US-Zone | US-Zone | US-Zone | US-Zone | US-Zone | | |
| 50% Brot | | | | | E 90 | E 90 | E 90 | E 90 | E 90 | | |
| 50 g Nahrungsmittel | | | | | E 90 9 | E 90 F | E 90 C | Nahrungsmittel | Nahrungsmittel | Nahrungsmittel | Nahrungsmittel |
| 50 g Nahrungsmittel | | | | | EA | LEA Würt. | LEA Würt. | US-Zone | US-Zone | US-Zone | US-Zone |
| 50 g Nahrungsmittel | | | | | E 90 8 | E 90 E | E 90 B | E 90 90 | E 90 90 | E 90 90 | E 90 90 |
| 50 g Nahrungsmittel | | | | | EA | LEA Würt. | LEA Würt. | US-Zone | US-Zone | US-Zone | US-Zone |
| 50 g Nahrungsmittel | | | | | E 90 7 | E 90 D | E 90 A | Quark | Käse | Käse | Käse |
| 50 g Nahrungsmittel | | | | | EA | LEA Würt. | LEA Würt. | US-Zone | US-Zone | US-Zone | US-Zone |
| 50 g Nahrungsmittel | | | | | E 90 7 | E 90 D | E 90 A | 100 g Quark | 62,5 g Käse | 125 g Käse | 125 g Käse |
| 50 g Nahrungsmittel | | | | | EA | LEA Würt. | LEA Würt. | US-Zone | US-Zone | US-Zone | US-Zone |
| 50 g Nahrungsmittel | | | | | E 90 7 | E 90 D | E 90 A | E 90 | E 90 | E 2 90 | E 1 90 |
| 50 g Nahrungsmittel | | | | | EA | LEA Würt. | LEA Würt. | US-Zone | US-Zone | US-Zone | US-Zone |
| 50 g Nahrungsmittel | | | | | E 90 7 | E 90 D | E 90 A | E 90 | E 90 | E 2 90 | E 1 90 |
| 50 g Nahrungsmittel | | | | | EA | LEA Würt. | LEA Würt. | US-Zone | US-Zone | US-Zone | US-Zone |

Food Ration Card

after the 2nd World War 1946 (as in the 2nd war)



Dad and son



Grandma and grandson



Son and mum

Dad's second run for a job



In 1944 after finalizing a construction, built by the company 'Eisenbetonbau Erfurt' he was working for



The march of those involved in the construction following the Nazi leaders

Großhessisches Staatsministerium
Der Minister für Wiederaufbau
und politische Befreiung.
Der öffentliche Kläger bei der Sprudkammer.

Hersfeld, d. 11.3.47
(Datum)

Aktenzeichen: Hersfeld He/21738

Auf Grund der Angaben in Ihrem Meldebogen sind Sie von dem Gesetz zur Befreiung von Nationalsozialismus und Militarismus vom 5. 3. 1946 nicht betroffen.

Der öffentliche Kläger:

An Herrn / Frau / Frl.
Werner Cuntze
in Heinboldshausen
Dorfstrasse 95

Formblatt 11. - Mitteilung an Nicht-Betroffene

De-Nazification receipt, 1947

With the receipt of above certificate, one became again a recognized citizen with all rights and obligations.

Very decisive for a new job!

Dad was now able to find work as a civil engineer in the potash mine, 1949



My uncle Rudi was a ‘Soft Nazi’. In the Russian-occupied zone, it was therefore possible for him to work!! Of course, he also needed things from home. We brought these things (me too, because at child's it was not so ‘easy to shoot’) with the handcart to the border 4 km away. (How could one manage this without a smartphone?). When my uncle later wanted to go home, he was sentenced and then sat in the prison of Hofgeismar. There, we children (when I visited grandma) brought food to the prison with the usual cooking utensils of the soldiers at that time, a so-called approximately ‘bretzel-shaped Henkelmann’, which I always had with me for receiving the school meal). I experienced something like this again in Ladakh in 1981, however there the relatives lived 10 km away from the prison and not just a few hundred meters. My uncle was guarded by a warden with a rifle, but this was no problem for the warden to go with the prisoner at his exit time to a newly opened pub.

Dad the biker: The ‘new’ life approaches the former life



1938



1950



1956 This was the great time to learn shorthand

School at Heringen-Werra



(left) **Class 1957.** Entrance examination 1950 (64 pupils), Abitur 1959 (just 9 pupils left at exams) (right) **about 2012.** Our strict, fantastic class teacher Hans-Heinrich Bähr

Quittung über DM in Ziffern 76 Pf

Deutsche Mark in Worten

Sechshundsechzig

von Herr Dr. Feilke, Honigs W

für 10 90 - Freundes der Biologie
in Idumea

erhalten zu haben bescheinigt:

Ort Honigs den 25. 10. 57

Buchungsvermerke

Firmenstempel - Unterschrift des Empfängers



Books had to be privately paid

Class trip

I remember that in 1946 there was excitement again: Suddenly soldiers of the Red Army appeared again with, among other things, Panje horses, they had probably mistakenly oriented themselves on old maps. This would have meant that the place where we wanted to build our new house would not have become western territory. The US Army tanks, however, quickly put an end to this exciting situation.





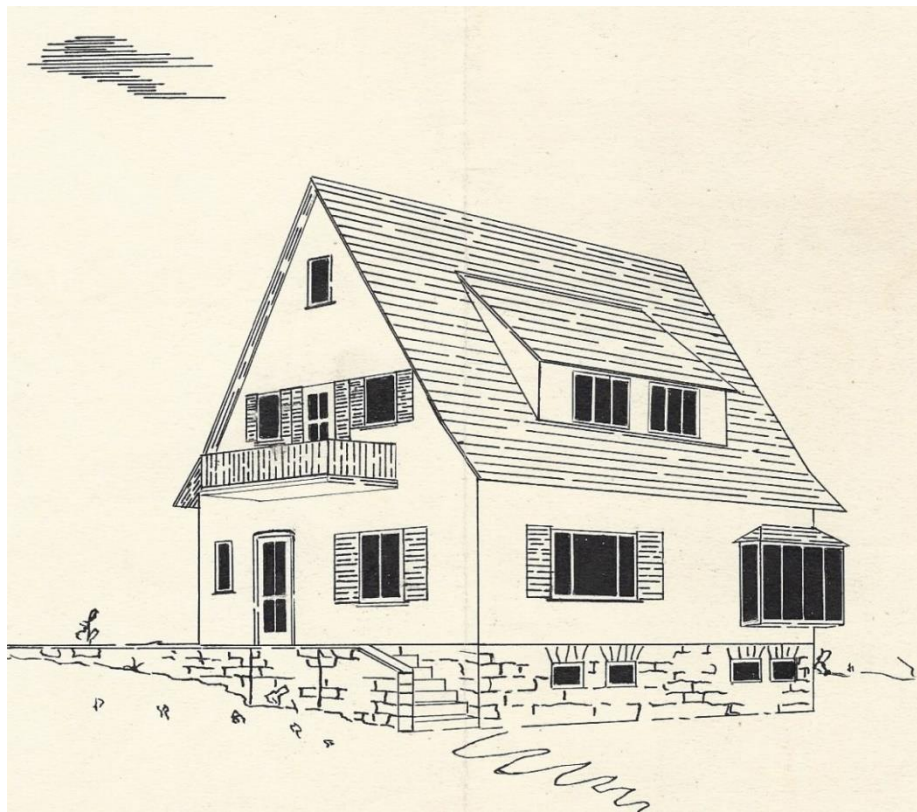


Our strong, however beloved teacher H.H. Bähr who finally married a class mate



Building a new home

My dad's drawing





Wooden log cabin



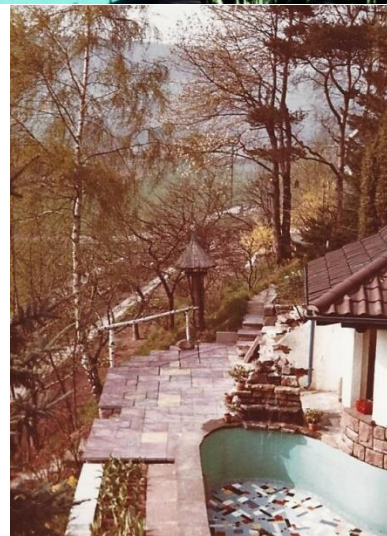
Living at home. All buildings self-made. Father: civil engineer

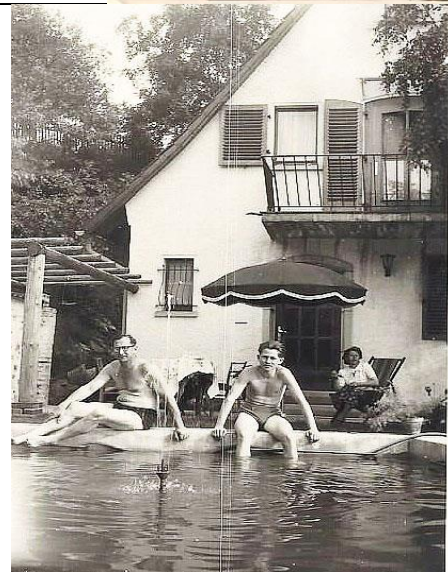
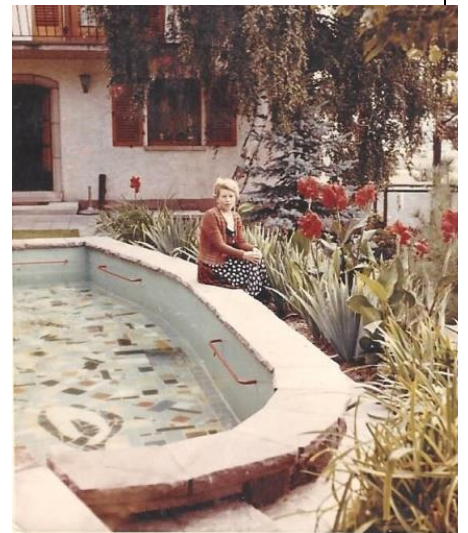


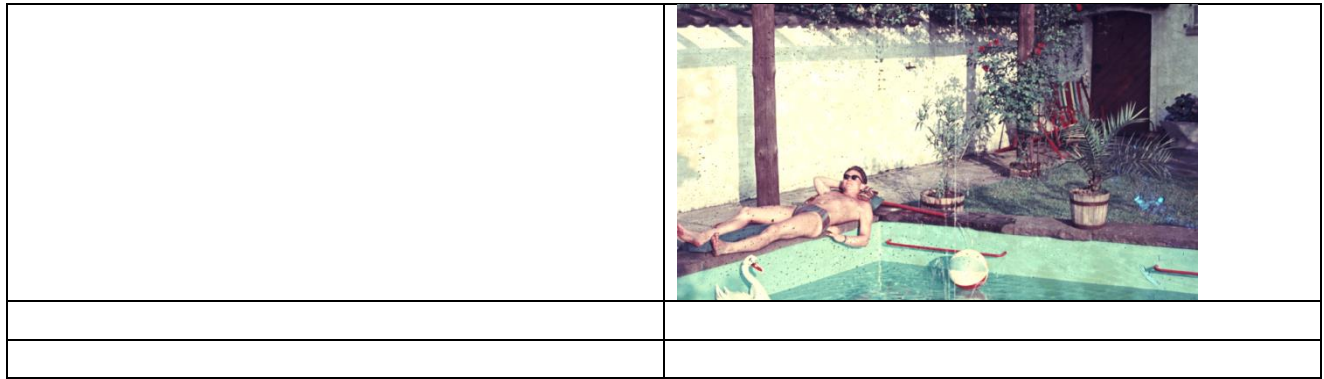
A source for own use



The 100 m long wooden fence asked for Ralf's care







Every year, the slaughter of a pig served to obtain a food supply



**With such good care,
one had a good laugh**



In the Grandparents general store at Heimbaldshausen, where the author lived and collected experience as a young merchant. About 1960.

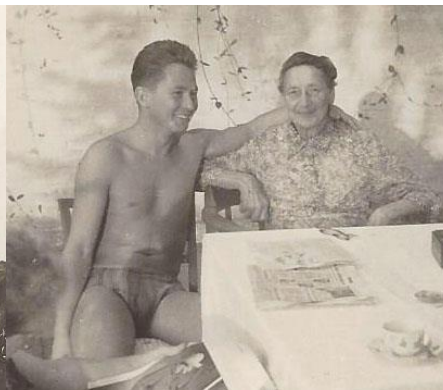
The figure shows the later owner, my relatives Heinrich Koch and his wife Friedel, my 'second' mother.



Working & Relaxing on the building site



Dad and son



Grandma and grandson



Son and mum







My Living at the Inner-German Border



Grown up directly at the Hessian border to, where the spoil heaps (200m above ground) from potash extraction are no longer allowed to get higher than the surrounding mountains, 2024.

In this potash mine my father could finally again find work. This 4 years after the WW-II end!

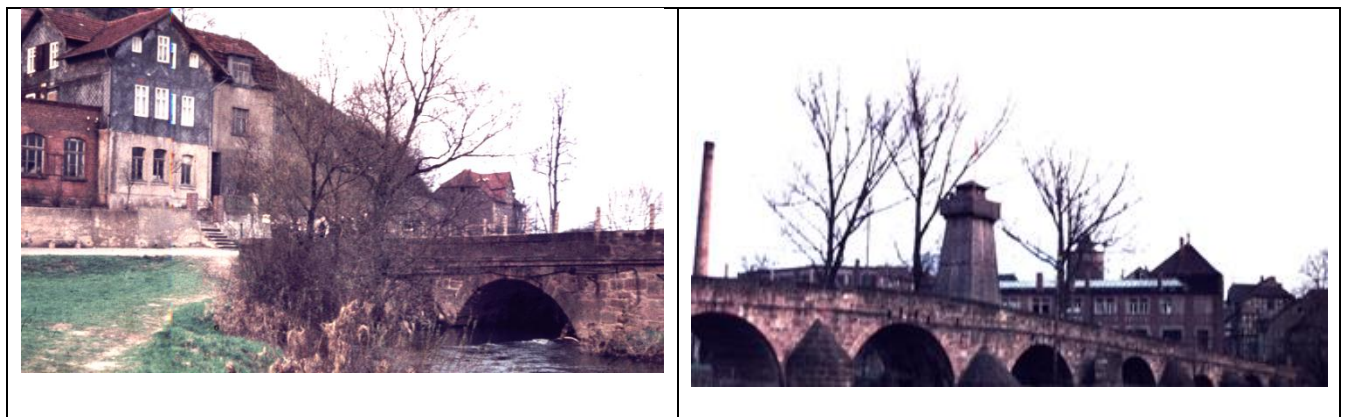




1967

Exclusion zone at the Philippsthal-Vacha zone border:

An exclusion zone was established on 26 and 27 May 1952. Its width was 5 km. This was the first tighter lockdown. You were no longer allowed to enter this restricted area of the GDR (DDR) unless you had a pass. The exclusion zone included a protective strip (500 m wide) and a control strip (10 m wide) directly at the border. On 4 June 1962, GDR work detachments began to build a three-metre-high wall from prefabricated elements on the banks of the Werra in Vacha, opposite of Philippsthal, where contact between acquaintances and relatives from Philippsthal and Vacha had previously been possible. (*I still used this to visit relatives. In 1950 I was also able to have an operation on my appendix in a clinic in Vacha*). This meant that the former Hoßfeld printing plant was divided into the FRG BDR) and the GDR. With effect from 1 January 1976, however, this well-known "house on the border" is no longer divided. The Hoßfeld family regains the right to unrestricted use of the walled-off eastern part of the printing house, which had previously been on the territory of the GDR, and the inner-German border is laid around the outside of the house in an imaginary line. This meant that Philippsthal was deprived of an attraction popular with border visitors, all acquaintances living further away were brought there.







Thanks to the eightieth anniversary celebration of our common uncle, we two nephews were able to see each other again in 1988 after 1957.

We could not imagine at that time that reunification would come a year later.

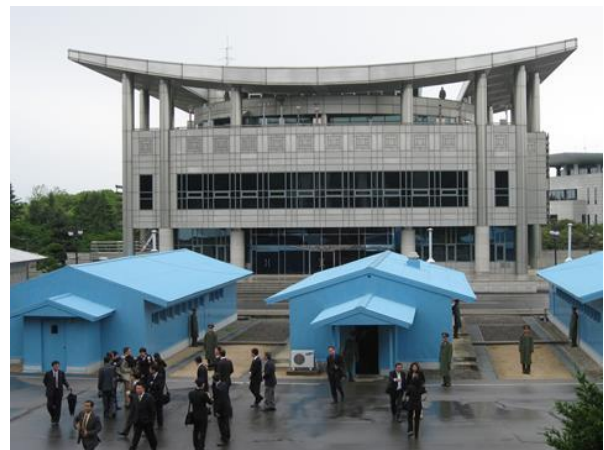
We were very thankful!



Anyone who grew up directly on a border and had to suffer the increasing isolation between relatives and friends can certainly understand that I was very interested in the Korean border which might show differences (similar unfortunately in 2008) regarding this also a nation-separating border.



Similar exclusion zone



Looking at the Southcorean Building from the neutral demarcation place

Amber Room (Bernsteinzimmer) 'Story'??

Transport of the real Amber Room from the Merkers Salt Mine by the US Armed Forces !?

In 1716, King Frederick William I, who was not very interested in art, exchanged the Amber Room with Russian Tsar Peter I for tall soldiers. During World War II, the Amber Room was captured by

the Wehrmacht in Saint Petersburg in 1941 and subsequently installed in the Königsberg Castle Amber Room. During the advance of the Red Army, it was removed in 1944 and has been lost ever since. However, I found a handwritten message about the transport from the hidden safe place in the potash mine Merkers, Thuringia, per train etc in direction USA

14 April 1945
 Auf dem 1. April 1945
 von Herrmann
 Frankfurter Hofstr. 20.
 Leipzig mit dem
 Bahnfahrplan n. 3. 2. 1. 1.
 und dem Fahrplan n. 2. 1. 1.
 Sonntag 1. April 1945 in
 Frankfurt für Leipzig
 der 1. April 1945 3. 1. 1.

© The Curtis Publishing Company HALLMARK CARDS, INC.

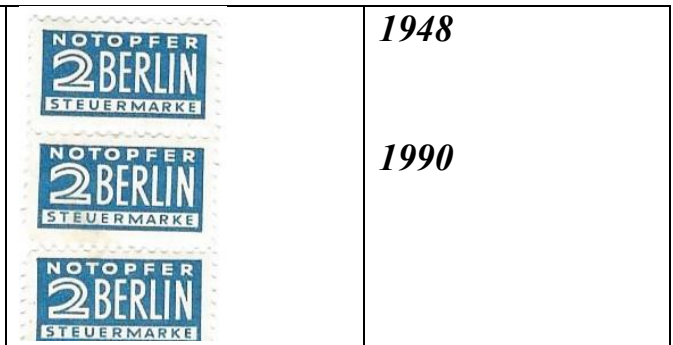
im folgenden Brief-
 garten sprachlich m.
 über den von dem
 da nicht ist.

Frankfurt am Main
 Leipzig
 Winter 1945
 Winter 1945
 Sonntag 1. April 1945
 Leipzig

1. April 1945 17.13

Photos of the confirmands 1954, 2004, 2024







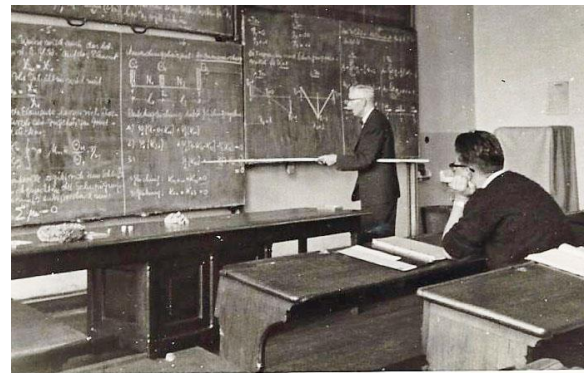
Technical University of Hannover and Institute of Civil Mechanics

Costs of study, 5 years: 10000 DM, half of which was earned by myself.

Student flat: Unheated, the water froze in the washing bowl. (Central heating already available at home).



Technische Hochschule Hannover



Supporting Prof. Eugen Doeinck by preparing his lecture tables. He had to stand in for his successor Hans Kauderer, who died suddenly, for a while.



Traditional get-together with the new 'boss' at the civil mechanics chair, Theodor Lehmann



The traditional summer trip of the chair members: Schäfer, Hartung, Lohmann, de Boer, Schroeder, etc



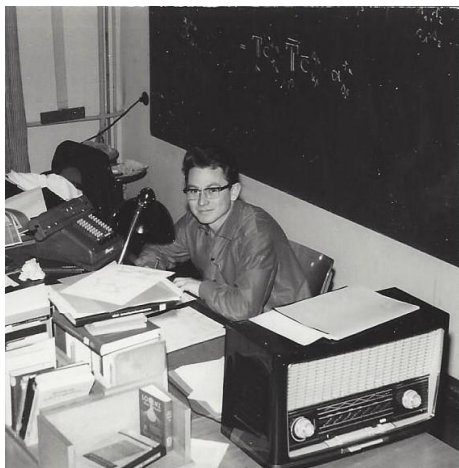
(1962) Christmas table round with emeritus Doeinck



Final measurement exercise



The 'inner Circle' of the 120 students of civil engineering at TU Hannover



Tensor analysis. On the way to becoming an engineer. This is how we worked at the mechanics chair (1962), and this was also the case on Sundays: In the unheated cold student apartment the water in the washbowl froze.

Frederick-Heinz Schroeder:
February 11, 1968
"What is this on your head ??"



The job title engineer supposedly appears for the first time in 'Le Roman de Rou' by Robert Wace (about 1150, citing H. Lange, Technik in Bayern 5/2004).

Following Google the term engineering is derived from the Latin ingenium, meaning "cleverness" and ingeniare, meaning "to devise". Engineering has existed since ancient times, when humans devised inventions such as the wedge, lever, wheel and pulley. The term engineering is derived from the word engineer, which itself dates back to 1390 when

an engine'er (one who operates an engine) referred to "a constructor of military engines."

Later, as the design of civilian structures, such as bridges and buildings, the term *civil engineering* emerged to distinguish between military engineers and those specialized in the construction of civil projects

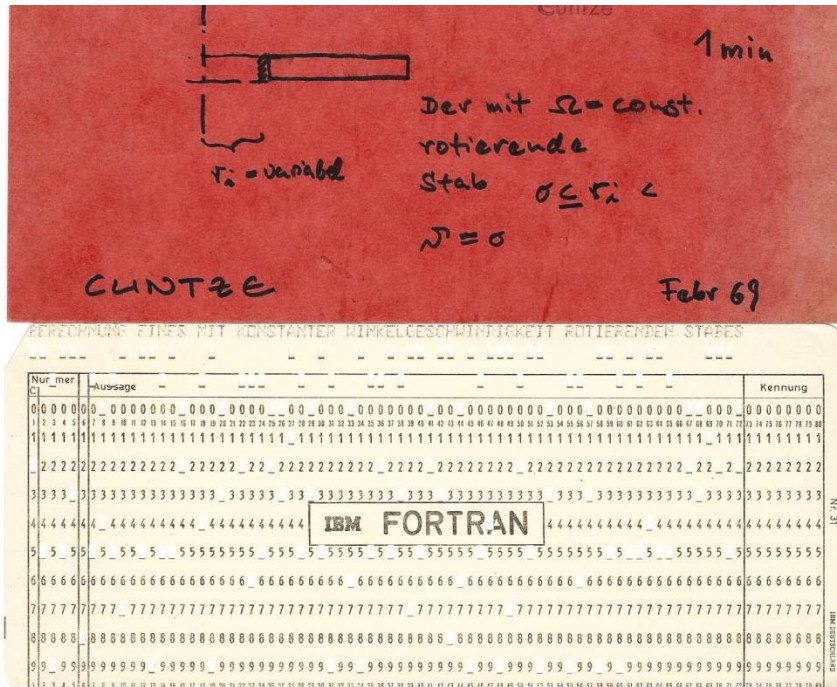


Meeting of former colleagues

Numerical Analysis and Finite Element Analysis

Tools for practicing engineering in 1960-1968

- ✓ Diploma work:
Application of an Arithmetic Stick (slide rule), the short one and for more precise calculations the 25 cm long one
- ✓ Dissertation:
I could not finish my dissertation in 1967 because o and 0 where not clearly marked in the coding which cost two months filled with despair
- ✓ FEA-Practice:
Later in the sixties, Finite Element Analysis came up, however, there was no commercial tool available. The author had to beg for the building blocks of his intended program (in experience for the program below at the DLR) at his old university in Hannover. To punch cards was a must. Waiting for the result: One week.

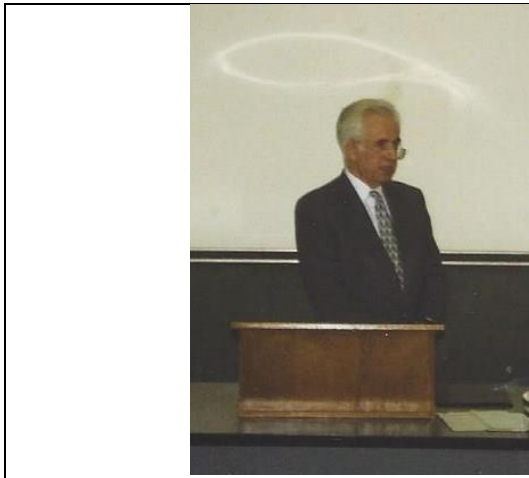


About the calculation accuracy:

„When calculating, it is always important to consider the accuracy that you either have to achieve or can achieve. It is quite inadmissible to calculate with great accuracy if the nature of the task either does not allow it or does not require it. If an approximate value contains superfluous decimal numbers, it must be rounded. In some cases, more realistic but more complicated models can be replaced by simpler ones that give a result with an acceptable error.“

Written by Bronstein-Semendjajev in 1960 in their famous booklet

Habilitation (1978) and Honorary Professorship (1998)



Working in Civil Engineer Applications

My work in the Aerospace domain is compiled in [CUN 22].

Application-oriented engineer with a scientific touch for material modelling and with the hope to be some bridge-builder between mechanical and civil engineering (construction).

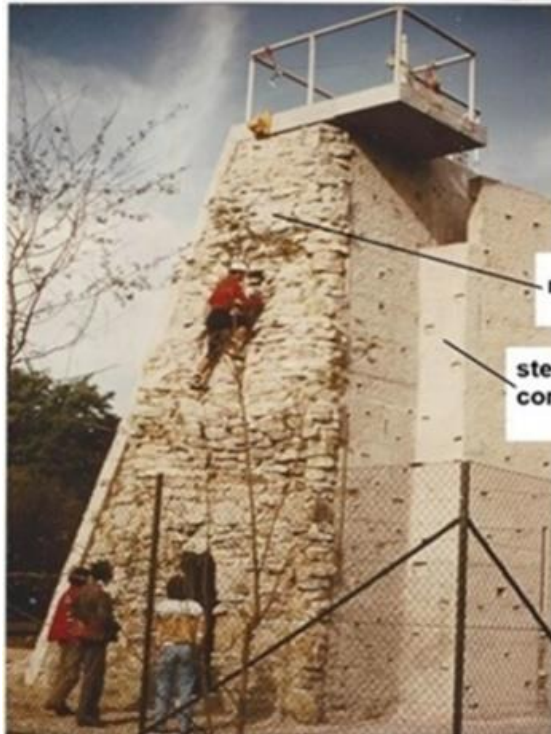
Working as a bricklayer:

(right, example) Wall of a so-called facing masonry. With piecework bricklaying, you were allowed to go home faster. Weekly working hours were 45 hours, overtime was gladly done because of the money, and it was also concreted until Saturday evening. There was no thought of 'work-life balance'. At the weekend, you received the so-called pay packet from the foreman. In this were billing, notes and remaining coins.



Structural engineering, armoring plans, pile foundation



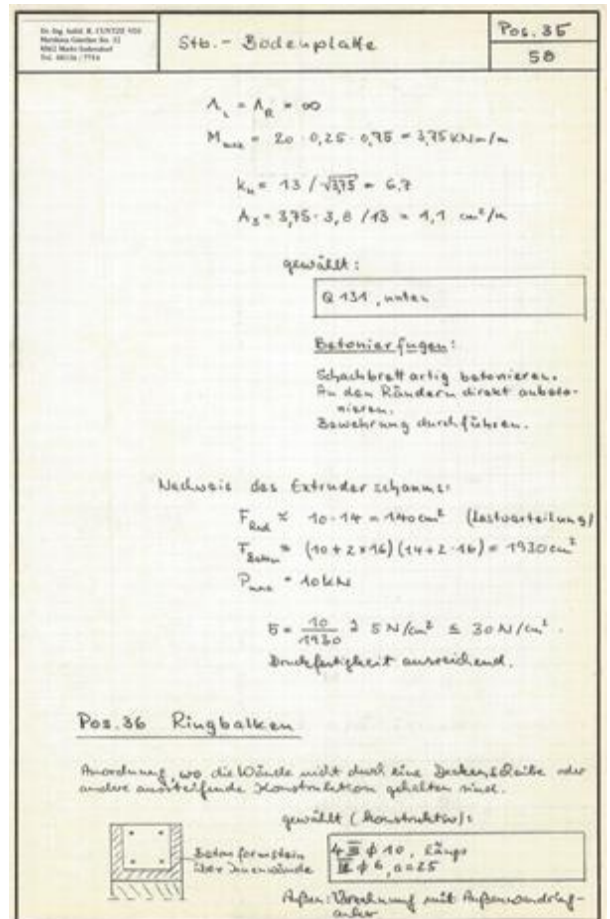
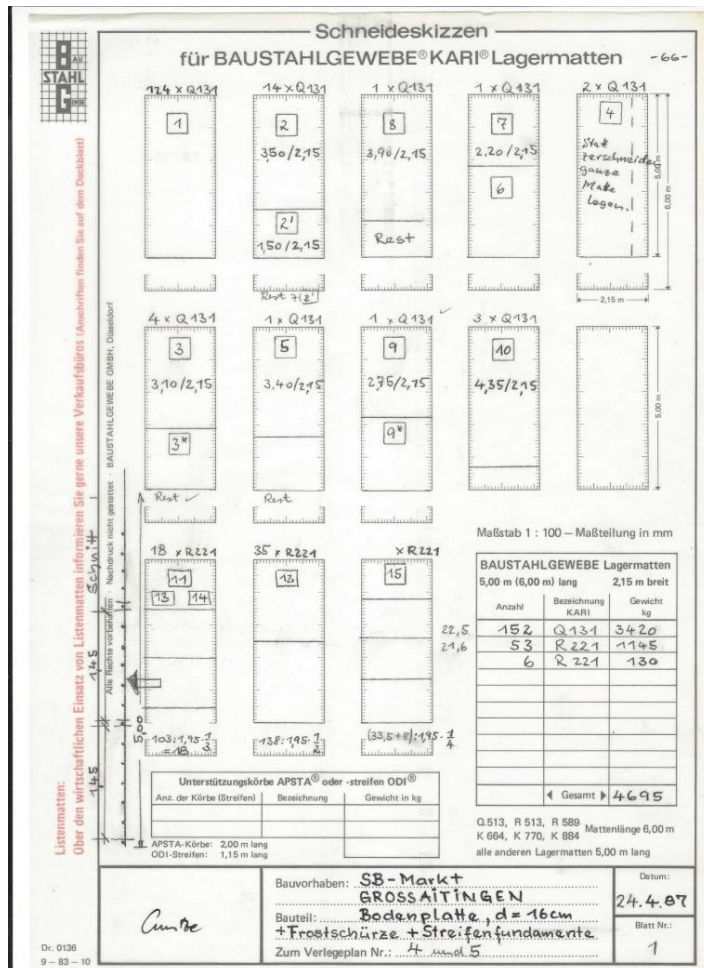


5th German
'climbing garden'

rock material

steel reinforced concrete

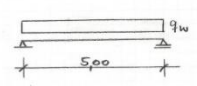
Designed,
concrete casted,
one side masoned with
rock stones from the alps
by the author



| | | |
|--|------------------|--------------|
| Dr.-Ing. habil. R. CUNTZE VDI Mühlen-Garten-Str. 27 3062 Markt Indersdorf Tel. 05136 / 7754 | GEB - Außenwände | Pos 37 59 |
|--|------------------|--------------|

Pos. 37 GEB - Außenwände

System



Belastung
Druck oder Sog $q_w = 0,6 \cdot 0,5 = 0,40 \text{ kN/m}^2$

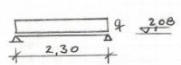
Schnittkräfte
 $A = B = Q = 0,40 \cdot 500 / 2 = 1,0 \text{ kN/m}$
 $M = 0,40 \cdot 5,00^2 / 8 = 1,25 \text{ kNm/m}$

Bemessung
 gewählt: lt. Zulassungsbereich
 Hebel Gasbeton Wandplatte,
 Liegend, $d = 15 \text{ cm}$.
 Befestigung an Stb.-Stütze siehe
 folgende Seiten.
 Stütze über Fenster und Türöffnungen
 konstruktiv nach Herstellerangaben.

Auschluss GEB - Mauerwerk
 mit S-Haken?

Pos. 38 Türschwelle, B25 (Achsen 4-B; 5/6-C/D)
 $b/d = 24/24$

System



Belastung
 $\approx 1,1 \text{ m}^2 \text{ m}^2 \text{ verjährt}$
 Mauerwerk
 Eigengewicht } $\approx 27 \text{ kN} \approx 1,1 \cdot 0,24 \text{ m} = 7,1 \text{ kN/m}$

Schnittkräfte
 $A = B = Q = 7,1 \cdot 2,3 / 2 = 8,2 \text{ kN}$

| | | |
|--|-----------------------------|--------------|
| Dr.-Ing. habil. R. CUNTZE VDI Mühlen-Garten-Str. 27 3062 Markt Indersdorf Tel. 05136 / 7754 | Stb-Stütze u. Köcherfundam. | Pos 32 57 |
|--|-----------------------------|--------------|

Pos. 32 Stb-Stütze und Köcherfundam.

Analog (29).

Pos 33 Frostschwelle, $b = 30 \text{ cm}$, B15

gewählt (konstruktiv):

R221 Mattenkorb
 III $\phi 12$ Länge
 In der Aufkantung:
 III $\phi 8$, $a = 20$ Steckbügel
 III $\phi 12$, Länge

Pos. 34 Straifenfundamente, $b = 50 \text{ cm}$, B15

gewählt (konstruktiv):

R221 Mattenkorb
 III $\phi 12$ Länge

Pos. 35 Stb-Bodenplatte, $d = 16 \text{ cm}$, B15

Die Platte soll von 20 kN -Gepäckstapfen befahren werden.
 Unter der Bodenplatte ist eine Dämmschicht aus $6 \times$
 Trittschall dämmung angeordnet (zul. $p = 300 \text{ kN/m}^2$).

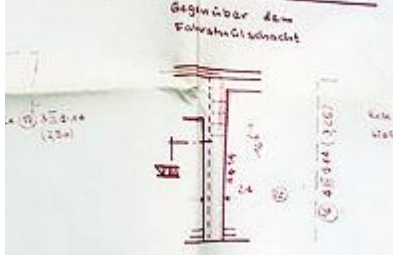
Nach WÜLFER:

$E_s \approx 6000 \frac{\text{MPa}}{\text{mm}^2}$; $\chi \approx 0,65$
 $C = E_s / \chi = 6000 / 0,65 = 9230 \text{ MPa/m}^2$
 $L = \sqrt[4]{\frac{4 \cdot 260000 \cdot 0,15^3}{12 \cdot 9230 \cdot 100}} = 0,75 \text{ m}$

Dimensioning

Pos. S 208, $b/d = 24/30$

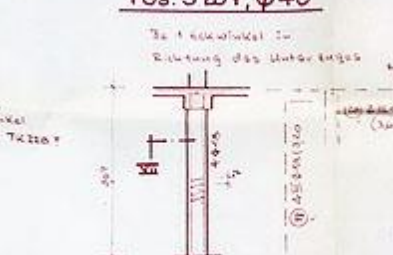
Gegenüber dem Fahrstuhlanschub



Schnitt VII-VII

Pos. S 207, $\phi 40$

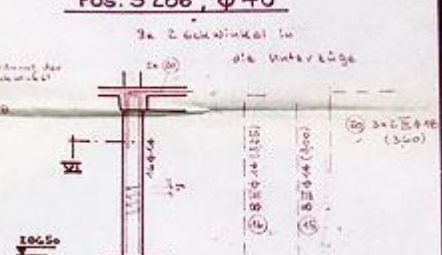
3x 1 schräg in Richtung des Unterbaus



Schnitt VII-VII

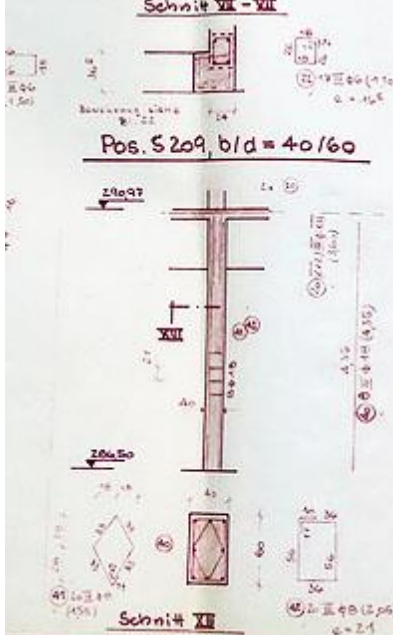
Pos. S 206, $\phi 40$

3x 2 schräg in die Unterzüge



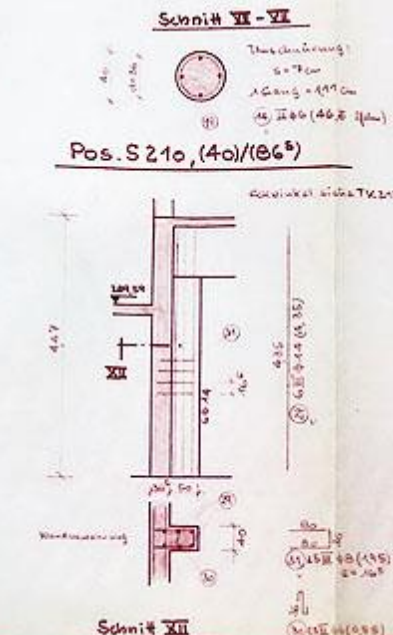
Schnitt VII-VII

Pos. S 209, $b/d = 40/60$




Schnitt XII

Pos. S 210, $(40)/(106^B)$



Schnitt XII



| | | | |
|---------------------------------|---|-------------------------------------|----------------|
| Tag | Name | Optim.-Signatur | |
| gezeichnet | 15.10.67 CM. | RODOLPHAS-SCHAPER-BERGMANN | |
| geprüft | | Beratende Ing. VDI für Bauwesen | |
| | | ZIMMERMANN - Knapen 32 - Tel. 72828 | |
| Bauherr: | Niedersächsische Heimstätte GmbH Hannover | Blatt Nummer | 24 |
| Bauwerk: | Göttingen-Gelsmar Bl. 8943 | Maßstab: | |
| Bauführer: | Kontrollplan: K.T.K. HAUS II W 201, S 204, 205, 206, 207, 208, 209, 210, 211, 212 TK 1111 + 1112 + 1113 | gezeichnet: | |
| Beton | Kornbreit. | Zementart | Zementgebalt |
| B 225 | v. | DIN | 1045 |
| | | | I ₂ |
| Lichtpausen A4 | | | |
| Architekt | Bauleitung | Bauführer | Prüfungsbauer |
| | | | |
| Alle Maße am Bau prüfen! | | | |

DGLR 1968-1970, German Society for Aeronautics & Astronautics

From DFL to DVL to DGLR

Christmas Festivity with Wolf Elber
(Father of the crack closure concept in
Fracture Mechanics, later director of a
NASA institute investigating the
Columbia Booster tragedy reasons).

Invitation of Wolf regarding our Ariane
Booster connection:
*Our tang-clevis connection closes under
a hazard condition in contrast to the
Columbia design*



FE-programming: Development of a Finite Element calculation program for the calculation of a rotationally symmetrical (more was not possible) centrifugal compressor wheel. However, at that time, the needed program modules for this had to be requested from friends at his former University at Hanover.

Später: Teilnahme an der Verabschiedung der **großen alten Dame der Stabilitätstheorie**



MAN (New) Technology

Founded in 1969 as an independent research and development division of MAN AG.
Objectives at that time: Development of new products and techniques, engage intensively in basic research, applying novel materials, open up new markets for the young division.



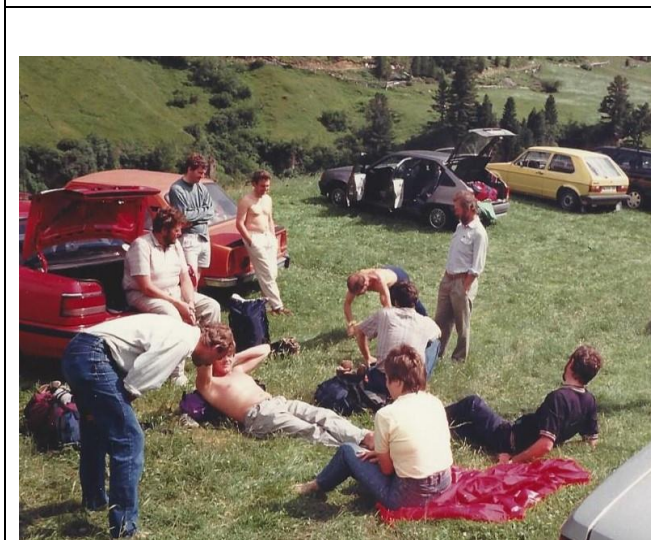
Two pictures of the team over the time



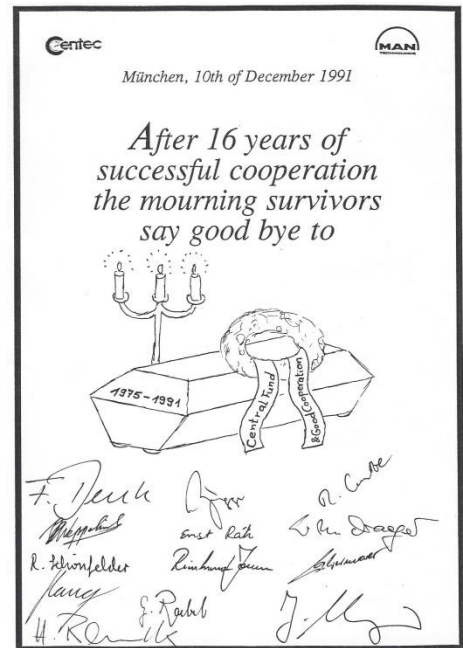
In deep discussion with Dr. Gernot Zippe about (*I guess*) UF6-uranium enrichment centrifuge questions

Finite Element Analysis Meeting at MAN Neue Technologie with heads Dr. Laußermair and H. Rauck





The production of gas ultracentrifuges for UF6 enrichment was completed at the end of the 80s. One reason was that the failure rate was fantastically small

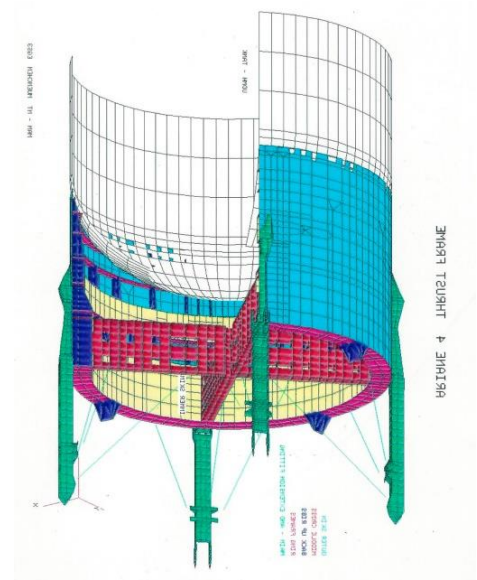


Continuing education at MAN

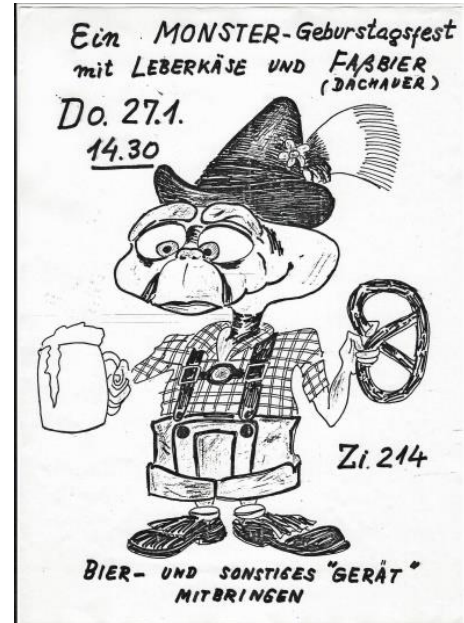




Keen soccer and mountain hiking team



Celebrating festivities together was an important fact





*Tragen Sie ein Lächeln
in Ihrem Gesicht, dann
kann bei unserer runden
Geburtstagsfeier gar nichts
mehr schief gehen.*

Wir lächeln trotz der 180 Jahre:

Reimund Schönfelder
Stan. Smith
Ralf Cuntze

3 mal 60 Jahre und kein bißchen weise !

Anläßlich unseres ersten Schrittes in das
Greisenalter laden wir Sie hiermit als
Zeuge dieses umwerfenden Ereignisses
zu einem **Imbiß** mit gesundheitlich
gerade noch vertretbaren **Betäubungs-**
getränken ein.

Ansprechpartner:

Ralf Cuntze
Stanley Smith
Reimund Schönfelder

Telefonische Rückantwort bitte bis zum
2. 9. 99, Fr. Schwarz, RA
Tel.: **K 1867** bzw. **08131/891867**

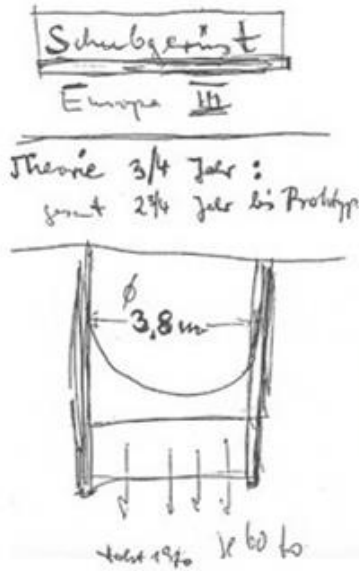
Datum: 16. 09. 1999
Uhrzeit: 15³⁰

Ort: **Karlsfeld, Liebigstr. 5a**
in: **Saal 3 und 4**

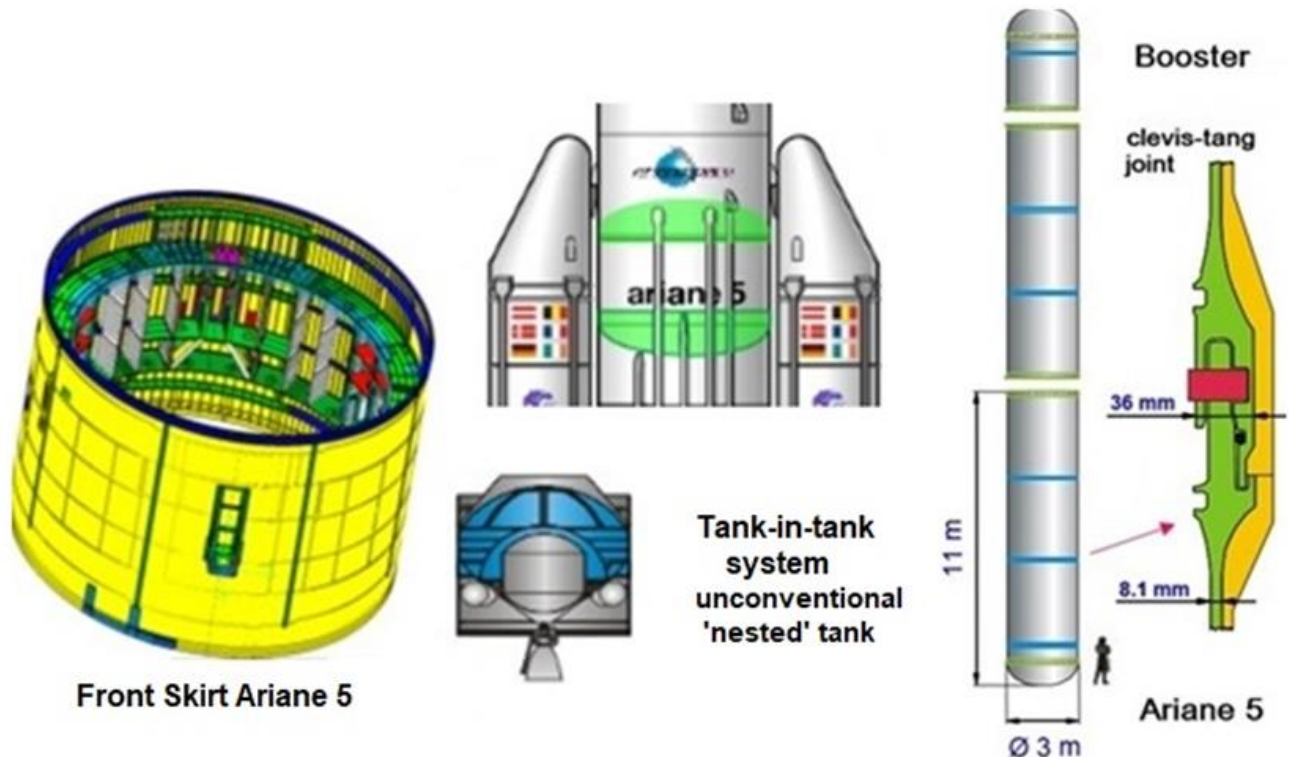


Some MAN Project Activities

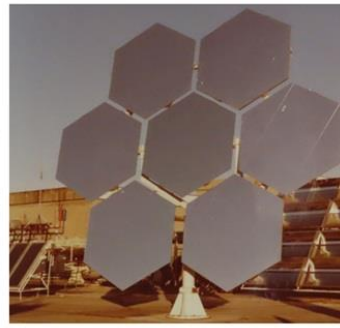
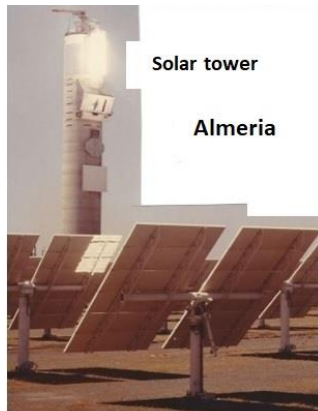
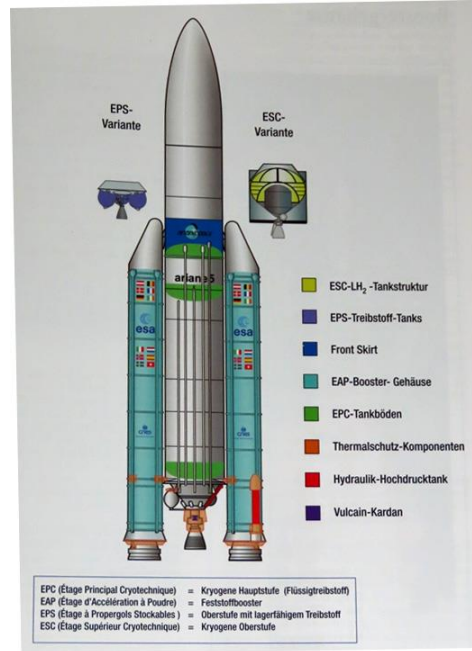
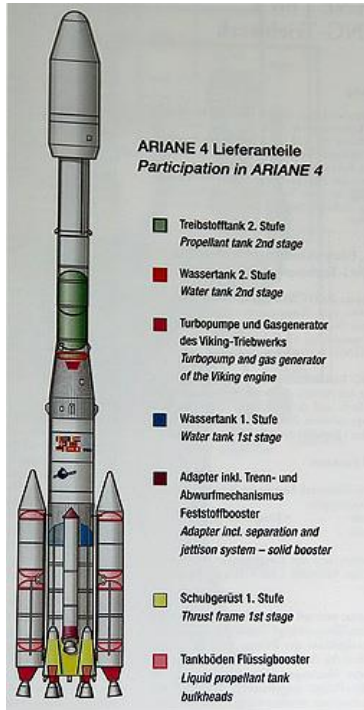
The slip of paper below was the start of my work at MAN-Neue Technologie in 1970 regarding the development of the **Ariane launcher family**.
"Make a design proposal for an offer, please" (Thrust structure).



Offer for Europe III as the Forerunner of Ariane I and following Family
(nearly all structural parts during the Ariane launcher family Development had to be designed)



Ariane Launcher 1-5



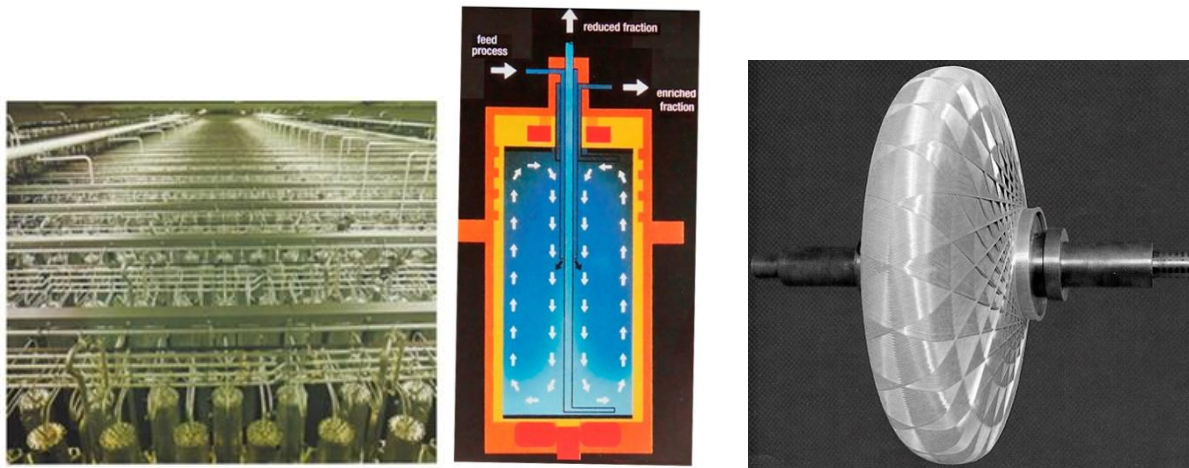
Solar field



Solar Farm Plant with tower.



Wind Energy Rotor GROWIAN, 1980, 51 m blade, GFRP shell) with wind speed measurement facility



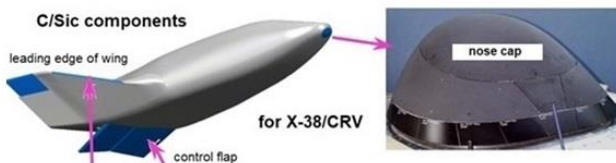
Uranium enrichment Centrifuges, Process. Composite brake energy storage flywheel for a bus



Airbus A330/340 drinking water tanks. High Pressure Vessels. Apogee Kevlar Motor Case

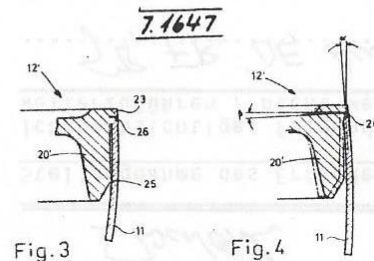
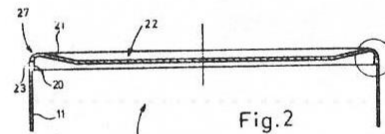
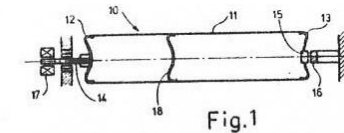
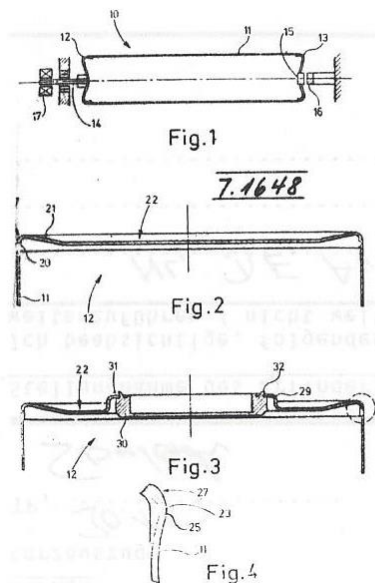


NIESYTO design, S. Guisard,





(left) FEA of a delamination-less energy storing fly wheel. (right) Development of a braided CFRP hip



Anspruch 1

Schleudertrommel für hohe Drehgeschwindigkeiten mit einer dünnwandigen Trommelwand und mindestens einem mit der Trommelwand verbundenen Deckel, dadurch gekennzeichnet, daß der Deckel (12, 13) in seinem Grundaufbau eine aus hochfestem Blech oder Band leicht kegelförmig geneigte Scheibe ist, deren Außenkontur für den Anschluß zur Trommelwand (11) einen Ringansatz (20) und deren Innenbereich (22) für die Aufnahme oder Verbindung von Antriebs- bzw. Lagerelementen annähernd kegelförmig oder topfförmig umgeformt ist.

2 - 10

Anspruch 1

Schleudertrommel mit mindestens einer mit der Trommelwand verbundenen rotationssymmetrischen Scheibe, dadurch gekennzeichnet, daß die Scheibe (12, 13) am Außenumfang ein im wesentlichen zylindrisches Teil (20) aufweist, das gegenüber dem Innendurchmesser der Trommelwand mit Übermaß ausgelegt ist und entlang einer Ringfläche mit der Innenseite der Trommelwand (11) verbunden ist, derart, daß die Scheibe bei Betrieb der Trommel aufgrund von Zentrifugalkräften eine Stülpbewegung ausübt, die Biegemomente der Trommel an der Fügestelle im wesentlichen aufhebt.

2 - 11

Patentbeteiligung P 27 09 410.6-23 "Thin-walled centrifuge cylinder (Schleudertrommel)"



Building an own Home at Markt Indersdorf (north of Munich)



break on the construction site





Of course, the text content in the scientific chapters would have deserved a revision and harmonization,
but the author is becoming 86.

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| 23 | Replacing fictitious UD Model Parameters $a_{\perp\perp}, a_{\parallel\parallel}$ by measurable Friction Values μ | 198 |
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This document comprises results of the author's never funded, non-supported research work performed in the vacant time at industry and as retired person. He assumes no liability for damages resulting from application.

Foreword

**This work is also widely *AI*-based, but here with the meaning *based on* hopefully some *Astonishing Ideas*.
Generative Deep Learning was necessary.**

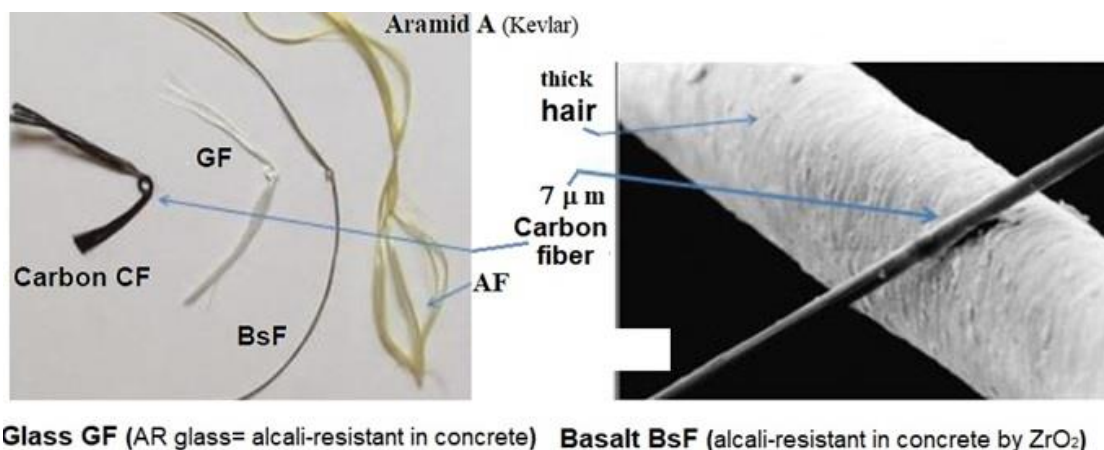
Findings of the author during his long-lasting Research Activities

Novel simulation-driven product development shifts the role of physical testing to virtual testing, to simulation, respectively. This requires High Fidelity and therefore the use of reliable material models. *Simulation means: Imitation of the operation of a real-world process and model adaption due to test information by performing many analyses.*

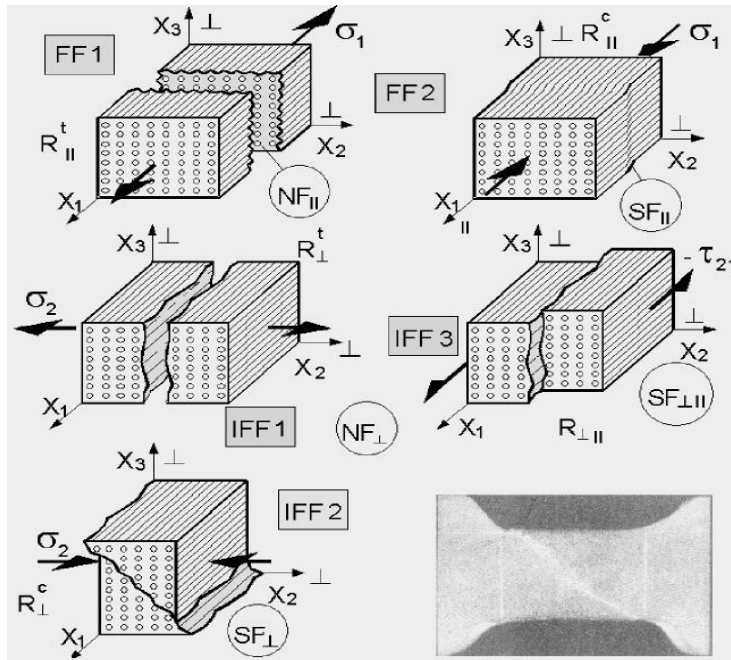
Basic desire of the macro-scopically working structural engineer is a material model linked to an ideally homogeneous material which might be isotropic or anisotropic. Connecting desire is: Be provided with a clear Strength Mechanics Building in order to get a cost-saving basis due to only analyze and test what is really physically necessary.

For the 3D-Demonstration of Strength are required - *nowadays practically a must regarding the usual 3D FEA stress output* – validated 3D Strength Failure Criteria (SFC) rendered by 3D failure bodies to firstly perform Design Dimensioning and to finally achieve Design Verification. All this is targeted in the following elaboration. Pre-information on the basic focus here, UD material:

* The following figure displays some of the different strengthening fibers applied in construction, and a comparison of a standard Carbon Fiber with a human hair.

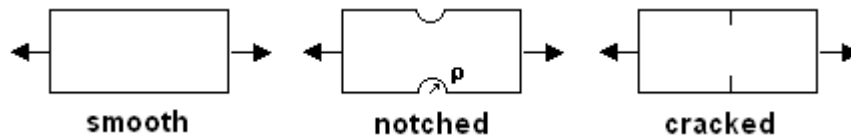


* And the next figure shall provide for the applied stringent failure mode thinking the observed 5 failure modes faced with Uni-directional fiber-reinforced materials.

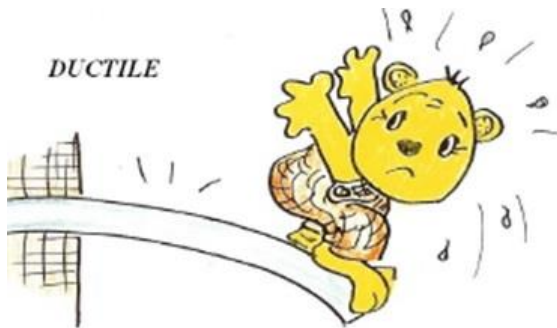


In the above context:

Two basic features are faced by the structure-designing engineers, three types of surfaces



and the behavior of the material, whether it is brittle (about $R^c > \approx 3 \cdot R^t$) or ductile.



One feels good until sudden fracture occurs

[Courtesy: Prof. C. Mattheck]

Ductile Fracture = type of failure in a material or a structure generally preceded by a large amount of plastic deformation



► Basic focus here: Smooth type structural parts.

1 Creation of the ‘Failure Mode Concept’ (FMC, about 1996)

Aim: Creation of a Static & Cyclic Strength Mechanics Building as basis for all material and of practical, physically-based SFCs.

Being since 1970 in the industrial composite business the author tried to firstly sort out in regular discussions with Alfred Puck applicable SFCs for UD materials. Puck developed in 1990 his Hashin-based Action-plane Inter-Failure-Failure SFC which was included in 2006 into the VDI 2014 guideline, sheet 3 (editor Cuntze).

Working with practically all material types the author was encouraged to find a Concept for all the material families isotropic, UD and further orthotropic ones including dense with porous materials.

The finally developed so-called Failure-Mode-Concept (FMC) incorporates a rigorous thinking in failure modes and can be briefly described by the FMC features, derived about 1995, which were the basis for the development of Cuntze’s macro-mechanical SFCs:

- Each failure mode represents 1 independent failure mechanism and thereby represents 1 piece of the complete failure surface.
- A failure mechanism at the lower micro-scopic mode level shall be considered in the applied desired macro-scopic SFC
- Each failure mechanism or mode is governed by 1 basic strength R , only (witnessed!)
- Each failure mode can be represented by 1 SFC.

This further includes:

- * Failure mode-wise mapping,
- * Stress invariant’s-based formulation,
- * Equivalent stress generation,
- * Each neat failure mode is governed by just one strength R^{mode} , witnessed for ductile and brittle materials, and
- * All SFC model parameters are measurable entities! Each SFC represents a failure surface, therefore for the originator the FMC will be the foundation upon which he physically based SFCs generated.

Hencky-Mises-Huber
(HMH)



Henri Tresca
1814-1885
Engineer



Richard von Mises
1883-1953
Mathematician



Eugenio Beltrami
1835-1900
Mathematician



Otto Mohr
1835-1918
Civil Engineer



Charles de Coulomb
1736-1806
Physician

‘Onset of Yielding’

‘Onset of Cracking (fracture)’

Fig.1-1: Some pioneers which set up strength failure hypotheses (ductile, brittle)

In the case of brittle materials the failure surface is the surface of a fracture failure body. Such a surface is determined by the peaks or ends of all failure stress vectors. The surface is mathematically defined by a Failure function F , which becomes 1 at ‘Onset-of-Failure’. $F = 1$ is the formulation of the SFC (*mathematically, we write a condition*). Fig.1-1 above presents the pioneers in the isotropic SFC field.

The author’s idea was to create physically-based SFCs and to note his Lessons Learned *LL* during the elaboration. The FMC was originally derived for UD materials because there was the big demand at that time. The employed stress invariants shall be presented via isotropic knowledge:

Beltrami, Schleicher et al. assumed at initiation of yield that the strain energy (denoted by W) in a solid cubic element of a material will consist of two portions:

$$W = \int \{\sigma\} \{\varepsilon\} d\{\varepsilon\} = W_{\text{Vol}} + W_{\text{shape}} \quad \text{with} \quad \{\sigma\} = (\sigma_1, \sigma_2, \sigma_3, \tau_{23}, \tau_{13}, \tau_{12})^T.$$

Including Hooke's law in the case of a **transversely-isotropic** (UD solid) the expression will take the form, using $s_{ik} :=$ compliance coefficients, $E :=$ elasticity modulus, $\nu :=$ Poisson’s ratio,

$$W = [s_{11} \cdot \sigma_1^2 + s_{22} \cdot \sigma_2^2 + s_{33} \cdot \sigma_3^2 + s_{44} \cdot \tau_{23}^2 + s_{55} \cdot (\tau_{12}^2 + \tau_{13}^2)] / 2 + s_{12} \cdot (\sigma_1 \sigma_2 + \sigma_1 \sigma_3) + s_{23} \cdot \sigma_2 \sigma_3 = \frac{I_1^2}{2 \cdot E_{\parallel}} + \frac{I_2^2 \cdot (1 - \nu_{\perp\perp})}{4 \cdot E_{\perp}} - \frac{\nu_{\parallel\parallel} \cdot I_1 \cdot I_2}{E_{\parallel}} + \frac{I_3}{2 \cdot G_{\parallel\perp}} + \frac{I_4 \cdot (1 + \nu_{\perp\perp})}{4 \cdot E_{\perp}}.$$

volume volume volume shape shape

with the invariants $I_1 = \sigma_1, I_2 = \sigma_2 + \sigma_3 ; I_3 = \tau_{31}^2 + \tau_{21}^2 ; I_4 = (\sigma_2 - \sigma_3)^2 + 4\tau_{23}^2 ;$
 $I_5 = (\sigma_2 - \sigma_3) (\tau_{31}^2 - \tau_{21}^2) - 4\tau_{23} \tau_{31} \tau_{21}.$

In the **isotropic** case analogously follows, however simpler,

$$W = \left[\frac{1 - 2\nu}{3} I_1^{iso2} + \frac{2 + 2\nu}{3} 3J_2^{iso} \right] / 2E$$

volume shape

with $I_1^{iso} = \underline{f(\sigma)} = \sigma_I + \sigma_{II} + \sigma_{III}, 6 \cdot J_2^{iso} = \underline{f(\tau)} = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2,$

It is known, both portions in the bracket above are used to formulate a failure function

$$F = c_1 \cdot \frac{(1 - 2\nu) \cdot I_1^{iso2}}{3\bar{R}^2} + c_2 \cdot \frac{(2 + 2\nu) \cdot 3J_2^{iso}}{3\bar{R}^2}.$$

volume shape

Fig. 1-2 below displays for the 2 material families above the physically-based choice of invariants:

From Beltrami , Mises (HMH), and Mohr / Coulomb (friction) can be concluded:

Below invariant terms - used in a FMC-based *failure function F* - can be dedicated to a physical mechanism in the solid = cubic material element:

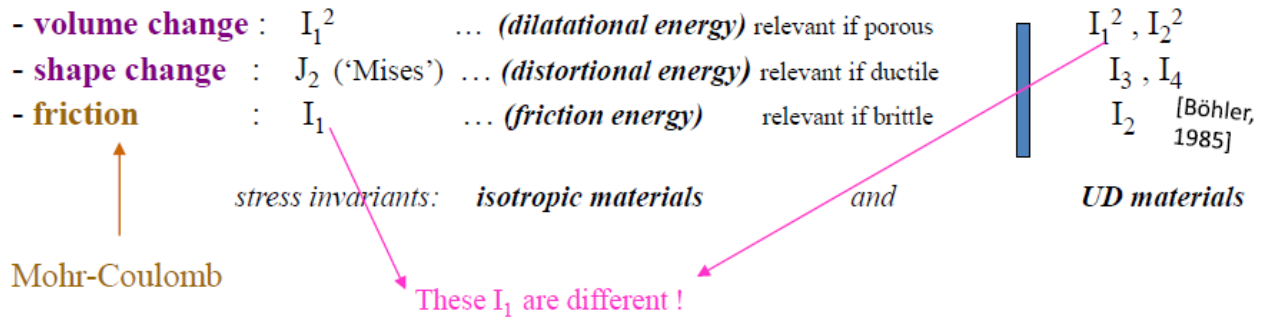


Fig.1-2: Reasons for choosing invariants when creating FMC-based SFCs

Exemplarily, the isotropic SFC model, spanning up the fracture body in the compression domain, shall be used for demonstration. The complete SFC reads:

$$\text{Shear Fracture SF, } I_1 < 0: F^{SF} = F^\tau = c_1 \cdot \frac{6 \cdot J_2}{2 \cdot \bar{R}^{c_2}} \cdot \Theta_\tau + c_2 \cdot \frac{I_1}{\bar{R}^c} + c_3 \cdot \left(\frac{I_1}{\bar{R}^c} \right)^2$$

Herein, the first part of the SFC represents the **shape change**, the second the **friction effect**, the third the **volume change** and the non-circularity parameter Θ_τ describing the inherent, nevertheless often not known 120°-symmetry of the failure bodies of isotropic brittle and ductile material, too (see a later chapter).

Above invariants can be formulated in 3D structural component stresses, in principal stresses and in Mohr stresses, which will become essential when deriving a stress state-caused fracture angle and the so-called cohesive strength..

Note, please: Strength notations

R means strength (resistance) in general and further Strength Design Allowable used for Design Verification. \bar{R} means average strength used for modelling, mapping of the course of test data.

LL: Similarly behaving materials possess the same shape of a fracture body using the same SFC!

2 Interaction of Stresses by the application of Strength Failure Criteria

Aim: Provision of a failure mode-based stress-interaction ('Modal') and not a mathematical global one.

The derivation of the FMC-based SFCs builds up on the hypotheses of Beltrami, Hencky-Mises-Huber (HMH) and Mohr-Coulomb. Therefore the depicted SFC approaches consider, that the solid material element may experience, generated from different energy portions, a shape change (HMH), a volume change and friction. FMC-based SFCs will be given for a large variety of isotropic brittle structural materials such as porous Concrete Stone, Normal Concrete, UHPC sandstone, monolithic ceramics and for the transversely-isotropic fiber-reinforced polymers Lamina (ply, lamella) and finally orthotropic fabrics inclusively fabric ceramics, see [CUN22, Cun23a,24b].

Since two decades the author believes in a macroscopically-phenomenological 'complete classification' system, where all strength failure types are included, see the figure below. In his assumed system several relationships may be recognized: (1) Shear stress yielding SY, followed by Shear fracture SF considering 'dense' materials. For porous materials under compression, the SF for dense materials is replaced by Crushing Fracture CrF. (2) In order to complete a mechanical system beside SY also NY should exist. This could be demonstrated by PMMA (plexiglass) with its chain-based texture showing NY due to crazing failure under tension and SY in the compression domain, [see subsection 9.1 or CUN22,§4.1]. The right side of the scheme outlines that a full similarity of the 'simpler' isotropic materials with the transversely-isotropic UD materials exists.

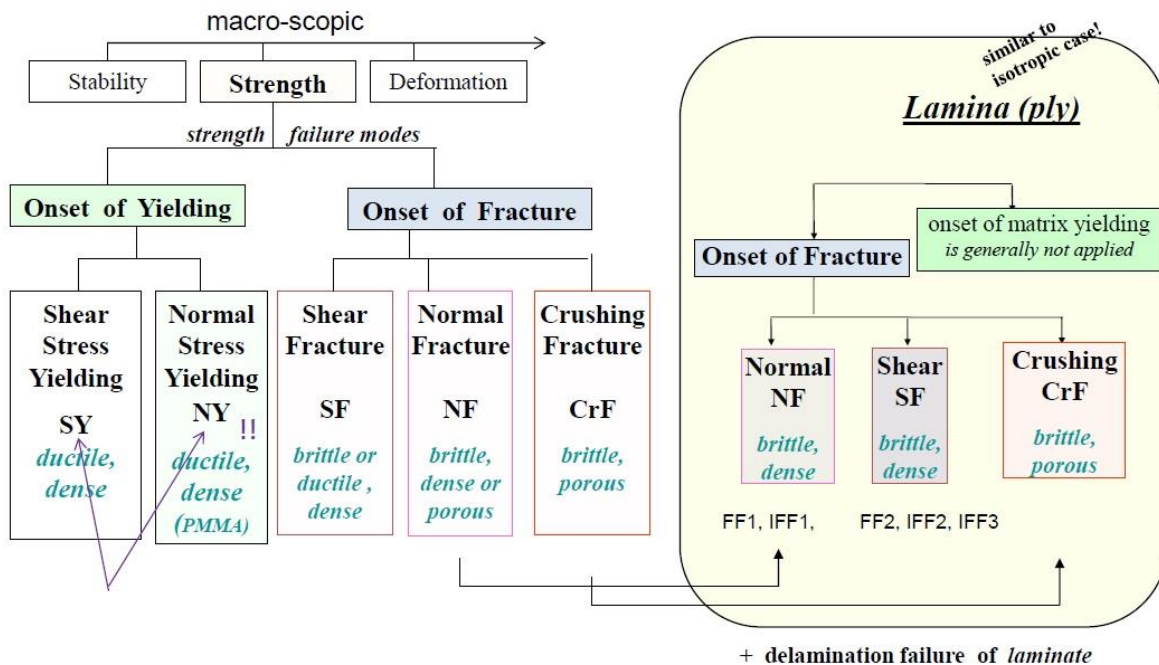


Fig.2-1.: Scheme of macro-scopic strength failure types and modes of isotropic materials and transversely-isotropic UD-materials (Cuntze1998)

LL:

* Failure behavior of Fiber-Reinforced materials is similar to isotropic ones

* Principally, instead of stress-based SFC, strain-based SFC might be applied if the full stress-strain history is accurately considered. However, just limit strain conditions are used in pre-dimensioning (§22), because the certification process is stress-based.

3 Material Symmetry and ‘Generic’ Number (*material inherent?*)

Aim: Consideration of the available material knowledge.

During the derivation of the FMC a closer look at material symmetry facts was taken whereby the question arose: “Does a material symmetry–linked Generic Number exist with a number 2 for isotropic and 5 for UD materials?”

Under the design-simplifying presumption “Homogeneity is a permitted assessment for the material concerned” and regarding the respective material tensors, it follows from material symmetry that the number of strengths equals the number of elasticity properties! Fracture

morphology gives further evidence: Each strength property corresponds to a distinct strength failure mode and to a distinct strength failure type, to Normal Fracture (NF) or to Shear Fracture (SF). This seems to mean, that a characteristic number of quantities is fixed: 2 for isotropic material and 5 for the transversely-isotropic UD lamina (\equiv lamellas in civil engineering). Hence, the applicability of material symmetry involves that in general just a minimum number of properties needs to be measured (benefits:& test cost + time) which is helpful when setting up strength test programs. \Rightarrow Witnessed material symmetry knowledge seems to tell: “There might exist a ‘generic’ (*term was chosen by the author*) material inherent number for”:

Isotropic Material: of **2**

- 2 elastic ‘constants’, 2 strengths, 2 strength failure modes fracture (NF with SF) and 2 fracture mechanics modes (defined as modes, where crack planes do not turn)
- 1 physical parameter (such as the coefficient of thermal expansion CTE, the coefficient of moisture expansion CME, and the friction value μ , etc.)

Transversely-Isotropic Material: of **5** for the these basically brittle materials

- 5 elastic ‘constants’, 5 strengths, 5 strength failure modes fracture (NFs with SFs)
- 2 physical parameters (CTE, CME, $\mu_{\perp\perp}$, $\mu_{\parallel\parallel}$ etc.).

Orthotropic Material: of **9** (6).

This looks to be proven by the investigation of Normal Yielding NY of plexiglass and (*theoretically*) by a compressive fracture toughness K_{IIcr}^c for a brittle material with an ideally homogeneous state at the crack tip [see section 9 or CUN22§4].

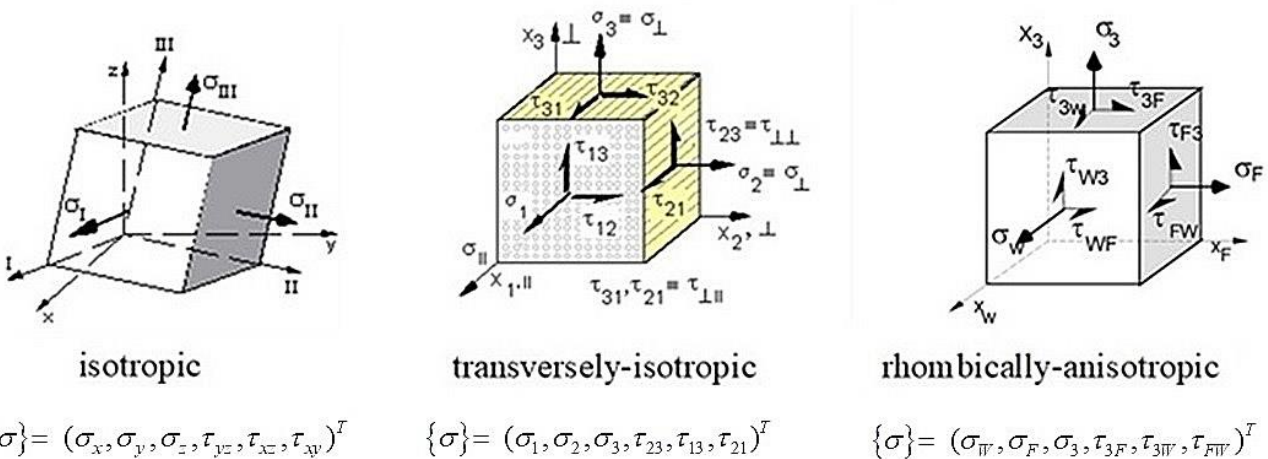


Fig.3-1: Presentation of the stresses faced with the envisaged three material families

LL: A ‘generic’ number seems to be inherent for the different material families, as the author found.

4 Direct use of a Friction Value μ in the SFCs of Isotropic and UD materials

Aim: Direct use of the measurable μ instead of applying a μ -hiding friction model parameter.

Mohr-Coulomb acts. Therefore, in the case of compressed brittle materials the effect of friction is to capture, which usually is performed by ‘fictitious’ friction-linked model parameters. Such a model parameter for friction, here the a or the b in the SFC, can be replaced by the measured μ . In order to achieve this, the very challenging task to transform an SFC in structural stresses into a SFC in Mohr stresses had to be successfully to be performed [Cun23c, Annex2]. Ultimately, an engineer prefers the application of a measurable and physically understandable value μ , especially, because it does not scatter that much, and this is essential in design.

For isotropic materials this direct use is depicted in Table 4-1.

Table 4-1, Isotropic materials: Simple 2D formulation

| |
|---|
| <p>Assumption: Fracture failure body is rotationally symmetric like Mises yield body.</p> $I_1 = (\sigma_I + \sigma_{II} + 0) = f(\boldsymbol{\sigma}), \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - 0)^2 + (0 - \sigma_I)^2 = f(\boldsymbol{\tau})$ <p>* Normal Fracture NF, $I_1 > 0$ \leftrightarrow * Shear Fracture SF, $I_1 < 0$</p> <p>Strength Failure Criterion (SFC), mode interaction exponent $m = 2.7$, $\mu = 0.2$</p> $Eff^{NF} = \frac{\sqrt{4J_2 - I_1^2/3} + I_1}{2 \cdot R^t} = \frac{\sigma_{eq}^{NF}}{\bar{R}^t} \leftrightarrow Eff^{SF} = \frac{c_2^{SF} \cdot I_1 + \sqrt{(c_2^{SF} \cdot I_1)^2 + 12 \cdot c_1^{SF} \cdot 3J_2}}{2 \cdot R^c} = \frac{\sigma_{eq}^{SF}}{\bar{R}^c}$ $c_1^{SF} = 1 + c_2^{SF}, \quad c_2^{SF} = (1 + 3 \cdot \mu) / (1 - 3 \cdot \mu) \text{ from } \mu = \cos(2 \cdot \theta_{fp}^c \cdot \pi / 180)$ $Eff = [(Eff^{NF})^m + (Eff^{SF})^m]^{m^{-1}} \rightarrow f_{RF} = 1 / Eff$ |
|---|

For UD material, this is executed within the full SFC set in the Table 4-21:

Table 4-21, UD materials: 3D SFC formulations for FF1, FF2 and IFF1, IFF2, IFF3 and 2D

| |
|---|
| <p>FF1: $Eff^{\parallel\sigma} = \bar{\sigma}_1 / \bar{R}_\parallel^t = \sigma_{eq}^{\parallel\sigma} / \bar{R}_\parallel^t$ with $\bar{\sigma}_1 \cong \boldsymbol{\varepsilon}_1^t \cdot E_\parallel$ (matrix neglected)</p> <p>FF2: $Eff^{\parallel\tau} = -\bar{\sigma}_1 / \bar{R}_\parallel^c = +\sigma_{eq}^{\parallel\tau} / \bar{R}_\parallel^c$ with $\bar{\sigma}_1 \cong \boldsymbol{\varepsilon}_1^c \cdot E_\parallel$</p> <p>IFF1: $Eff^{\perp\sigma} = [(\sigma_2 + \sigma_3) + \sqrt{\sigma_2^2 - 2\sigma_2 \cdot \sigma_3 + \sigma_3^2 + 4\tau_{23}^2}] / 2\bar{R}_\perp^t = \sigma_{eq}^{\perp\sigma} / \bar{R}_\perp^t$</p> <p>IFF2: $Eff^{\perp\tau} = [a_{\perp\perp} \cdot (\sigma_2 + \sigma_3) + b_{\perp\perp} \sqrt{\sigma_2^2 - 2\sigma_2 \sigma_3 + \sigma_3^2 + 4\tau_{23}^2}] / \bar{R}_\perp^c = \sigma_{eq}^{\perp\tau} / \bar{R}_\perp^c$</p> <p>IFF3: $Eff^{\perp\parallel} = \{ [b_{\perp\parallel} \cdot I_{23-5} + (\sqrt{b_{\perp\parallel}^2 \cdot I_{23-5}^2 + 4 \cdot \bar{R}_{\perp\parallel}^2 \cdot (\tau_{31}^2 + \tau_{21}^2)})] / (2 \cdot \bar{R}_{\perp\parallel}^3) \}^{0.5} = \sigma_{eq}^{\perp\parallel} / \bar{R}_{\perp\parallel}$</p> $\{ \sigma_{eq}^{mode} \} = (\sigma_{eq}^{\parallel\sigma}, \sigma_{eq}^{\parallel\tau}, \sigma_{eq}^{\perp\sigma}, \sigma_{eq}^{\perp\tau}, \sigma_{eq}^{\perp\parallel})^T, \quad I_{23-5} = 2\sigma_2 \cdot \tau_{21}^2 + 2\sigma_3 \cdot \tau_{31}^2 + 4\tau_{23}\tau_{31}\tau_{21}$ <p>Inserting the compressive strength point $(0, -\bar{R}_\perp^c) \rightarrow a_{\perp\perp} \cong \mu_{\perp\perp} / (1 - \mu_{\perp\perp})$, $b_{\perp\perp} = a_{\perp\perp} + 1$ from a measured fracture angle $\rightarrow \mu_{\perp\perp} = \cos(2 \cdot \theta_{fp}^c \cdot \pi / 180)$, for $50^\circ \rightarrow \mu = 0.174$.</p> <p>$b_{\perp\parallel} = 2 \cdot \mu_{\perp\parallel}$. Typical friction value ranges: $0 < \mu_{\perp\parallel} < 0.25$, $0 < \mu_{\perp\perp} < 0.2$.</p> |
|---|

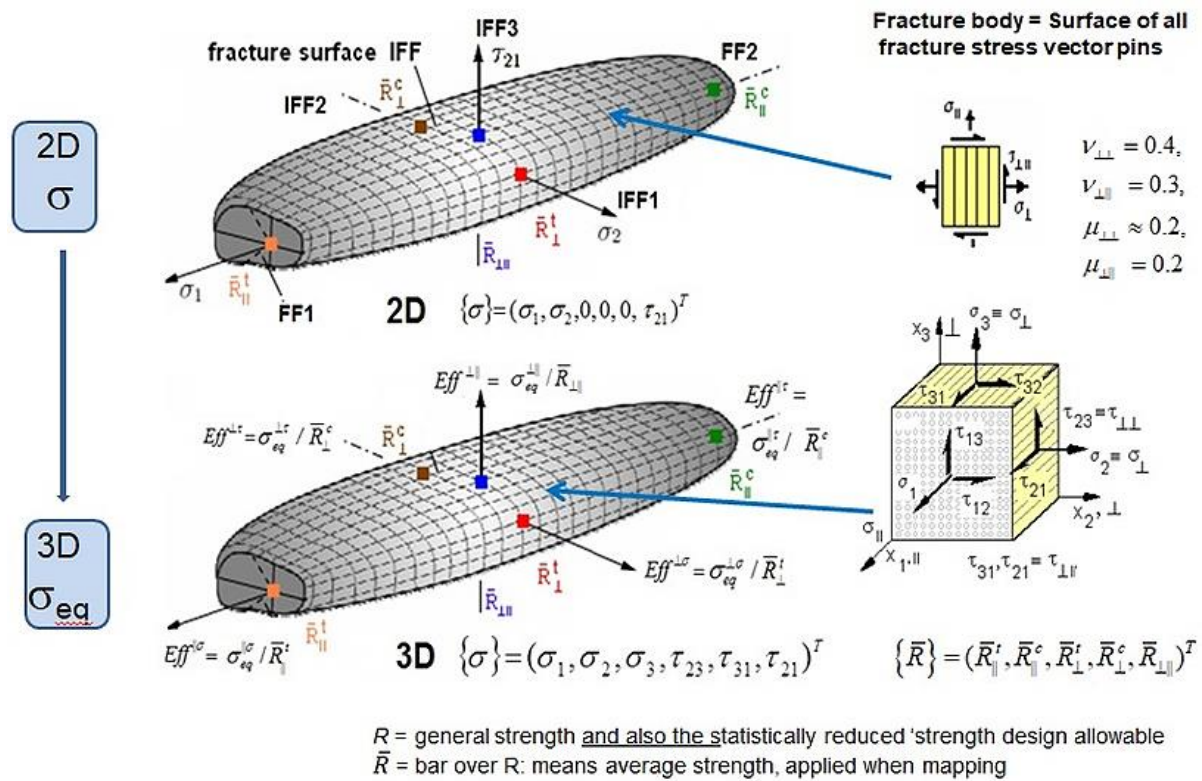


Fig.4-2: From a 2D failure body to a 3D failure body by replacing stresses by equivalent stresses

The upper figure displays the UD failure body as the visualization of the associated SFC set. The lower figure documents that if moving from the ply stresses to the mode-linked equivalent ply stresses one keeps the same UD failure body, usable now as 3D failure body!

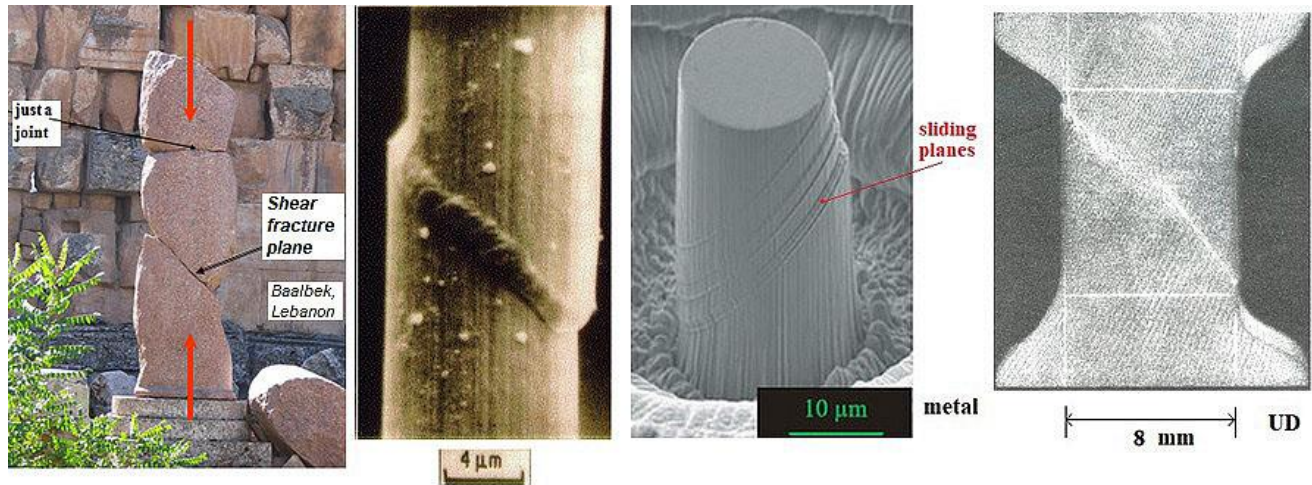


Fig.4-3, Friction driven shear fracture planes at extreme length scales: Fracture angles of the brittle materials Rock material, Carbon fiber [K. Schulte, TU Hamburg-Harburg], Ductile metal compression cut from a single crystal (deformed pillar after compression testing. Monnet, G. & Pouchon, M. A. (2013), Determination of the so-called critical resolved shear stress and the friction stress in austenitic stainless steels by compression of pillars extracted from single grains', Mater. Letters 98, 128-130) and laterally compressed UD-CFRP

LL:

- * Often, SFCs employ just strengths and no friction value. This is physically not accurate and the undesired consequence in Design Verification is: The Reserve Factor may be not on the safe side..
- * In contrast to the 'doing': Friction must and can now be directly considered by the measured μ
- * Friction occurs similarly over the scales.

5 Material stressing effort Eff (Werkstoffanstrengung)

Aim: Generation of a physical basis for the interaction of failure modes and for an excellent understanding of a failure body (Eff = 100%) with multi-axial strength (capacity) values.

If several failure modes are activated by the stress state then the application of the so-called *material stressing effort Eff* is very helpful (in German: *Werkstoffanstrengung*. This artificial name had to be created in the World Wide Failure Exercise on UD-SFCs, together with the UK-organizers, because an equivalent term to the excellent German term is not known in English).

The full *Eff* consists of all mode portions Eff^{mode} . It works analogous to ‘Mises’

$$Eff^{yield\ mode} = \sigma_{eq}^{Mises} / R_{0.2} \rightarrow Eff^{fracture\ mode} = \sigma_{eq}^{fracture\ mode} / R .$$

The contribution of each single Eff^{mode} informs the designing engineer about the importance of the single portions in the SFC and thereby about the critical failure driving mode and thereby outlining the design-driving mode.

Whereas the structural engineer is more familiar with the equivalent stress the material engineer prefers above ‘material stressing effort’ *Eff*. The terms are linked by $\sigma_{eq}^{mode} = Eff^{mode} \cdot R^{mode}$.

The use of *Eff* supports ‘Understanding the multi-axial strength capacity of materials’ (Fig.13-4): For instance, 3D-compression stress states have a higher bearing capacity, but the value of *Eff* nevertheless stays at 100%. Consequently, this has nothing to do with an increase of a (*uniaxial*) technical strength *R* which is a fixed result of a Standard!

The following fracture test result of a brittle concrete impressively shows how a slight hydrostatic pressure of 6 MPa increases the strength capacity in the longitudinal axis from 160 MPa up to 230 MPa - 6 MPa = 224 MPa. Thereby, the benefit of 3D-SFCs–application could be proven as the fracture stress states below depict both the Effs are 100% :

$$\sigma_{fr} = (\sigma_I, \sigma_{II}, \sigma_{III})_{fr}^T = (-160, 0, 0)^T \text{MPa} \leftrightarrow (-224-6, -6, -6)^T \text{MPa} .$$

Because both the Effs are 100% for $(-160, 0, 0)^T$ and for $(-224-6, -6, -6)^T$ [CUN, §5.5] !

This can be transferred to the quasi-isotropic plane of the transversely-isotropic UD-materials, $\sigma_2 - \sigma_3$, see [Cun23c], and to the orthotropic CMC fabric, when beside shear τ_{WF} the compressive stress σ_W^c acts together with σ_F^c and both activate friction on the sides [Cun24b].

LL:

The physically clear-based quantity Eff gives an impressive interpretation of what 100% strength capacity in 1D- 2D- and 3D stress states physically really means.

6 So-called ‘Global’ SFCs and (failure mode-linked) ‘Modal’ SFCs, Mode-interaction

Aim: Shortly explaining the difference of ‘Global’ and ‘Modal’ SFCs.

There are a lot of possibilities to generate SFCs. *Fig.6-1* presents a survey:

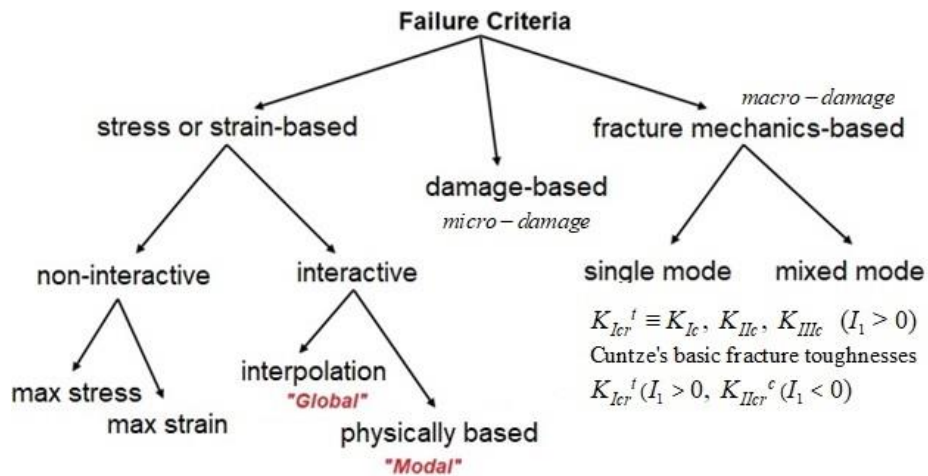


Fig.6-1: Possibilities to generate SFCs when following Klaus Rohwer [Rohwer K.: Predicting Fiber Composite Damage and Failure. Journal of Composite Materials, published online 26 Sept. 2014 (online version of this article can be found at: <http://jcm.sagepub.com/content/early/2014/09/26/00219983145538851>]

Present so-called interacting SFCs are a sub-part, which can be basically separated into two groups, ‘global’ and ‘modal’ ones. The HMH yield failure condition is a modal SFC that captures just one failure mode. The author choose the term global as a ”play on words” to modal and to being self-explaining. Global SFCs describe the full failure surface by one single mathematical equation. This means that for instance a change of the UD *tensile* strength \bar{R}_1^t affects the failure curve in the *compression* domain, where no physical impact can be. Global SFCs couple physically different failure modes whereas modal SFCs describe each single failure mode and therefore will better map the course of test data and not lead to a wrong Reserve Factor in any mode domain:

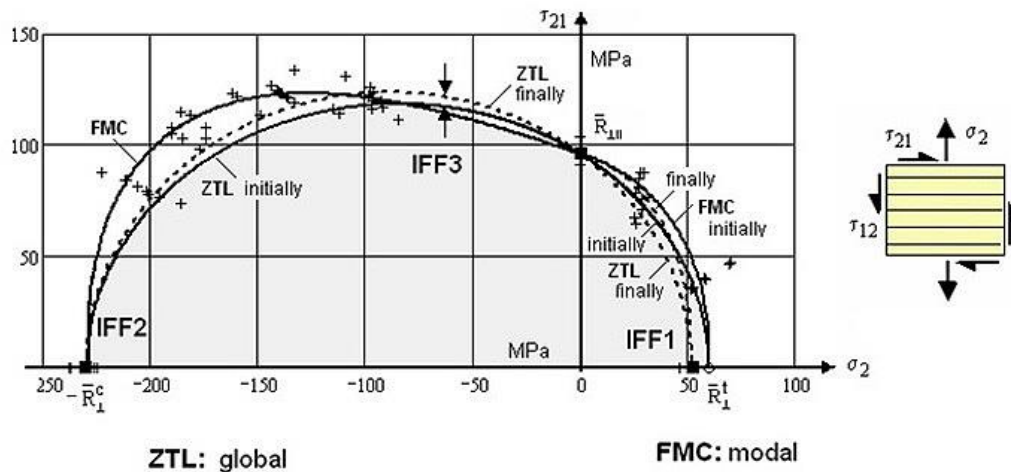
$$1 \text{ Global SFC} \quad F(\{\sigma\}, \{R\}) = 1 \quad \text{mathematically ‘married’ modes}$$

$$\text{Set of Modal SFCs} \quad F(\{\sigma\}, \{R^{\text{mode}}\}) = 1 \quad \text{single mode formulations.}$$

In the case of modal SFCs (such as the FMC-based ones) also equivalent stresses can be computed, like ‘isotropic Mises’,

$$\{\sigma_{eq}^{\text{mode}}\} = \left(\sigma_{eq}^{\parallel\sigma}, \sigma_{eq}^{\parallel\tau}, \sigma_{eq}^{\perp\sigma}, \sigma_{eq}^{\perp\tau}, \sigma_{eq}^{\parallel\perp} \right)^T .$$

and this is advantageous for design decisions. Within a ‘global’ SFC formulation all modes are mathematically married. This has a very bad impact: Each change, coming from a new test information for any pure mode, has an effect on all other independent failure modes and might include some redesign, see the full change of the ZTL-curve in *Fig.6-2*. Such a bad impact is never faced using a ‘modal’ formulation, like the FMC one.



$$\frac{\sigma_2^2}{\bar{R}_\perp^t \cdot \bar{R}_\perp^c} + \sigma_2 \cdot \left(\frac{1}{\bar{R}_\perp^t} - \frac{1}{\bar{R}_\perp^c} \right) + \frac{\tau_{21}^2}{\bar{R}_{\perp\parallel}} = 1 \quad \left(\frac{\sigma_2}{\bar{R}_\perp^t} \right)^m + \left(\frac{-\sigma_2}{\bar{R}_\perp^c} \right)^m + \left(\frac{|\tau_{21}|}{\bar{R}_{\perp\parallel} - \mu_{\perp\parallel} \cdot \sigma_2} \right)^m = 1$$

Fig.6-2: Modelling example, impact of a novel test information in the mode IFF1 considering a global (ZTL-SFC, still used in the HSB) and a modal SFC,

Considering the shortcomings of ‘global’ UD SFCs, my friend John Hart-Smith cited:

“It is scientifically incorrect to employ polynomial interaction failure models (the ‘global’ ones), if the mechanism of failure changes”!

Of course, a modal FMC-approach requires an interaction in all the mode transition zones. This is performed by a probabilistic approach, using a ‘series failure system’ in the transition zone of adjacent modes NF with SF, reading

$$Eff = \sqrt[m]{(Eff^{\text{mode } 1})^m + (Eff^{\text{mode } 2})^m + \dots} = 1 = 100\% \quad \text{for Onset-of-Failure}$$

and applying a mode interaction exponent m , also termed rounding-off exponent, the size of which is high in case of low scatter and vice versa. The value of m is obtained by curve fitting of test data in the transition zone of the interacting modes. Experience delivered that $2.5 < m < 2.9$.

With the FMC-based SFCs for the three ‘material families’ available multi-axial fracture test data were mapped by the author to validate the SFCs being the mathematical descriptions of the envisaged fracture failure models. For a large variety of materials the associated fracture bodies were displayed in later chapters with distinct cross-sections of them, for instance for the isotropic applications: Principal stress plane, octahedral stress plane and tensile and meridian planes. Various links or interrelationships between the materials could be outlined.

LL:

- * So-called ‘Global’ SFCs couple physically different failure modes whereas the Modal SFCs describe each single failure mode and therefore will better map the course of test data and not lead to a wrong Reserve Factor
- * Here, global and modal have a similar level of abstraction, as in the case of stability the terms ‘global’ and ‘local’ have
- * Similarities between the materials could be found
- * The surface of the failure body reads: $F = 1$, for a ‘global’ formulation and $Eff = 1$ for a ‘modal’ formulation.

7 Collection of Derived SFCs, Interaction of Failure Modes and a Multi-fold Mode

7.1 Presentation of the derived Failure Mode Concept-based Strength Failure Criteria

Aim: Provision of SFCs which were derived on the same concept basis.

For the mentioned three material families the associate SFCs are tabled on the following pages:

I a : 3D-isotropic SFCs of dense Isotropic Materials for NF and SF, 120°-rotational symmetry

2 modes → 2 SFCs, is in line with ‘generic’ number according to the FMC.

| | | |
|--|---|--|
| <p>Normal Fracture NF for $I_1 > 0$</p> $F_{\sigma}^t = F^{NF} = c^{NF} \cdot \Theta^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t} = 1$ $Eff^{NF} = c^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t} = \frac{\sigma_{eq}^{NF}}{\bar{R}^t}$ | ↔ | <p>Shear Fracture SF for $I_1 < 0$</p> $F_{\tau}^c = F^{SF} = c_{1\Theta}^{SF} \cdot \frac{6J_2 \cdot \Theta^{SF}}{2\bar{R}^{c2}} + c_{2\Theta}^{SF} \cdot \frac{I_1}{\bar{R}^c} = 1$ $Eff^{SF} = \frac{c_{2\Theta}^{SF} \cdot I_1 + \sqrt{(c_{2\Theta}^{SF} \cdot I_1)^2 + c_{1\Theta}^{SF} \cdot 12J_2 \cdot \Theta^{SF}}}{2 \cdot \bar{R}^c} = \frac{\sigma_{eq}^{SF}}{\bar{R}^c}$ |
|--|---|--|

If a failure body is rotationally symmetric, then $\Theta = 1$ like for the neutral or shear meridian.

A two-fold acting mode makes the rotationally symmetric fracture body 120°-symmetric and is modelled by $\Theta(J_3)$ using the invariant J_3 and Θ as non-circularity function with d as non-circularity parameter

$$\Theta^{NF} = \sqrt[3]{1 + d^{NF} \cdot \sin(3\vartheta)} = \sqrt[3]{1 + d^{NF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}}, \quad \Theta^{SF} = \sqrt[3]{1 + d^{SF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}}$$

Lode angle ϑ , here set as $\sin(3 \cdot \vartheta)$ with ‘neutral’ shear meridian angle set $\vartheta = 0^\circ$;

tensile meridian angle $30^\circ \rightarrow \Theta^{NF} = \sqrt[3]{1 + d^{SF} \cdot (+1)}$; compr. merid. angle $-30^\circ \rightarrow \Theta^{SF} = \sqrt[3]{1 + d^{SF} \cdot (-1)}$.

Equation of the fracture failure body: $Eff = [(Eff^{NF})^m + (Eff^{SF})^m]^{m^{-1}} = 1 = 100\%$ total effort

$$Eff = \sqrt[m]{\left(c^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t}\right)^m + \left(\frac{c_{2\Theta}^{SF} \cdot I_1 + \sqrt{(c_{2\Theta}^{SF} \cdot I_1)^2 + c_{1\Theta}^{SF} \cdot 12J_2 \cdot \Theta^{SF}}}{2 \cdot \bar{R}^c}\right)^m} = 1.$$

Curve parameter relationships obtained by inserting the compressive strength point $(0, -\bar{R}^c, 0)$:

* Rotationally symmetric: $c_1^{SF} = 1 + c_2^{SF}$, $d^{SF} = 0$, friction parameters are equal $c_2^{SF} \equiv c_{2\Theta}^{SF}$

$$c_2^{SF} = c_{2\Theta}^{SF} \approx (1 + 3 \cdot \mu) / (1 - 3 \cdot \mu) \text{ from } \mu = \cos(2 \cdot \theta_{fp}^\circ \cdot \pi / 180) \text{ and for } 50^\circ \rightarrow \mu = 0.174.$$

* 120°-rotationally symmetric: $c_{1\Theta}^{SF} = 1 + c_2^{SF} \cdot \sqrt[3]{1 + d^{SF} \cdot (-1)}$ with

c^{NF} , Θ^{NF} from the two points $(\bar{R}^t, 0, 0)$ and $(\bar{R}^{tt}, \bar{R}^{tt}, 0)$ or by a minimum error fit, if data,

c^{SF} , Θ^{SF} from the two points $(-\bar{R}^c, 0, 0)$ and $(-\bar{R}^{cc}, -\bar{R}^{cc}, 0)$ or by minimum error fit.

A paraboloid serves as closing cap $\frac{I_1}{\sqrt{3} \cdot \bar{R}^t} = s^{cap} \cdot \left(\frac{\sqrt{2J_2 \cdot \Theta^{NF}}}{\bar{R}^t}\right)^2 + \frac{\max I_1}{\sqrt{3} \cdot \bar{R}^t}$.

$$I_1 = (\sigma_I + \sigma_{II} + \sigma_{III}) = f(\sigma), \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2 = f(\tau)$$

$$27J_3 = (2\sigma_I - \sigma_{II} - \sigma_{III}) \cdot (2\sigma_{II} - \sigma_I - \sigma_{III}) \cdot (2\sigma_{III} - \sigma_I - \sigma_{II}).$$

I b : 3D-isotropic SFCs of dense Isotropic Materials for NF and SF, 120°-rotational symmetry

Table I b collects all information necessary to design dimension a porous isotropic material like a foam or a concrete stone. These materials experience 120°-rotational symmetry.

'Porous' isotropic material: SFC formulations for NF and CrF, 120°-rotational symmetry

$$\begin{aligned} \text{Normal Fracture NF for } I_1 > 0 & \leftrightarrow \text{Crushing Fracture CrF for } I_1 < 0 \\ F^{NF} = c^{NF} \cdot \Theta^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t} = 1 & \leftrightarrow F^{CrF} = c^{CrF} \cdot \Theta^{CrF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{CrF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^c} = 1 \\ Eff^{NF} = c^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t} = \frac{\sigma_{eq}^{NF}}{\bar{R}^t} & \leftrightarrow Eff^{CrF} = c^{CrF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{CrF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^c} = \frac{\sigma_{eq}^{CrF}}{\bar{R}^c} \end{aligned}$$

If a failure body is rotationally symmetric, then $\Theta = 1$ like for the neutral or shear meridian, respectively .

A 2-fold acting mode makes the rotationally symmetric fracture body 120°-symmetric and is modelled by using the invariant J_3 and Θ as non-circularity function with d as non-circularity parameter

$$\Theta^{NF} = \sqrt[3]{1 + d^{NF} \cdot \sin(3\vartheta)} = \sqrt[3]{1 + d^{NF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}} \quad \leftrightarrow \quad \Theta^{CrF} = \sqrt[3]{1 + d^{CrF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}}$$

Lode angle ϑ , here set as $\sin(3 \cdot \vartheta)$ with 'neutral' (shear meridian) angle $\vartheta = 0^\circ$ ($\rightarrow \Theta = 1$) ;

tensile meridian angle $30^\circ \rightarrow \Theta^{NF} = \sqrt[3]{1 + d^{NF} \cdot (+1)}$; compr. mer. angle $-30^\circ \rightarrow \Theta^{CrF} = \sqrt[3]{1 + d^{CrF} \cdot (-1)}$.

Mode interaction \rightarrow Equation of the fracture body: $Eff = [(Eff^{NF})^m + (Eff^{SF})^m]^{m^{-1}} = 1 = 100\%$

$$Eff = \sqrt[m]{\left(c^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t}\right)^m + \left(c^{CrF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{CrF} - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^c}\right)^m} = 1.$$

Curve parameter relationships obtained by inserting the compressive strength point $(0, -\bar{R}^c, 0)$:

* Rotationally symmetric $\Theta = 1$: $d^{SF} = 0$, $c_{1\Theta}^{SF} = 1 + c_2^{SF}$

* 120°-rotat. symmetric $\Theta \neq 1$: $c_{1\Theta}^{SF} = 1 + c_2^{SF} \cdot \sqrt[3]{1 + d^{SF} \cdot (-1)}$, with

c^{NF} , Θ^{NF} from the two points $(\bar{R}^t, 0, 0)$ and $(\bar{R}^{tt}, \bar{R}^{tt}, 0)$ or by minimum error fit, if data available,

c^{CrF} , Θ^{CrF} from the two points $(-\bar{R}^c, 0, 0)$ and $(-\bar{R}^{cc}, -\bar{R}^{cc}, 0)$ or by minimum error fit.

The failure surface is closed at both the ends: A paraboloid serves as closing cap and bottom

$$\frac{I_1}{\sqrt{3} \cdot \bar{R}^t} = s^{cap} \cdot \left(\frac{\sqrt{2J_2 \cdot \Theta^{NF}}}{\bar{R}^t}\right)^2 + \frac{\max I_1}{\sqrt{3} \cdot \bar{R}^t} \quad , \quad \frac{I_1}{\sqrt{3} \cdot \bar{R}^t} = s^{bot} \cdot \left(\frac{\sqrt{2J_2 \cdot \Theta^{CrF}}}{\bar{R}^t}\right)^2 + \frac{\min I_1}{\sqrt{3} \cdot \bar{R}^t}$$

Slope parameters s are determined connecting the respective hydrostatic strength point with the associated point on the tensile and compressive meridian, $\max I_1$ must be assessed whereas $\min I_1$ can be measured. \bar{R}^t is normalization strength.

II : 3D-SFCs of (quasi-)Brittle Dense UD Materials

5 modes → 5 SFCs, is in line with ‘generic’ number according to the FMC. IFF1 generates a straight line in the stress plane!

$$\begin{aligned}
 \text{FF1: } Eff^{\parallel\sigma} &= \check{\sigma}_1 / \bar{R}_{\parallel}^t = \sigma_{eq}^{\parallel\sigma} / \bar{R}_{\parallel}^t \quad \text{with } \check{\sigma}_1 \cong \varepsilon_1^t \cdot E_{\parallel} \text{ (matrix neglected)} \\
 \text{FF2: } Eff^{\parallel\tau} &= -\check{\sigma}_1 / \bar{R}_{\parallel}^c = +\sigma_{eq}^{\parallel\tau} / \bar{R}_{\parallel}^c \quad \text{with } \check{\sigma}_1 \cong \varepsilon_1^c \cdot E_{\parallel} \\
 \text{IFF1: } Eff^{\perp\sigma} &= [(\sigma_2 + \sigma_3) + \sqrt{\sigma_2^2 - 2\sigma_2 \cdot \sigma_3 + \sigma_3^2 + 4\tau_{23}^2}] / 2\bar{R}_{\perp}^t = \sigma_{eq}^{\perp\sigma} / \bar{R}_{\perp}^t \\
 \text{IFF2: } Eff^{\perp\tau} &= [a_{\perp\perp} \cdot (\sigma_2 + \sigma_3) + b_{\perp\perp} \sqrt{\sigma_2^2 - 2\sigma_2 \sigma_3 + \sigma_3^2 + 4\tau_{23}^2}] / \bar{R}_{\perp}^c = \sigma_{eq}^{\perp\tau} / \bar{R}_{\perp}^c \\
 \text{IFF3: } Eff^{\perp\parallel} &= \{ [b_{\perp\parallel} \cdot I_{23-5} + (\sqrt{b_{\perp\parallel}^2 \cdot I_{23-5}^2 + 4 \cdot \bar{R}_{\perp\parallel}^2 \cdot (\tau_{31}^2 + \tau_{21}^2)^2}) / (2 \cdot \bar{R}_{\perp\parallel}^3)] \}^{0.5} = \sigma_{eq}^{\perp\parallel} / \bar{R}_{\perp\parallel} \\
 \{ \sigma_{eq}^{\text{mode}} \} &= (\sigma_{eq}^{\parallel\sigma}, \sigma_{eq}^{\parallel\tau}, \sigma_{eq}^{\perp\sigma}, \sigma_{eq}^{\perp\tau}, \sigma_{eq}^{\perp\parallel})^T, \quad I_{23-5} = 2\sigma_2 \cdot \tau_{21}^2 + 2\sigma_3 \cdot \tau_{31}^2 + 4\tau_{23}\tau_{31}\tau_{21} \\
 \text{Inserting the compressive strength point } (0, -\bar{R}_{\perp}^c) &\rightarrow a_{\perp\perp} \cong \mu_{\perp\perp} / (1 - \mu_{\perp\perp}), \quad b_{\perp\perp} = a_{\perp\perp} + 1 \\
 \text{from a measured fracture angle } \rightarrow \mu_{\perp\perp} &= \cos(2 \cdot \theta_{fp}^c \cdot \pi / 180), \quad \text{for } 50^\circ \rightarrow \mu = 0.174. \\
 b_{\perp\parallel} &\cong 2 \cdot \mu_{\perp\parallel}. \quad \text{Typical friction value ranges: } 0 < \mu_{\perp\parallel} < 0.25, \quad 0 < \mu_{\perp\perp} < 0.2. \\
 \text{Interaction Equation:} \\
 Eff^m &= [(\sigma_{eq}^{\parallel\sigma} / \bar{R}_{\parallel}^t)^m + (\sigma_{eq}^{\parallel\tau} / \bar{R}_{\parallel}^c)^m + (\sigma_{eq}^{\perp\sigma} / \bar{R}_{\perp}^t)^m + (\sigma_{eq}^{\perp\tau} / \bar{R}_{\perp}^c)^m + (\sigma_{eq}^{\perp\parallel} / \bar{R}_{\perp\parallel})^m] \\
 \text{or } Eff &= \sqrt[m]{(Eff^{\text{mode } 1})^m + (Eff^{\text{mode } 2})^m + \dots} = 1 = 100\% \quad \text{for Onset-of-Failure.}
 \end{aligned}$$

As abbreviation, $I_2 \cdot I_3 - I_5 = I_{23-5}$ is used. In the equations above, \bar{R} denotes an average = typical strength value that should be used for the stress-strain curves in stress and deformation analysis. In the design verification the statistically reduced strength values are applied. The superscripts t, c stand for tensile, compressive. The superscripts σ and τ mark the type of fracture failure whether it is caused by a tensile stress (Normal Fracture, NF, 'cleavage') or a shear stress (Shear Fracture, SF), e.g. due to a compressive normal stress σ_{\parallel}^c or a transverse normal stress σ_{\perp}^c .

Failure activated in two directions is considered by adding a multi-fold failure term, proposed in [Awa78] for isotropic materials. It can be applied to brittle UD material in the transversal (quasi-isotropic) plane as well.

For the 2D-case, a simplified friction modelling (IFF3) is possible:

$$\begin{aligned}
 Eff^m &= [(Eff^{\parallel\sigma})^m + (Eff^{\parallel\tau})^m + (Eff^{\perp\sigma})^m + (Eff^{\perp\tau})^m + (Eff^{\perp\parallel})^m] \text{ with the mode portions inserted, 2D,} \\
 Eff &= \left[\left(\frac{(\sigma_1 + |\sigma_1|)}{2 \cdot \bar{R}_{\parallel}^t} \right)^m + \left(\frac{(-\sigma_1 + |\sigma_1|)}{2 \cdot \bar{R}_{\parallel}^c} \right)^m + \left(\frac{(\sigma_2 + |\sigma_2|)}{2 \cdot \bar{R}_{\perp}^t} \right)^m + \left(\frac{(-\sigma_2 + |\sigma_2|)}{2 \cdot \bar{R}_{\perp}^c} \right)^m + \left(\frac{|\tau_{21}|}{\bar{R}_{\perp\parallel} + 0.5 \cdot \mu_{\perp\parallel} \cdot (-\sigma_2 + |\sigma_2|)} \right)^m \right]^{1/m}
 \end{aligned}$$

II b: 3D-SFCs of (quasi-)Brittle Porous UD Materials

This practically meets just IFF2. The table below shows the difference.

* IFF2 Failure Function for the dense UD material (for comparison)

$$\begin{aligned}
 F^{SF} &= [a_{\perp\perp} \cdot I_2 + b_{\perp\perp} \cdot \sqrt{I_4}] / \bar{R}_{\perp}^c = 1 \text{ with } a_{\perp\perp} = b_{\perp\perp} - 1 \text{ after inserting } \bar{R}_{\perp}^c \\
 &= [a_{\perp\perp} \cdot (\sigma_2 + \sigma_3) + b_{\perp\perp} \cdot \sqrt{(\sigma_2 - \sigma_3)^2 + 4\tau_{23}^2}] / \bar{R}_{\perp}^c = 1 \\
 &= [a_{\perp\perp} \cdot (\sigma_2^{pr} + \sigma_3^{pr}) + b_{\perp\perp} \cdot \sqrt{(\sigma_2^{pr} - \sigma_3^{pr})^2 + 0^2}] / \bar{R}_{\perp}^c = 1 \leftarrow 2 \text{ structural stresses}
 \end{aligned}$$

* IFF2 Failure Function for the porous UD material (index por, author's simple approach)

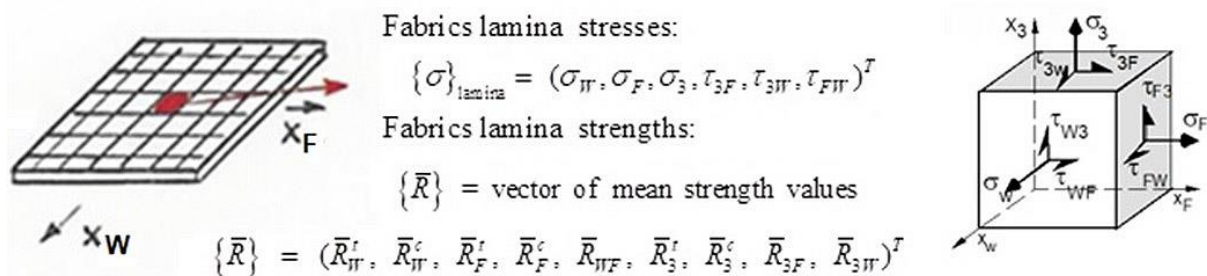
$$F_{porosity}^{SF} = \sqrt{a_{\perp\perp por}^2 \cdot I_2^2 + b_{\perp\perp por}^2 \cdot I_4 - a_{\perp\perp por} \cdot I_2} / 2\bar{R}_{\perp}^c = 1 .$$

The two curve parameters are determined - as before performed - from insertion of the compressive strength point and from the bi-axial fracture stress point.

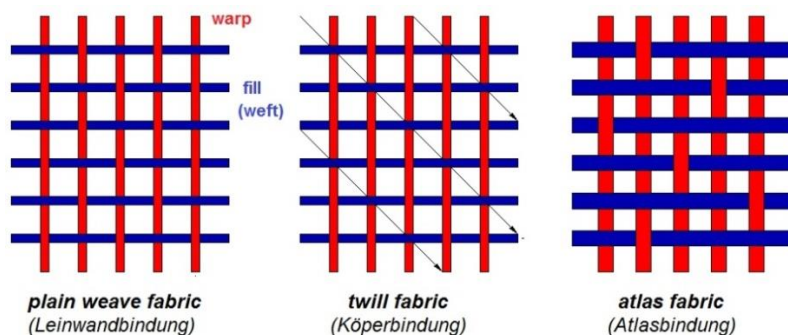
Mind: In contrast to an isotropic dense material the fracture body of a compressed dense UD-material has a closed bottom fracture surface, because the filaments may break under the tensile stress caused by biaxial compression due to the Poisson effect, when $\varepsilon_{axial}^t = \varepsilon_{||}^{fr}$.

III: 3D-SFCs of the Orthotropic Fabrics, (see [Cun24b])

9 modes → 9 SFCs. This is in line with Cuntze's 'generic' number 9 according to the FMC. In this context, my thanks to Roman (Prof. Dr. Keppeler, UniBw; formerly Siemens AG).



The following table includes the FMC-based SFCs for **porous orthotropic** (rhombic-anisotropic) **materials** composed for instance of 2D-woven fabrics. Three essential 2D-woven fabrics (Atlas or Satin) are depicted



$$Eff = \left(\frac{\sigma_W + |\sigma_W|}{2 \cdot \bar{R}_W^t} \right)^m + \left(-\frac{\sigma_W + |\sigma_W|}{2 \cdot \bar{R}_W^c} \right)^m + \left(\frac{\sigma_F + |\sigma_F|}{2 \cdot \bar{R}_F^t} \right)^m + \left(-\frac{\sigma_F + |\sigma_F|}{2 \cdot \bar{R}_F^c} \right)^m + \left(\frac{|\tau_{WF}|}{\bar{R}_{WF} - \mu_{WF} \cdot (\sigma_W + \sigma_F)} \right)^m$$

$$+ \left(\frac{\sigma_3 + |\sigma_3|}{2 \cdot \bar{R}_3^t} \right)^m + \left(-\frac{\sigma_3 + |\sigma_3|}{2 \cdot \bar{R}_3^c} \right)^m + \left(\frac{|\tau_{3W}|}{\bar{R}_{3W} - \mu_{3W} \sigma_3^c} \right)^m + \left(\frac{|\tau_{3F}|}{\bar{R}_{3F} - \mu_{3F} \sigma_3^c} \right)^m = 1 = 100\% .$$

For a cross-ply fabric with Warp = Fill $\rightarrow \bar{R}_W^t = \bar{R}_F^t, \bar{R}_W^c = \bar{R}_F^c$, the inter-laminar *Effs*, suffix ₃, vanish and just the in-plane (intra-laminar) *Effs* remain.

The range of parameters is for the interaction-exponent $2.5 < m < 2.9$, and since the strong porosity-dependency is very different \rightarrow recommendation: $\mu_{WF} < 0.2, \mu_3 < 0.2$.

If σ_F is also active, this double mode contributes via $\left(\frac{|\tau_{WF}|}{\bar{R}_{WF} - \mu_{WF} \cdot (\sigma_W^c + \sigma_F^c)} \right)^m$.

Modelling of laminates may be lamina-based (basic layers are UD layers), sub-laminate-based (semi-finished non-crimp orthotropic fabrics) or even laminate-based. Thereby, modelling complexity grows from UD, via non-crimp fabrics (NCF) through plain weave and finally to the spatial 3D-textile materials. Model parameters are just the measurable technical strengths R and the friction values μ , and on top the Weibull statistics-based interaction exponent m . The value of μ comes from mapping the compression stress-shear stress domain and of m by mapping the transition zone between the modes. A good guess is $m = 2.6$ for all mode transition domains and all material families. Model parameters are just the measurable technical strengths R and the friction values μ , and on top the Weibull statistics-based interaction exponent m . The value of μ comes from mapping the compression stress-shear stress domain and of m by mapping the transition zone between the modes. A good guess is $m = 2.6$ for all mode transition domains and all material families.

Validity Limits of UD SFC Application



Visiting the angle:

I am sure that this hose had to be replaced!

Experience:

The 4 m long caiman mother Maria observes the Limit “No trespassing (No pase!)”.

Maria stopped at the tape, marked with “No pase“!

That behavior was very good for the **personal** health of my friend Eddi
(he unfortunately fell in front of her snout while running away).



We learn from this:

*Engineers should always observe the limits set by specifications etc.
This is good for ‘**structural** health’ or Structural Integrity, respectively.*

And: This fully meets the SFC applications.

My present feeling considering the 3D-applications:

Much is reached with plenty of effort!

However, much more effort is required for the 3D-Validation. Test Data are missing.

Only when you get ‘higher’ the real ‘peaks’ do appear:

3D-applications are much more challenging.



8 Cracking at statically-loaded Notches using FM and Finite Fracture Mechanics (FFM)

Aim: Giving the user some advices regarding the so-called 'Open Hole Panels'.

Summary

Full Design Verification requires the verification of Strength and of Damage Tolerance in the case of potentially cracked (macro-damaged) statically-loaded structural components under sudden overloading.

The Strength Analysis (SA) requires that the effective multi-axial stress state is not above the given Strength Design Allowable and the Damage Tolerance Analysis (DTA) the same for the so-called residual strength of the structural component containing a pre-crack.

Lying between Strength analysis and Fracture Mechanics (FM) analysis 'Onset-of-Cracking' (OoC) is experienced at stress concentration sites such as notches like open holes in a panel of a sufficiently brittle material. In this context, Leguillon's Hypothesis [1] says

"A (generating) crack is (becomes) critical when and only when both the released energy and the local stress reach critical values along an assumed finite crack".

This novel hypothesis, 'Neuber'-improving, shall be presented here. It captures the prediction of the instantaneous OoC. The name of the tool is Finite Fracture Mechanics (FFM), see Fig.1. It predicts for notched components that loading level where the Strength Failure Criterion (SFC) equals the FM criterion or it determines as a coupled (hybrid) stress-energy criterion the critical loading that causes the finite crack size Δa_c . Because FM is one part of the FFM as introduction and for better understanding at first the well-known FM analysis tool R-curve shall be presented.

| smooth structure | notched structure | "transition domain" | cracked structure |
|------------------------------|--|--|--|
| no steep stress decay SFC | stress concentration Neuber method (up to now) | 'onset-of cracking' assumed crack, FFM (novel replacement) | stress intensity real pre-cracks, FM 'no hole' and 'with hole' |

Fig. 1: Stress situations in a structural component

Fig.2 visualizes the task to be solved. For practical application the concept of a linear-elastic stress intensity factor K may be sufficient and is usually applied. Coordinates used are depicted.

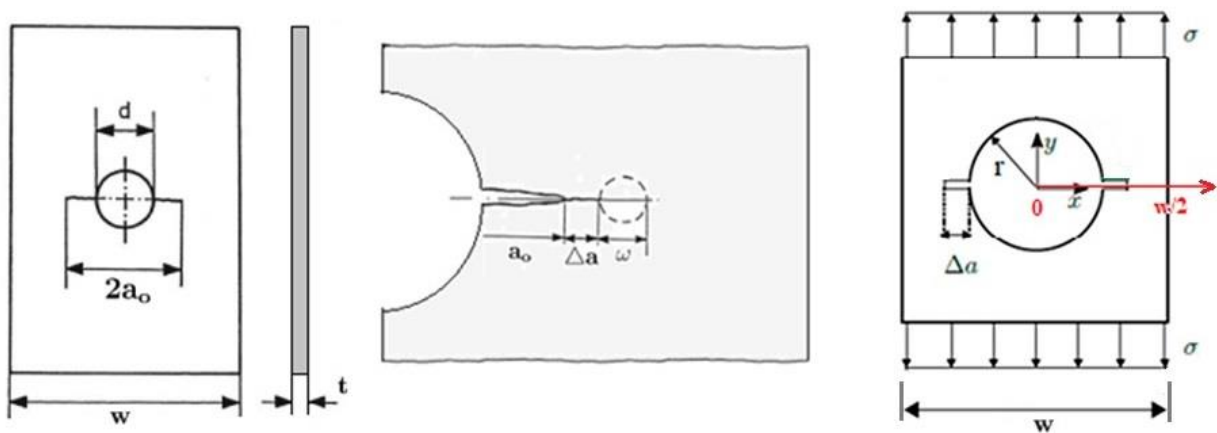


Fig. 1: Plate strip with a central open hole and an existing through crack of the size $a = a_0 - r$.

(left) characterization of an open hole panel with existing crack, w = plate width, t = plate thickness, (center) crack growth details in the case of slight crack tip yielding ω of not fully brittle materials, (right) σ = remote tensile stress, leading to cracking for $\sigma = \sigma_{fail}$, Δa = assumed FFM crack, $d = 2r$

Key Words: Residual strength, critical crack length, R-curve, Finite Fracture Mechanics, coupled criterion.

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(As-member of the IASB this document is structured similar to a HSB design sheet)

1 General

There are three approaches available to perform Design Verification (DV) for occurring static stress situations: Strength Failure Criteria (SFC), Continuum (micro-)Damage Mechanics (CDM, *not yet DV-capable*) criteria and Fracture Mechanics (FM) criteria for cracked (macro-damaged) components. A novel approach is the hybrid tool Finite Fracture Mechanics (FFM) which captures the ‘onset-of-cracking’ (OoC) at stress concentration (SC) points and at higher stress singularities.

The FFM is a coupled (hybrid) criterion that fills a gap in FM by assuming an instantaneous formation of a crack of finite size [1, 2]. Intention is to initially show the classical application of FM, because FM provides one part tool of the FFM. *Fig.1* gave a survey on the situations faced.

Due to FFM, the Neuber method is now obsolete, but falls as a special case. What Neuber called "support length" is precisely the crack length supplied by the FFM, without the need for acceptance or experimental identification!

The provided analyses are restricted to the 2D-case, 3D-extension will be a future task.

A SFC is a necessary condition but might not be a sufficient condition for the prediction of ‘Onset-of-cracking’, seen here as onset of failure:

*This is known for the author for about 50 years from the so-called ‘thin layer effect’ of UD-layer-composed laminates: *Due to being strain-controlled, the material flaws in a thin lamina (transversely-isotropic material) cannot grow freely up to micro-crack size in the thickness direction, because the neighboring laminas act as micro-crack-stoppers.* Considering fracture mechanics, the strain energy release rate, responsible for the development of damage energy in the 90° plies - from flaws into micro-cracks and larger -, increases with increasing ply thickness. Therefore, the actual absolute thickness of a lamina in a laminate is a driving parameter for initiation or onset of micro-cracks, i.e. [Fla82].

*Further and generally more known in metallic applications is in the case of discontinuities of the here focused isotropic materials such as notch singularities with steep stress decays: only a *toughness + characteristic length-based energy balance condition* may form a sufficient set of two fracture conditions.

When applying SFCs usually ideal solids are viewed which are assumed to be free of essential micro-voids or micro-crack-like flaws, whereas applying Fracture Mechanics the solid is considered to contain macro-voids or macro-cracks, respectively.

Since about 20 years Finite Fracture Mechanics (FFM) tries to fill a gap between the continuum mechanical strength analysis and the classical FM analysis. FFM is an approach to offer a criterion to predict the crack onset in brittle isotropic and UD materials.

This is a bridge that had to be built from the strength failure to the fracture mechanics failure ground. Attempts to link SFC-described ‘onset of fracture’ prediction methods and FM prediction methods for structural components have been performed. Best known is the still cited Hypothesis of Leguillon, within he assumes cracks of finite length Δa . Thus using FFM one obtains one more unknown but also a further equation to solve together with the SFC the equation system.

This coupled criterion does not refer to microscopic mechanisms to predict crack-*nucleation*!

Considering FFM it is referred to the literature [1, 2, 3]

2 List of Symbols

| Symbol | Unit | Description |
|-----------------|-------------------------|--|
| a | mm | crack length |
| a_0 | mm | initial crack length (open hole panel: crack a + hole radius r) |
| a_c | mm | <u>critical</u> crack length |
| a_e | mm | <u>effective crack length</u> $a_e = a_p + \omega/2$ |
| a_p | mm | <u>physical crack length</u> $a_p = a_0 + \Delta a$ |
| c_{ij} | | abbreviating functions and abbreviations |
| $f(a)$ | | correction function of the stress intensity factor (SIF) |
| f_d | | correction function concerning the hole diameter |
| f_w | | correction function concerning the specimen width w |
| t | mm | panel specimen thickness |
| w | mm | width of panel, test specimen |
| Δa | mm | stable increase of a due to static loading |
| Δa_e | mm | effective crack elongation (R-curve abscissa) $\Delta a_e = a - a_0$ |
| A | mm | parameter of the R-curve model |
| B | - | parameter of the R-curve model |
| E | MPa | Young's modulus (MPa = N/mm ²) |
| F | N | force |
| G_{Ic} | MPa · m | Cracking resistance: potential strain energy release rate at failure. Under plane strain conditions (most critical case) $G_{Ic} = K_{Ic}^2 \cdot (1 - \nu^2) / E$ $dW \sim \sigma_{\text{applied}}^2 \cdot \text{mm} / E \equiv \text{MPa} \cdot \text{mm}$, $\sqrt{\text{m}} = 31.6 \cdot \sqrt{\text{mm}}$ |
| $K(\sigma, a)$ | MPa · $\sqrt{\text{m}}$ | Cracking action: stress intensity factor, (SIF) $K = \sigma \cdot \sqrt{\pi \cdot a} \cdot f(a)$, |
| K_{as} | MPa · $\sqrt{\text{m}}$ | parameter of the R-Curve model (<u>as</u> ymptotic value of R-curve) |
| K_b | MPa · $\sqrt{\text{m}}$ | parameter of the R-Curve model (value at <u>b</u> eginning of R-curve) |
| K_{app} | MPa · $\sqrt{\text{m}}$ | apparent fracture toughness (general) = critical SIF (not the often used K_c) |
| K_p | MPa · $\sqrt{\text{m}}$ | physical value of the SIF K : $K_p = \sigma \cdot \sqrt{\pi \cdot a_p} \cdot \sqrt{\sec(\pi \cdot a_p / w)}$, $\sec = 1/\cos$ |
| K_e | MPa · $\sqrt{\text{m}}$ | <u>E</u> ffective SIF: $K_e = \sigma \cdot \sqrt{\pi \cdot a_e} \cdot \sqrt{\sec(\pi \cdot a_e / w)}$, often termed K_R |
| K_{Ic} | MPa · $\sqrt{\text{m}}$ | Cracking resistance: critical SIF (fracture mechanics Mode I testing) at onset of unstable sharp crack propagation in the plane strain state = most brittle condition, otherwise called K_c ; or = fracture toughness of uni-axially tensile-loaded, minimum ductile (<i>brittle</i>) material specimens = material resistance to crack propagation $K_{Ic} = \sigma \cdot \sqrt{\pi \cdot a_c} \cdot f = \sigma_c \cdot \sqrt{\pi \cdot a} \cdot f$ |
| K_R | MPa · $\sqrt{\text{m}}$ | Cracking resistance, R-curve ordinate |
| <u>R-curve</u> | MPa · $\sqrt{\text{m}}$ | material <u>Resistance</u> to fracture curve in case of slow, stable crack propagation from a sharp notch, accompanied by growth of the plastic zone at the crack-tip (<i>unfortunately also the letter R was taken</i>) |
| R ; R_{p02} | MPa | failure stress \equiv strength (<u>Resistance to stress action</u>); tensile yield str. |
| dW | MPa · mm | energy $dW \sim \int \sigma \cdot \varepsilon \cdot d\varepsilon = \int \sigma^2 / E \cdot d\varepsilon$ |
| ν | - | Poisson's ratio |
| ω | mm | full plastic zone at the crack-tip |
| σ | MPa | Action: remote (far field) uniform tensile stress |
| σ_c | MPa | critical value of σ = residual strength |

(In structural mechanics x is usually the length coordinate, but in fracture mechanics the net section direction)

3 Analysis using the Crack Growth Resistance curve = ‘R-curve’

3.1 General on Fracture Mechanics quantities and R-curve Concept

Basic assumption: Use of largest crack size that can be expected, following the ‘weakest link’ failure model and regarding quality assurance measurement limits.

In the Damage Tolerance procedure of cracked (macro-damaged) structural components two basic questions are posed in analysis:

1. What is the static strength if a crack is present (residual strength problem)?
2. How is the propagation behavior of the present crack (large crack growth problem)?

In order to perform this for isotropic materials some different quantities are used to predict the stress state at the crack tip caused by a far-field stress or remote stress, respectively.

*The stress intensity factor (SIF) K , applied to homogeneous linear elastic materials. Its measured size depends on test specimen width w , the crack size a , the location of the present crack and the material. It can be written as $K = \sigma \cdot \sqrt{\pi \cdot a_0} \cdot f(a/w)$, where the SIF K_I of the fracture mechanics mode I is applied here, (Fig. 4).

*The strain energy release rate \mathcal{G} , defined as the instantaneous loss of total potential energy Π per unit crack growth area (crack length $\Delta a \cdot$ plate thickness t) of the fresh surface S , by $\mathcal{G} = -\Pi / S$. In the case of brittle materials for its ‘basic’ Fracture Mode-I a relationship exists $\mathcal{G}_I = K_I^2 / E'$ with $E' = E / (1 - \nu^2)$ for plane strain.

*The J-integral J , characterizing the singular stress field at the crack tip in nonlinear elastic-plastic materials where the size of the plastic zone is small compared to the crack length. It is one way of determining the strain energy release rate \mathcal{G} . For brittle materials J corresponds to \mathcal{G} .

Macrocrack extension occurs when the stress intensity factor (SIF) K attains a critical value. Thereby the *Action-linked* SIF is entirely dependent on the structure geometry and loading condition, whilst the *Resistance-linked* R-curve is basically a material property dependent on temperature, environment, and loading rate as well the geometric test specimen range, etc.

Crack-growth resistance curves, the so-called R-Curves, are used here to predict:

- the residual strength of the structure for a given crack position and crack length,
- the critical length of an initial crack under given loadings.

These curves are conveniently plotted with crack extension Δa instead of crack size a , because the shape of the R-curve does not vary with the crack size.

* For very brittle materials with its flat R-curves, there is no stable crack extension and the initial crack size a_0 is the same as the critical crack size a_c . Then a single value of toughness characterizes the material, the cracking resistance K_{Ic} .

* For ductile materials (such as low strength steels) with a rising R-curve there is no single value of toughness that characterizes the material. Reason is that the plastic zone w at the crack tip increases with crack growth and length, hence the energy dissipated to overcome plastic deformation will increase. In materials with a rising R-curve, stable crack growth occurs and the critical crack size will be larger than the initial crack size. Mind: These R-curves (italic R letter) shall not be mixed up with the ‘R-curves’ in fatigue $R = \min\sigma / \max\sigma$.

Fracture mechanics regards small scale ductility (usually described by its diameter ω) at the crack tip and multi-axial loading, *Fig 2*.

In the case of a mixed-mode loading and opening of a crack, the energy release rate consists of the three parts \mathcal{G}_I , \mathcal{G}_{II} , \mathcal{G}_{III} that correspond to the respective three fracture modes. The fracture-effective formulation then is $\mathcal{G} = \mathcal{G}_I + \mathcal{G}_{II} + \mathcal{G}_{III}$.

Crack extension occurs when above strain energy release rate \mathcal{G} attains a critical value \mathcal{G}_c . In the case of fracture it becomes $\mathcal{G} \geq \mathcal{G}_c$. \mathcal{G} is directly related to the stress intensity factor K . It is associated in two-dimensional fracture mechanics with the loading modes (Mode-I, Mode-II, or Mode-III) the so-called Mixed-Mode Problem, applicable to cracks under plane stress, plane strain and anti-plane shear, see *Fig.4*. For the Fracture Mode-I, the energy release rate \mathcal{G} is related to the Mode-I stress SIF K_I for a linearly-elastic material.

The two questions at the beginning of this sub-chapter can be answered using the analytical methods of fracture mechanics. For practical application the concept of the linear-elastic K is usually applied:

“A structural component will fail in the case of static loading if the stress intensity factor (SIF) K of a brittle material reaches its critical value at $K = K_c$, termed fracture toughness, which depends on the material behavior”.

The determination of the K_c -values requires in the so-called K -concept the fulfilment of a geometric bound in order to achieve the real minimum K_{Ic} -value by a test specimen thickness of

$$t > 2.5 \cdot (K_{Ic} / R_{p0.2})^2 \rightarrow \sigma_c = K_{Ic} / (\sqrt{\pi \cdot a_0} \cdot f(a_0)).$$

Instead of the "Plain Strain Fracture Toughness" K_{Ic} (which is a material property but subject to certain minimum geometric requirements), an "Apparent Fracture Toughness" is inevitably to apply, adapted to the current geometric conditions.

A plot of strain energy release rate \mathcal{G} versus crack extension Δa for a particular loading situation is termed driving force curve $\mathcal{G}(\Delta a)$. The driving force for crack propagation can be quantified by above characterizing parameters K , \mathcal{G} , or J . A plot of R versus crack extension Δa is a resistance curve, as still cited termed R -curve $R(\Delta a)$.

3.2 Models for R -curve (resistance) and for Stress Intensity Factor (SIF)-curve

3.2.1 Resistance: R -curve, ordinate K_R (using a test data mapping function)

For well mapping the test data course of the R -curve J . Broede proposed the mapping function

$$K_e(a) = K_{as} - (K_{as} - K_b) \cdot \frac{1-B}{\exp\left(\frac{\Delta a_e}{A}\right) - B} \quad \text{with inverse} \quad \Delta a_e = A \cdot \ln\left(B + (1-B) \cdot \frac{K_{as} - K_b}{K_{as} - K_e}\right)$$

$$\Rightarrow \quad \text{new } a_0 = a_0 + \Delta a_e = a_0 - A \cdot \ln\left(B + (1-B) \cdot \frac{K_{as} - K_b}{K_{as} - K_e}\right)$$

in [2] including the effective quantities K_e and Δa_e . The plot $K_e(\Delta a_e)$ is termed effective R -curve.

3.2.2 Action: *Stress Intensity Factor (SIF)-curve, K_{SIF}* (using a width correction function f_w)

With the so-called geometry correction functions f - *correcting the original infinite plate term $\sqrt{\pi \cdot a}$* - concerning hole diameter (index d) and width (index w) of the centrally cracked panel ('plate strip') the SIF reads for the two cases:

Panel, version 'No hole' $_{nh}$:

$$K_{nh} = \sigma \cdot \sqrt{\pi \cdot a} \cdot f_w(a) \quad \text{with} \quad f_w(a) = \sqrt{\sec \frac{\pi \cdot a}{w}} \quad \text{capturing the panel width}$$

$$K_{nh}(a) = \sigma \cdot \sqrt{\pi \cdot a} \cdot \sqrt{\sec \frac{\pi \cdot a}{w}}, \quad (\sec = 1/\cos).$$

Panel, version 'With hole' $_{wh}$: (Tada delivered in [9] a hole considering correction function $f(a)$):

$$K = \sigma \cdot \sqrt{\pi \cdot a} \cdot f(a) \quad \text{with} \quad f(a) = f_d(a) \cdot f_w(a) \quad \text{in the case of an open hole panel}$$

$$f_d(a) = \sqrt{1 - \frac{r}{a}} \cdot (1 + 0.358 \cdot \frac{r}{a} + 1.425 \cdot \left(\frac{r}{a}\right)^2 - 1.578 \cdot \left(\frac{r}{a}\right)^3 + 2.156 \cdot \left(\frac{r}{a}\right)^4), \quad f_w(a) = \sqrt{\sec\left(\frac{\pi \cdot r}{w}\right) \cdot \sec\left(\frac{\pi \cdot a}{w}\right)}.$$

$$K_{wh}(a) = \sigma \cdot \sqrt{\pi \cdot a} \cdot \sqrt{1 - \frac{r}{a}} \cdot (1 + 0.358 \cdot \frac{r}{a} + 1.425 \cdot \left(\frac{r}{a}\right)^2 - 1.578 \cdot \left(\frac{r}{a}\right)^3 + 2.156 \cdot \left(\frac{r}{a}\right)^4) \cdot \sqrt{\sec \frac{\pi \cdot r}{w} \cdot \sec \frac{\pi \cdot a}{w}}.$$

3.3 Conditions to Determine the Unknowns: *critical quantities σ_c, a_{ce}*

'Crack growth will occur when $dG/da > dR/da$ and $G \geq R$ '.

This corresponds to *'The driving force curve is tangent with the R-curve'* as depicted in [Fig.3](#). It can be interpreted as the critical condition when the energy available in the component for crack growth exceeds the maximum amount that the material can dissipate. In order to solve this task the following conditions must be met:

3.3.1 $K_{SIF}(\sigma_c, a_{ce}) = K_e(a_{ce} - a_0)$ with $\Delta a_e = a - a_0$. This means, that

firstly the coordinates of the touch point of SIF curve with R-curve are to determine.

$$K_e(a) = \sigma \cdot \sqrt{\pi \cdot a} \cdot f(a) = K_{as} - (K_{as} - K_b) \cdot \frac{1 - B}{\exp\left(\frac{\Delta a_e}{A}\right) - B}.$$

3.3.2 $dK_{SIF}(\sigma_c, a_{ce})/da = dK_e(a_{ce} - a_0)/da$. This means, that

secondly, the two slopes of both the curves must become the same at the touch point, task which requires a differentiation (*Mathcad 15 code symbolic application*), delivering

$$\frac{dK_e}{da} \Rightarrow \sigma \cdot \frac{d}{da}(\sqrt{\pi \cdot a} \cdot f(a)) = K_{as} + \frac{(B-1) \cdot (K_b - K_{as})}{B - \exp\left(\frac{\Delta a_e}{A}\right)} \Leftarrow \frac{dK_R}{da}.$$

For the SIF-curve holds for the two versions, SIF_{nh} no hole and SIF_{wh} with hole:

$$\text{No hole' : } \frac{dK_{\text{SIFnh}}}{da} = \frac{d\left(\sigma \cdot \sqrt{\pi \cdot a} \cdot \sqrt{\sec \frac{\pi \cdot a}{w}}\right)}{da}$$

$$= \sigma \cdot \frac{\sqrt{\pi} / (2\sqrt{a} \cdot caw) + \pi^{1.5} \cdot \sqrt{a} \cdot saw / (w \cdot caw^2)}{2 \cdot \sqrt{\sqrt{\pi \cdot a} / caw}}, \quad saw = \sin\left(\frac{\pi \cdot a}{w}\right), caw = \cos\left(\frac{\pi \cdot a}{w}\right).$$

'With hole' :

$$\frac{dK_{\text{SIFwh}}}{da} = \frac{d\left(\sigma \cdot \sqrt{\pi \cdot a} \cdot \sqrt{1 - \frac{r}{a}} \cdot (1 + 0.358 \cdot \frac{r}{a} + 1.425 \cdot \left(\frac{r}{a}\right)^2 - 1.578 \cdot \left(\frac{r}{a}\right)^3 + 2.156 \cdot \left(\frac{r}{a}\right)^4) \cdot \sqrt{\sec \frac{\pi \cdot r}{w} \cdot \sec \frac{\pi \cdot a}{w}}\right)}{da}$$

$$= \sigma \cdot \sqrt{\pi \cdot a} \cdot \left[c3 \cdot \sqrt{1 - \frac{r}{a}} \cdot (c1) + \frac{c3 \cdot \sqrt{1 - \frac{r}{a}} \cdot (c2)}{2 \cdot a} + \frac{r \cdot c3 \cdot (c2)}{2 \cdot a^2 \cdot \sqrt{1 - \frac{r}{a}}} + \frac{\sin\left(\frac{\pi \cdot a}{w}\right) \cdot \sqrt{1 - \frac{r}{a}} \cdot (c2)}{\sqrt{a} \cdot 2 \cdot w \cdot \cos\left(\frac{\pi \cdot a}{w}\right)^2 \cdot \cos\left(\frac{\pi \cdot r}{w}\right) \cdot c3} \right]$$

and the abbreviation functions

$$c1 = \frac{4.734 \cdot r^3}{a^4} - \frac{8.624 \cdot r^4}{a^5} - \frac{0.358 \cdot r}{a^2} - \frac{2.85 \cdot r^2}{a^3}, \quad c2 = \frac{0.358 \cdot r}{a} + \frac{2.156 \cdot r^4}{a^4} + \frac{1.425 \cdot r^2}{a^2} - \frac{1.578 \cdot r^3}{a^3} + 1, \quad c3 = \sqrt{\frac{1}{\cos\left(\frac{\pi \cdot a}{w}\right) \cdot \cos\left(\frac{\pi \cdot r}{w}\right)}}$$

* In the HSB sheet 62232-3 *J. Broede* mapped the *R*-curve by an appropriate analytical model, model parameters were determined there and finally $\sigma_c = R_{res}$ was derived by iteratively increasing the crack size up to a_c . This provides the failure stress for the maximally sustainable loading of the pre-cracked component.

* In *Table 1*, bottom, Cuntze delivers a continuous implicit mathematical computation.

3.4 Solution of the equation set to predict the unknowns

The Mathcad computation delivers the searched quantities for the open hole panel. *Fig.3* provides the full data set. In the computation, the usually in $\text{MPa} \cdot \sqrt{\text{m}}$ given fracture toughness (= critical SIF) is taken, which however requires a final factorization of the obtained critical stress by $\sqrt{1000}$ to get into the MPa, mm system.

In *Fig.3*, for the envisaged panel, the *R*-curve is plotted together with two SIF-curves, one for an initially guessed reference stress of $\text{sig}_{wh}=15$ (**dashed**) and one for the computed critical reference value $\text{sig}_c=12.5$ (**bold**).

*For the 'no hole-panel' the critical SIF reads $K_c = 180 \text{ MPa} \cdot \sqrt{\text{m}} = 180 \cdot \sqrt{1000} \text{ MPa} \cdot \sqrt{\text{mm}}$ and the results are: $a_{ce} = 55.4 \text{ mm}$, $\sigma_c = 12.5 \cdot \sqrt{1000} = 396 \text{ MPa}$.

*For the 'hole panel', in order to check any influence of the hole the associated rising **SIF-curve** was plotted, too. The same tangent point is obtained for this SIF-curve.

The computation of the ‘no hole-panel’ delivers as critical stress = residual strength, the value $\sigma_{res} = 396$ MPa (Mathcad computation scheme in *Table 1*).

Table 1: Determination of the touch point = instability tangent point (w width effect, no hole)

Vorgabe $\text{sigw} = 11 \quad a = 33$

$$\text{sigw} \cdot \sqrt{\pi \cdot a} \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot a}{w}\right)} = K_{as} - (K_{as} - K_b) \cdot \frac{1 - B}{\exp\left(\frac{a - a_0}{A}\right) - B} \quad \text{(point)}$$

$$\frac{\pi \cdot \text{sigw} \cdot \left(\frac{\cos\left(\frac{\pi \cdot a}{w}\right) + \frac{\pi \cdot a}{w} \cdot \sin\left(\frac{\pi \cdot a}{w}\right)}{\cos\left(\frac{\pi \cdot a}{w}\right)^2} \right)}{2 \cdot \sqrt{\frac{a \pi}{\cos\left(\frac{\pi \cdot a}{w}\right)}}} = \frac{e^{\frac{a - a_0}{A}} \cdot (B - 1) \cdot (K_b - K_{as})}{A \cdot \left[B - e^{\frac{1}{A} \cdot (a - a_0)} \right]^2} \quad \text{(slope)}$$

$Aa := \text{Suchen}(\text{sigw}, a) \quad Aa = \begin{pmatrix} 12.5 \\ 55.4 \end{pmatrix} \quad \text{sigw} := Aa_0 \quad \text{sigw} = 12.5 \quad \text{awc} := Aa_1 \quad \text{awc} = 55.4$

$\sigma_{wc} := \text{sigw} \cdot \frac{\text{MPa} \cdot \sqrt{\text{m}}}{\sqrt{\text{mm}}} \quad \sigma_{wc} := \text{sigw} \cdot \sqrt{1000}$

$\text{KSIFwc0} := \text{sigwc} \cdot \sqrt{\pi \cdot \text{awc}} \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot \text{awc}}{w}\right)} \quad w = 300 \quad \text{KSIFwc0} = 180 \quad \sigma_{wc} = 396$

For information, however – no practical effect in *Fig.3* comparing the blue curve *KSIFwh* – the associated (point) condition with considering the hole is added below:

$$\text{sig} \cdot \sqrt{\pi \cdot a} \cdot \left(c_3 \cdot \sqrt{1 - \frac{r}{a}} \cdot c_1 + \frac{\sqrt{\pi} \cdot \text{sig} \cdot c_3 \cdot \sqrt{1 - \frac{r}{a}} \cdot c_2}{2 \cdot \sqrt{a}} + \frac{\sqrt{\pi} \cdot \text{sig} \cdot r \cdot c_3 \cdot c_2}{2 \cdot a^{1.5} \cdot \sqrt{1 - \frac{r}{a}}} + \frac{\pi^{1.5} \cdot \sqrt{a} \cdot \text{sig} \cdot \sin\left(\frac{\pi \cdot a}{w}\right) \cdot \sqrt{1 - \frac{r}{a}} \cdot c_2}{2 \cdot w \cdot \cos\left(\frac{\pi \cdot a}{w}\right)^2 \cdot \cos\left(\frac{\pi \cdot r}{w}\right) \cdot c_3} \right)$$

with the to be inserted abbreviation functions c_1, c_2, c_3

$$= K_{as} + \frac{(B - 1) \cdot (K_b - K_{as})}{B - e^{\frac{1}{A} \cdot (a - a_0)}}$$

The computation of the critical crack length a_c at the end of static loading is determined by the application of the formula below and there inserting K_{ec} (see application later). As K-values are usually given in $\text{MPa} \cdot \sqrt{\text{m}}$ this is intentionally widely followed here!

$$\text{new } a_0 = a_0 + \Delta a_{ec} = a_0 - A \cdot \ln \left(B + (1 - B) \cdot \frac{K_{as} - K_b}{K_{as} - K_{ec}} \right)$$

Results:

The R-test curve (resistance, marked KR) captures all physical effects such as small scale yielding at the crack tip, marked by the letter ω ! It is effective, therefore K_e . Therefore, in order to be compatible the SIF-curve (action, marked KSIF) has to incorporate this effect. It does not depend on a_0, w .

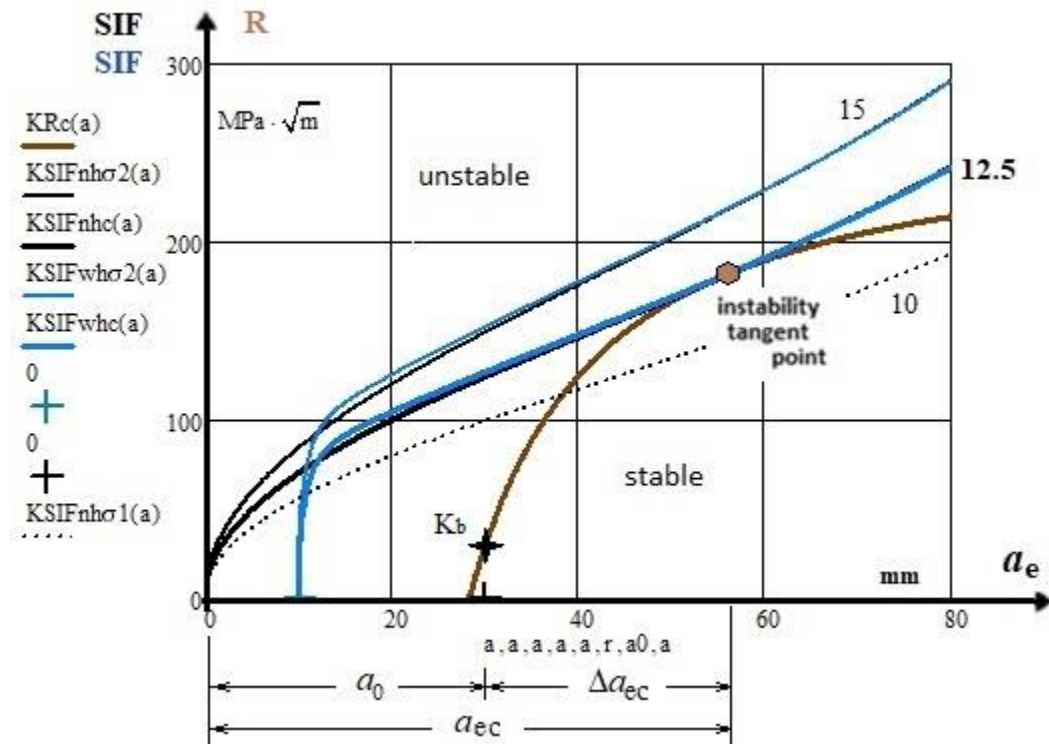


Fig.3: Wide panel example (HSB 62232-03) with $w = 300$ mm, $t = 8$ mm, $a_0 = 30$ mm, $d = 20$ mm.

Aluminum Alloy 7475-T7351 in LT-direction: $A = 55.7$ mm, $B = 0.75$, $K_{as} = 246$ MPa $\cdot \sqrt{\text{m}}$,

$K_b = 29$ MPa $\cdot \sqrt{\text{m}}$, $R_{p02} = 425$ MPa (B-value for $t = 6 \dots 38$ mm).

Instability point: $K_e = 180$ MPa $\cdot \sqrt{\text{m}} = \text{MPa} \cdot \sqrt{\text{mm}} \cdot \sqrt{1000}$, $a_{ce} = 55.4$ mm.

SIF-curve: reference stresses in MPa, to factor by $\sqrt{1000} = 31.6$: $\sigma_2 = 15 > \sigma_c = 12.5 > \sigma_1 = 10$, Table 1.

(For simplification the simple letter a was taken in the formulas instead of a_e)

Note:

The R-curve does not run out from a_0 . This is caused because just the test data domain has to be fitted best. In the HSB sheet this end is therefore not sketched. The model point K_b lies on the a_0 -line.

► The test-based R-curve is essential for FFM to determine in future a more correct fracture toughness value K_{app} instead of the previous K_{Ic} for the usually FFM-treated very brittle material.

4 Analysis using Finite Fracture Mechanics (FFM)

4.1 General

To prove Structural Integrity several design verifications (DVs) must be performed for components having the following features: Smooth, notched (stress concentrations) and cracked (stress singularities), see Fig.4, left. Thereby, static and cyclic loadings must be taken into account focusing uni-axial and multi-axial stress states.

FFM-focus here is static loading under uni-axial stresses, which means Mode I-linked.

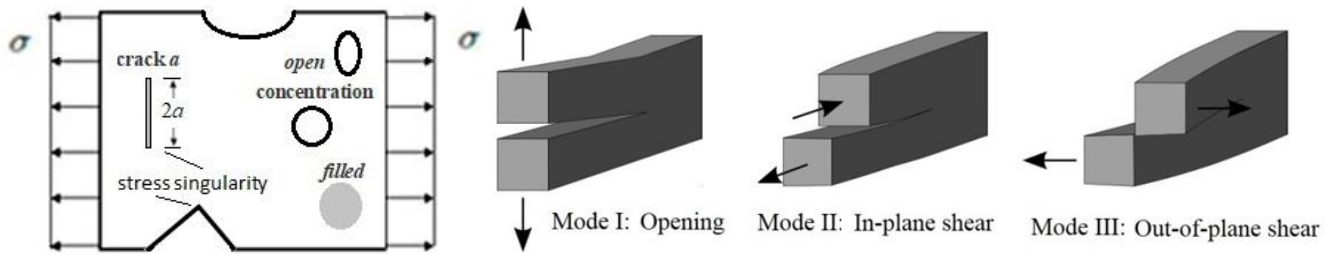


Fig. 4: (left) Stress concentrations and stress singularities under uni-axial stressing.
(right) The 3 FM-modes, crack length a

The following levels are relevant when generating stress-related DV tools:

1. Stresses: *Strength Failure Conditions (SFC)*, as local design verifications to predict onset-of-cracking (several strength fracture failure modes and one yield mode, practically just one for tension loading,)
2. Stress concentration: *Application of (local) stress concentration factors K_t* to predict onset-of-cracking (fracture) for the assessment of these internal discontinuity-caused probably locally infinite, singular stresses.
3. Stress intensity (singularity): *(non-local) Fracture mechanics methods using stress intensity factors $K \sim \sigma \cdot \sqrt{\pi \cdot a}$ (SIFs) and fracture toughness* (representing the resistance of brittle materials to the propagation of flaws under an activated stress, assuming: the longer the flaw, the lower the bearable fracture stress) *being a critical K_c which is needed for a crack to grow under monotonic loading. For the usually envisaged tension loading* (pressure-linked geo-mechanics is not the focus) *there are three fracture mechanics modes to consider as depicted in Fig.4 above.*

All design verifications are required in parallel in accordance with the applicable regulations.

Tackling above three structural cases, then it can be attributed:

1. Stresses: *In the strength fracture failure criterion (SFC) strength values R (isotropic: here $R^t \equiv R_m$) are to insert, which capture any flaws and micro-cracks in the material data set of the test specimen. All effects are considered.*
2. Stress concentrations: *Experience tells that the application of a SFC with the application of a factor K_t is not sufficient. Here, a non-local DV method is required, which combines a strength fracture criterion and fracture mechanics criterion. This is the focus of FFM.*
3. Stress intensity: *The necessary ('large') crack size value is identified by Quality Assurance or fixed as the minimum measurable crack size. The crack situation at hand is to model and toughness values K_{Ic} are to insert. A large crack analysis does not need a coupled DV in order to predict onset-of- further cracking, because the SFC is fulfilled.*

Note:

There are stress-related and strain-related SFCs. Stress-related ones have the advantage, compared to strain-related ones that "Residual stresses can be simply incorporated").

4.2 Introduction

Since about 20 years Finite Fracture Mechanics (FFM) intends to fill the gap between the continuum mechanical strength failure criteria (SFC) and the classical FM. FFM is an approach to offer a criterion to predict ‘Onset-of-Cracking’ in brittle isotropic and UD materials. This is a bridge that had to be built from strength failure to fracture mechanics failure. Attempts to link SFC-described ‘Onset-of-Cracking (OoC, fracture)’ prediction methods and FM prediction methods for structural components have been performed. Best known is the still cited Hypothesis of Leguillon “*A crack is critical when and only when both the released energy and the local stress reach critical values along an assumed finite crack*”. Within the FFM, Leguillon assumes instantaneous cracks of finite length Δa . Thus, using FFM one obtains one more unknown but also one more equation to solve together with the SFC the equation system.

Of the basic two previous FFM concept variants, the integral concept used here has proven to be the best. In this case, the stress curve is averaged over the fictitious, critical crack length for the SFC, i.e. converted into a locally evenly distributed stress curve averaged over this length.

As long as this is done over a comparatively small area, this is fine, but if it is a very large crack depth, where the crack extends far into an area of the stress profile where the stress peak has already been significantly reduced, the stress value averaged in this way becomes quite small. The question then is whether this procedure can still lead to a valid SFC application. In the future therefore, it would make sense to limit the range over which the stress curve is averaged appropriately in such cases?

This coupled criterion does not refer to microscopic mechanisms to predict micro-crack nucleation.

Reasons to develop the FFM were some facts from studying ‘Onset-of-Cracking’:

- Isotropic material

The minor failure behavior of absolutely small holes compared to large holes, although the stress concentration factor K_t takes the same value, namely 3. With large holes, more material volume is highly stressed and thus physically-based the probability of failure due to more activated, material-inherent flaws is increased.

Further known is in the case of discontinuities such as notch singularities with steep stress decays: only a *toughness + characteristic length-based energy balance condition* may form a sufficient set of two fracture conditions. Hence, a SFC is a necessary condition but might not be a sufficient condition for the prediction of ‘Onset-of-Cracking’.

When applying SFCs usually ideal solids are viewed which are assumed to be free of essential micro-voids or microcrack-like flaws, whereas applying Fracture Mechanics tools the solid is considered to contain macro-voids or macro-cracks.

- Transversely-isotropic material

It is also known for a long time from the so-called ‘Thin layer effect’ of UD-layer-composed laminate that the SFC-application is not sufficient to understand failure: *Due to being strain-controlled, the material flaws in a thin lamina cannot grow freely up to micro-crack size in the thickness direction, because the neighboring laminas act as micro-crack-stoppers*. In other words: Thin plies, embedded in a laminate, fail at a higher loading level than thick ones.

Employing here fracture mechanics, the strain energy release rate, responsible for the development of damage energy in the 90° plies - *from flaws into micro-cracks and larger cracks* -, increases with increasing ply thickness. Therefore, the actual absolute thickness of a lamina in a laminate is a driving parameter for initiation of cracks, i.e. [Fla82].

4.4 FFM modelling, isotropic material focused

The FFM concept is demonstrated here by the example “Uni-axially loaded symmetric open-hole plate strip”. For this case, the coupled criterion can be simplified and can be analytically solved. Thereby no initial crack a_0 is to treat. Brittle fracture behavior is presumed.

The energy criterion postulates that the critical energy release rate $G_{Ic} = K_{Ic}^2 \cdot (1 - \nu^2) / E$, being proportional to the square of the fracture toughness, is met and that the stress criterion = SFC postulates that the concentrated stress within the net-section area, *averaged along the crack length* Δa , reaches a material strength value. This averaging is an assumption, which should to be checked. Whereas the FM is more concerned about the full net section width, in the FFM the concern is basically just the net section length Δa , a portion of the width!

The coupled FFM criterion

Goal of the coupled FFM criterion is to derive two fracture conditions, a strength R -related one and a fracture mechanical one assuming a crack of the size Δa . Finally the two conditions are equated and deliver an equation for the unknown critical crack Δa_c being the crack level at which OoC would occur under a critical stress and fracture mechanical condition, simultaneously.

The establishment of the coupled model is to perform on basis of average properties in order to obtain the optimally achievable reliability of 50 %. This means model validation, whereas in the DV statistically based Design Allowables are to apply.

The two parts of the coupled criterion can be expressed by equalities from a Fracture Mechanics (FM) criterion and a Strength Failure Criterion (SFC):

$$\text{FM: } \frac{1}{\Delta a} \cdot \int_r^a K_I^2(a) \cdot da = K_{Ic}^2 \quad \text{and} \quad \text{SFC: } \frac{1}{\Delta a} \cdot \int_r^a \sigma(x, y=0) \cdot dx = R_m.$$

For a simpler comparison, for the SFC the square usually is taken, whereby – advantageously - the remote stress σ cancels out in the coupled equation. Fracture failure occurs if both these criteria are simultaneously fulfilled. This leads to the required equation for the determination of

the generated critical crack size Δa_c via

$$\frac{\frac{1}{\Delta a} \cdot \int_r^a K_I^2(x) \cdot dx}{\left(\frac{1}{\Delta a} \cdot \int_r^a \sigma_y(x) \cdot dx \right)^2} = \frac{K_{Ic}^2}{(R_m)^2} = c_{KR}.$$

Later, the author will use the upper single versions, because this better displays the parallel working of FM-condition together with the SF-condition.

As the two required resistance quantities are not fully clear and not given, it is sufficient for the following first numerical application of the FFM to apply the available values ‘*Plain Strain Fracture*

Toughness' K_{Ic} (the inherent lowest material property, subject to certain minimum geometric test specimens requirements to achieve a plain strain condition), and tensile strength R_m . This will mean the application to a brittle metal. In general, the real critical fracture toughness should be termed 'Apparent Fracture Toughness' K_{app} (to be understood as a component property, adapted to the current geometrical conditions). For K_{app} seldom a value is available. Hence, K_{Ic} will be used for the FFM here, despite of the necessity to consider small scale yielding at the crack tip when using structural metal materials, like shown in the chapter R-curve.

Validation of the FFM model is effort-fully to be performed by running isotropic test series for different w/d-ratios of panels.

5 Design Verification of a 'Through center cracked Open hole Panel'

Presumptions and given data for geometry, loading from testing

Presumptions:

- Linear Structural Analysis permitted
- Not fully brittle materials which generate small scale yielding at the crack tip
- Worst case loading situation, no residual stresses.

Material resistance: Aluminum alloy 7475-T7351 in L(ength)-T(ransverse) direction, example from [3]

- R-curve: $A = 55.7$ mm, $B = 0.75$, $K_{as} = 246$ MPa $\cdot \sqrt{\text{m}}$, $K_b = 29$ MPa $\cdot \sqrt{\text{m}}$. $R_m = 850$ MPa
- Yield strength: $R_{p02} = 425$ MPa (B-value, for $t = 6...38$ mm), HSB 62232-03. Concluding the 445 MPa, as used in HSB 62232-01, can be seen an average value.
- $K_{Ic} = 48$ MPa $\cdot \sqrt{\text{m}} = 1518$ MPa $\cdot \sqrt{\text{mm}}$, $(K_{Ic} / R_m)^2 = 3.23$ mm⁻¹.

Panel dimensions

- Width $w = 300$ mm, thickness $t = 8$ mm, open hole radius $d = 25$ mm
- Initial crack size $a_0 = 30$ mm.

Loading Action with Design Factor of Safety (FoS)

- $j=1$, Design Limit Load representative
- Uni-axial stress state $\{\sigma\}_{\text{design}} = \{\sigma_L\} \cdot j$ with $\{\sigma\}_L = (\sigma_x, \sigma_y, \tau_{xy})^T \cdot j = (0, 250, 0)^T$ MPa.

5.1 Application of FM, R-curve, concerning 'Open hole panel fracture', pre-crack a_0

See Fig.3 with the procedure attached. $a_0 = 30$ mm, $d = 25$ mm, $w = 300$ mm.

Design case: Remote loading stress $\sigma_{\text{design}} = 250$ MPa $\equiv \sigma_1$.

5.1.1 Determination of the residual strength [HSB 62232-03] with the R-curve

The computation in Table 1 delivers the following values in the instability point (touch point)

FM-resistance: $K_{ec} = 180$ MPa $\cdot \sqrt{\text{m}} = 180 \cdot \sqrt{1000}$ MPa $\cdot \sqrt{\text{mm}}$, proof in Fig.3

and further residual strength $\sigma_c = 396$ MPa and critical crack length $a_{ec} = 55$ mm.

Above remote failure stress = structural residual strength of the panel (plate strip) reads

$$\sigma_{\text{fail}} = R_{\text{res}} = R_{\text{struct}} = \sigma_c .$$

For comparison, the following analyses deliver the satisfactory information:

- * Stress concentration: $\sigma_{\text{fail}} = R_m / K_t (d = \infty) = 850 / 3 = 283 \text{ MPa} > \sigma_I = 250 \text{ MPa}$.
- * Fracture Mechanics: for a Quality Assurance-defined crack size such as $a_{\text{defined}} = 33 \text{ mm}$,
 $\sigma_{\text{fail}}(33) > \sigma_{\text{fail}}(55)$.

► Computation of the Reserve Factor for Design Limit load level, Design Load case $j=1$

Linear analysis is sufficient (presumption of FFM model at hand): then $\sigma \sim \text{load}$.

$$RF = \frac{\text{Structural strength Design Allowable } R_{\text{struct}}}{\text{Stress } \sigma \text{ at } j \cdot \text{Design Limit Loading}} = \frac{R_{\text{struct}}}{\sigma_{\text{design}}} = \frac{396}{250} = 1.58 > 1.$$

According to the regulations, R_{struct} has to be a Design Allowable too, which is assumed here due to R_m being a strength Design Allowable and K_{Ic} being statistically-based, too.

Yielding Check in the net-section: as a limit-of-usage check. One obtains:

$$\sigma_{\text{fail}} = \sigma_{\text{netyield}} = R_{p02} \cdot \left(1 - \frac{2 \cdot a_{ec}}{w}\right) = 425 \cdot \left(1 - \frac{2 \cdot 55}{300}\right) = 267 \text{ MPa} \rightarrow RF = \frac{267}{250} = 1.14 > 1.$$

Result: Due to the requirement $\sigma_{\text{netyield}} < \sigma_c$ net section yielding limits the loading here.

5.1.2 Determination of the critical crack length, touch point, considering ‘no hole, ‘with hole’

In the effective curve (*index e is written*) defined by $K_{ec} = 180 \text{ MPa} \cdot \sqrt{\text{m}}$ the plastic zone ω and the hole diameter are included.

The computation of the critical data set had to be still performed for the establishment of Fig.3.

► Computation of the design stress-linked Touch Point + generated crack growth $\Delta a_{\text{design loading}}$

Employing both the SIF functions from § 3.3.1

$$KSIF_{nh}(a) = \sigma \cdot \sqrt{\pi \cdot a} \cdot \sqrt{\sec \frac{\pi \cdot a}{w}} , \quad \text{and}$$

$$KSIF_{wh}(a) = \sigma \cdot \sqrt{\pi \cdot a} \cdot \sqrt{1 - \frac{r}{a}} \cdot \left(1 + 0.358 \cdot \frac{r}{a} + 1.425 \cdot \left(\frac{r}{a}\right)^2 - 1.578 \cdot \left(\frac{r}{a}\right)^3 + 2.156 \cdot \left(\frac{r}{a}\right)^4\right) \cdot \sqrt{\sec \frac{\pi \cdot r}{w} \cdot \sec \frac{\pi \cdot a}{w}}$$

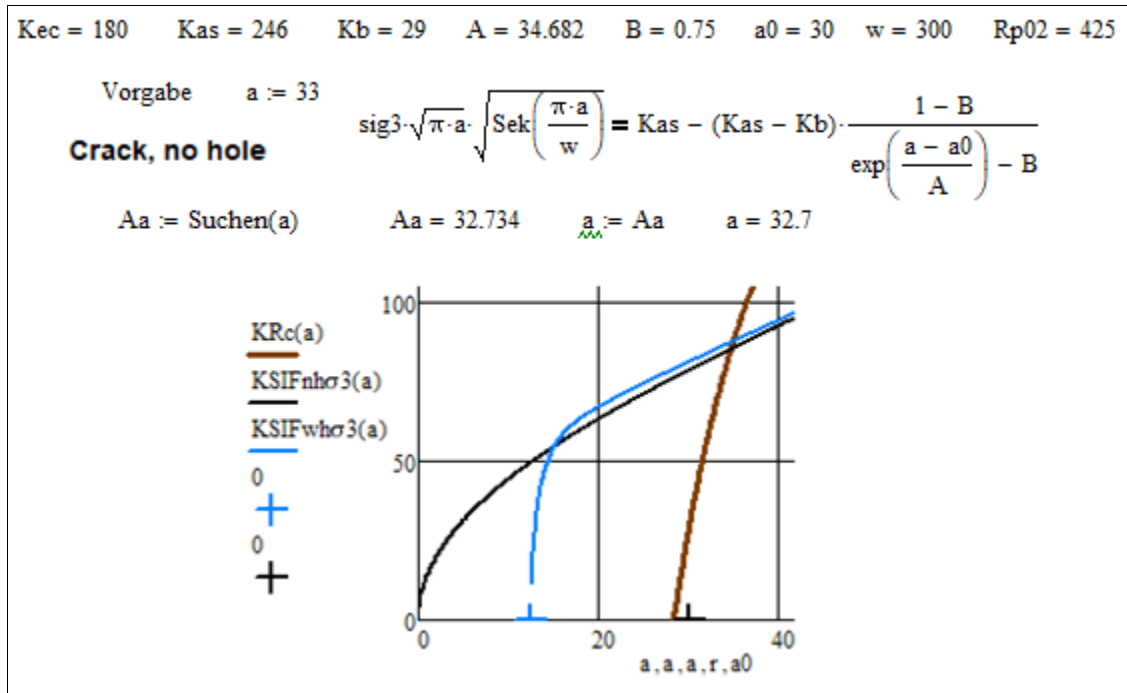
the Mathcad computation in Table 2 was executed. (See [3]).

Results for $\sigma_{\text{design}} = 250 \text{ MPa} = sig3$:

The crack grew under the design stress by $\Delta a_{\text{design loading}} = 3 \text{ mm}$.

$$\rightarrow \text{new } a_0 = a_0 + \Delta a_{\text{design loading}} = 30 + 3 = 33 \text{ mm.}$$

Table 2 Derivation of a ductility-considering SIF K with improved associate crack a



Additional information: Determination of the (physical) K_p from the effective values a_e

There are two methods to determine data of a R-curve The Potential method is used to determine physical data and the Compliance-Method (*applied here*) effective data for the given initial crack length a_0 and the loading stress σ [13, 12, 11].

If necessary, physical data can be derived from effective data by inserting

$$a_p = a_e - 0.5 \cdot \omega, \quad \omega = \frac{1}{\pi} \cdot \left(\frac{K_p}{R_{p0.2}} \right)^2 \quad \text{into} \quad K_p = \sigma \cdot \sqrt{\pi \cdot a_p} \cdot \sqrt{\sec\left(\pi \cdot a_p / w\right)}$$

implicit equation via

Vorgabe $K_p := 180$

$$K_p = \sigma \cdot \sqrt{\pi \cdot \left[a_e - 0.5 \cdot \left[\frac{1}{\pi} \cdot \left(\frac{K_p}{R_{p0.2}} \right)^2 \right] \right]} \cdot \sqrt{\frac{1}{\cos\left[\pi \cdot \frac{a_e - 0.5 \cdot \left[\frac{1}{\pi} \cdot \left(\frac{K_p}{R_{p0.2}} \right)^2 \right]}{w}\right]}}$$

D := Suchen(K_p)

Whether this might be important could be checked by inserting K_{pc} through K_{ec} calculating

$$\omega_{K_{ec}} = \frac{1}{\pi} \cdot \left(\frac{K_{ec}}{R_{p0.2}} \right)^2$$

In order to present a good feeling for the difference between K_p and K_e the respective values shall be computed below for the critical case, indexed c :

$$\begin{aligned}
 \sigma_{fail} &:= 12.5 & \sigma &:= \sigma_{fail} \cdot \sqrt{1000} & \sigma &= 395 & R_{p02} &:= 425 & w &:= 300 & a_0 &:= 30 & K_{ec} &:= 180 \cdot (\sqrt{1000}) \\
 a_{ec} &:= 55.4 & \Delta a_{ec} &:= a_{ec} - a_0 & \Delta a_{ec} &= 25.4 \text{ mm} & K_{ec} &= 5692 \\
 \text{Vorgabe} & & K_{pc} &:= 5555 \\
 K_{pc} &= \sigma \cdot \sqrt{\pi \cdot \left[a_{ec} - 0.5 \cdot \left[\frac{1}{\pi} \cdot \left(\frac{K_{pc}}{R_{p02}} \right)^2 \right] \right]} \cdot \frac{1}{\sqrt{\cos \left[\pi \cdot \frac{a_{ec} - 0.5 \cdot \left[\frac{1}{\pi} \cdot \left(\frac{K_{pc}}{R_{p02}} \right)^2 \right]}{w} \right]}} \\
 D &:= \text{Suchen}(K_{pc}) & D &= 4479 \\
 \omega_{ec} &:= 0.5 \cdot \left[\frac{1}{\pi} \cdot \left(\frac{K_{ec}}{R_{p02}} \right)^2 \right] & \omega_{pc} &:= 0.5 \cdot \left[\frac{1}{\pi} \cdot \left(\frac{K_{pc}}{R_{p02}} \right)^2 \right] & a_{pc} &:= a_{ec} - 0.5 \cdot (\omega_{ec}) \\
 \omega_{ec} &= 28.5 & \omega_{pc} &= 27.2 \text{ mm} & a_{pc} &= 41.1
 \end{aligned}$$

5.2 Application of FFM, concerning 'Onset-of-Cracking' at a open hole edge, (no a_0)

Determination of finite crack length Δa and failure stress of the panel: Mathcad 15 application

In this sub-chapter the 'classical' FFM-procedure with the square will be presented.

- The FM-linked failure portion: The equation reads:

$$\begin{aligned}
 \frac{1}{\Delta a} \cdot \int_r^a [K_I^2(a)] \cdot dx = \\
 \frac{1}{\Delta a} \cdot \int_r^a \left(\sigma \cdot \sqrt{\pi \cdot a} \cdot \sqrt{1 - \frac{r}{a}} \cdot \left(1 + 0.358 \cdot \frac{r}{a} + 1.425 \cdot \left(\frac{r}{a} \right)^2 - 1.578 \cdot \left(\frac{r}{a} \right)^3 + 2.156 \cdot \left(\frac{r}{a} \right)^4 \cdot \sqrt{\sec \frac{\pi \cdot r}{w} \cdot \sec \frac{\pi \cdot a}{w}} \right)^2 \cdot dx
 \end{aligned}$$

- The SFC-linked failure portion: For details see Annex I

For this portion a model for the stress distribution along the net section is to provide, namely,

$$\sigma_{netsec}(x) = \sigma \cdot c_{wd} \cdot \left[0.335 + 0.665 \cdot \left(1 + c_{11} \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c_{12}} + c_{13} \cdot \left(\frac{x-r}{0.5 \cdot w - r} \right)^4 \right], \quad [8]$$

with the abbreviation functions $c_{wd} = 3.215 - \left(\frac{w}{d} \right)^{-0.5} + 4.294 \cdot \left(\frac{w}{d} \right)^{-1.5}$ and

$$c_{11} = -3.765 + 2.148 \cdot \left(\frac{w}{d} \right)^{0.879}, \quad c_{12} = -2.552 - 42.894 \cdot \left(\frac{w}{d} \right)^{-3.17}, \quad c_{13} = -0.7497 \cdot \left(\frac{w}{d} \right)^{-1.858}.$$

The equilibrium equation of the SFC-portion reads

$$\begin{aligned}
 \frac{1}{\Delta a} \cdot \int_r^{r+\Delta a} \sigma(x, y=0) \cdot dx &= \frac{1}{\Delta a} \cdot \int_r^{r+\Delta a} \sigma_{netsec}(x) \cdot dx = \\
 \frac{1}{\Delta a} \cdot \int_r^{r+\Delta a} \sigma \cdot c_{wd} \cdot \left[0.335 + 0.665 \cdot \left(1 + c_{11} \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c_{12}} + c_{13} \cdot \left(\frac{x-r}{0.5 \cdot w - r} \right)^4 \right] \cdot dx
 \end{aligned}$$

The implicit FFM-solution procedure of the Mathcad software in standard FFM-formulation is shown below:

Vorgabe $\Delta a := 1$ $\sigma := 11$

$$\left[\frac{1}{\Delta a} \int_r^{r+\Delta a} \left[\sigma \cdot \sqrt{\pi \cdot x} \cdot \left[\sqrt{1 - \frac{r}{x}} \cdot \left[1 + 0.358 \cdot \frac{r}{x} + 1.425 \cdot \left(\frac{r}{x}\right)^2 - 1.578 \cdot \left(\frac{r}{x}\right)^3 + 2.156 \cdot \left(\frac{r}{x}\right)^4 \right] \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot r}{w}\right) \cdot \text{Sek}\left(\frac{\pi \cdot x}{w}\right)} \right] \right]^2 dx \right] = \frac{K_{Ic}^2}{R_m}$$

$$\frac{1}{\Delta a} \int_r^{r+\Delta a} \sigma \cdot crw \cdot \left[0.335 + 0.665 \cdot \left(1 + c11 \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c12} + c13 \cdot \left(\frac{x-r}{0.5 \cdot w - r}\right)^4 \right] dx$$

$$\frac{1}{\Delta a} \int_r^{r+\Delta a} \sigma \cdot crw \cdot \left[0.335 + 0.665 \cdot \left(1 + c11 \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c12} + c13 \cdot \left(\frac{x-r}{0.5 \cdot w - r}\right)^4 \right] dx = R_m$$

A := Suchen(Δa, σ) A = $\begin{pmatrix} 1.77 \\ 420.83 \end{pmatrix}$ $\Delta a := A_0$ $\Delta a_c := \Delta a$ $\sigma := A_1$ $\sigma_{fail} := \sigma$ $\Delta a_c = 1.77$ $\sigma_{fail} = 421$

Results:

Within the FFM, two models from FM and from strength analysis are commonly employed to predict the failure event ‘Onset-of-Cracking’ at a non-cracked hole. In the case at hand, the instantaneously generated finite crack length reads $\Delta a_c = 1.77$ mm and the associated remote average structural failure stress of the panel σ_{struc} reads $\sigma_{fail} = 421$ MPa.

Fig.5 finally tries to illustrate the FFM hypothesis “Both the conditions must be fulfilled”. It points out the failure-causing relationship and the dominated domains, where stress states may happen to be.

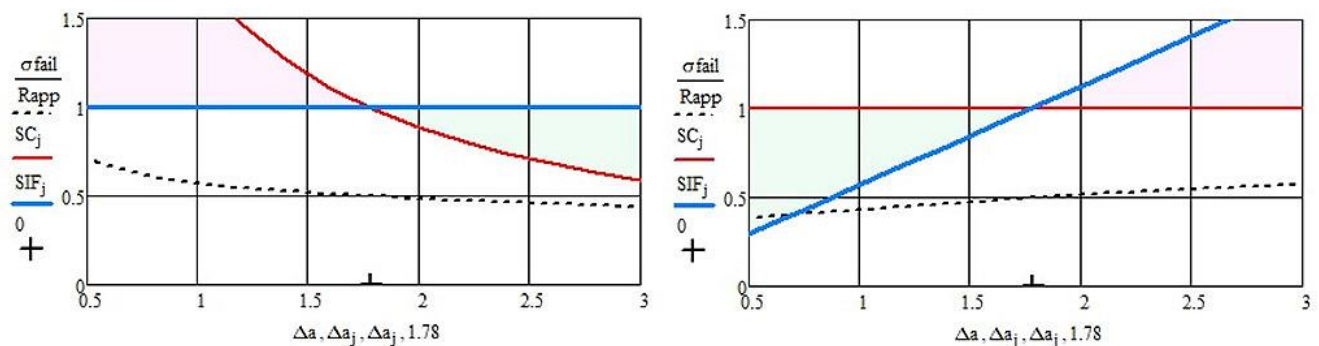


Fig.5: (left) ‘SIF’ assumed 100% with the question “When does the SC not show failure?”
 Vice versa: (right) SC assumed 100% with the question “When does the ‘SIF’ not show failure?”

One basic interest is how a varying resistance ratio $c_{KR} = K_{Ic}^2/R_m^2$ affects critical **crack length** and **failure stress**. Fig.6 shows the mapped numerical results for a number of ratios.

Result: With increasing resistance ratio both critical crack size and failure stress naturally grow.

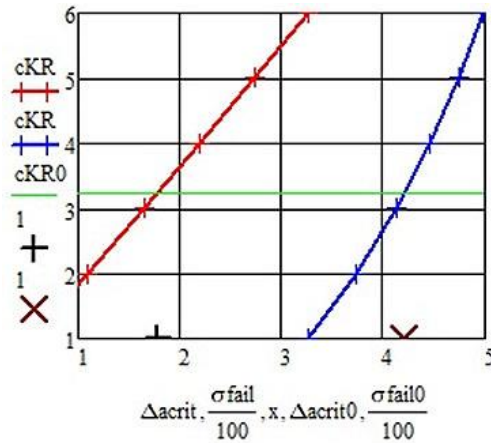


Fig.6, $w=36\text{mm}$, $a_0 = 30\text{ mm}$, $d = 6\text{ mm}$: Effect of varying resistance ratio c_{KR} on Δa_c and σ_{fail} .

$$AA\ 7475\text{-}T7351: c_{KR0} = (K_{Ic} / R_m)^2 = 3.23\ \text{mm}^{-1}$$

Of further interest might be how the FM-linked and the SC-linked portions change with the crack length. Fig.7 depicts these courses after employing the two integrals, termed ‘SIF’ and SC, below.

$$\frac{\frac{1}{\Delta a} \cdot \int_r^a K_I^2(x) \cdot dx}{\left(\frac{1}{\Delta a} \cdot \int_r^a \sigma_y(x) \cdot dx\right)^2} = \frac{K_{Ic}^2}{(R_m)^2} = c_{KR} \Rightarrow \frac{\frac{1}{\Delta a} \cdot \int_r^a K_I^2(x) \cdot dx / K_{Ic}^2}{\left(\frac{1}{\Delta a} \cdot \int_r^a \sigma_y(x) \cdot dx\right)^2 / R_m^2} = \frac{\text{'SIF'}}{\text{SC}} \quad \text{with}$$

$$SIF_i := \frac{\frac{1}{\Delta a_i} \int_r^{r+\Delta a_i} \left[\sigma \cdot \sqrt{\pi \cdot x} \cdot \left[\sqrt{1 - \frac{r}{x}} \left[1 + 0.358 \cdot \frac{r}{x} + 1.425 \cdot \left(\frac{r}{x}\right)^2 - 1.578 \cdot \left(\frac{r}{x}\right)^3 + 2.156 \cdot \left(\frac{r}{x}\right)^4 \right] \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot r}{w}\right) \cdot \text{Sek}\left(\frac{\pi \cdot x}{w}\right)} \right]^2 dx}{K_{Ic}^2}$$

$$SC_i := \frac{1}{(\Delta a_i)^2} \int_r^{r+\Delta a_i} \sigma \cdot crw \cdot \left[0.335 + 0.665 \cdot \left(1 + c11 \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c12} + c13 \cdot \left(\frac{x-r}{0.5 \cdot w - r} \right)^4 \right]^2 dx \Bigg/ R_m^2$$

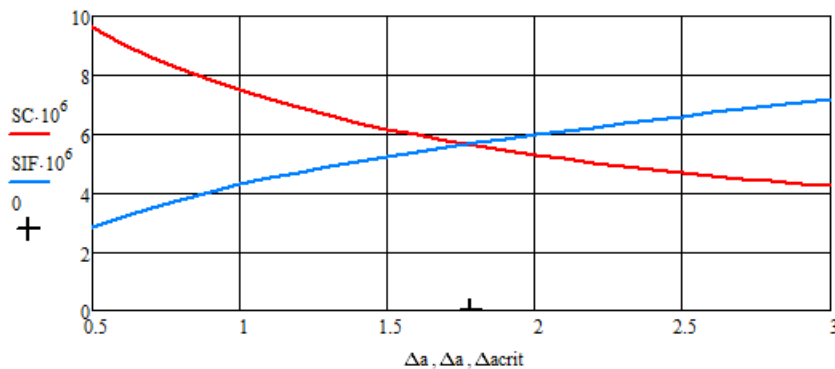


Fig.7, $w = 36\text{mm}$, $a_0 = 30\text{ mm}$, $d = 6\text{ mm}$: Representation of the course of the growing FM-portion (SIF) and the decaying Strength Mechanics portion (SC)

Result: The critical point at $a_c = 1.78\text{ mm}$ is clearly outlined at 'SIF' = SC.

After having depicted the influence of the resistance ratio $c_{KR} = K_{Ic}^2/R_m^2$ in Fig.6 the effect of a fixed ratio 'panel width/hole diameter' w/d shall be displayed for two widths in Fig.8 presenting how the remote failure stress σ_{fail} of the panel changes with Δa .

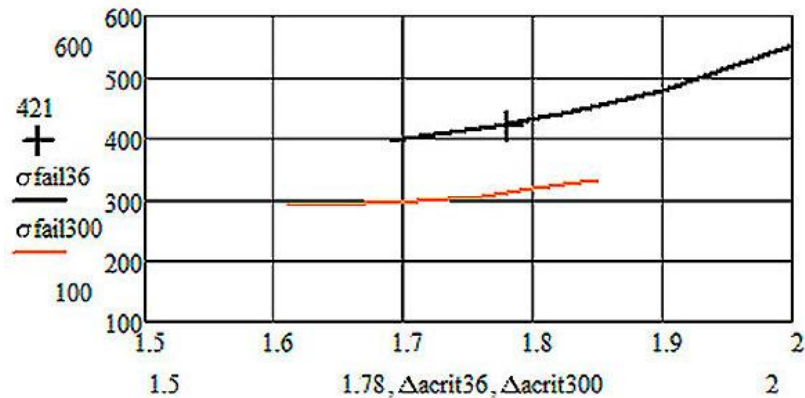


Fig.8, $w = 36\text{mm}$, $w = 300\text{mm}$: Effect of different panel geometry, ratios $w/d = 6$.

Result:

For a given resistance ratio c_{KR} , for two panel widths, above stress failure curves are plotted as functions of the individually given critical crack size. The wider panel allows a lower stress only, because more volume is highly stressed.

► Computation of the Reserve Factor for Design Limit load level, $j = 1$

Remote loading stress $\sigma = \sigma_I = 250\text{ MPa}$, $a_0 = 30\text{ mm}$, $d = 25\text{ mm}$.

Linear analysis is sufficient (presumption of FFM model): then $\sigma \sim \text{load}$

Assumed σ_{fail} , to be a Design Allowable, the Reserve Factor against 'Onset-of-Cracking' at the hole edge is

$$RF = \frac{\sigma_{struct}}{\sigma_{design}} = \frac{421\text{ MPa}}{250\text{ MPa}} = 1.7 .$$

According to the regulations, a Design Allowable has to be applied, too, which is assumed here, because R_m is a Strength Design Allowable and K_{Ic} is assumed to be statistically based.

Yielding Check in the net-section:, as a limit-of-usage check. One obtains:

$$\sigma_{fail} = \sigma_{netyield} = R_{p02} \cdot \left(1 - \frac{2 \cdot a_{ce}}{w}\right) = 425 \cdot \left(1 - \frac{2 \cdot 55.4}{300}\right) = 268\text{ MPa} .$$

$$RF = \frac{268}{250} = 1.14 > 1 .$$

Result: Due to the requirement $\sigma_{\text{netyield}} < \sigma_c$ net section yielding limits the loading here.

7 Application of the FFM to an HSB-example

Task: Mapping of the critical stress ‘ σ_c -curve’ as function of the running crack size a .

The course of just 3 test points of a fixed open hole panel (from HSB 62232-01 on ‘Width dependency of the Feddersen-parameter’, [10]), is to map. These fracture values are given for the original $a_0 = a + r$, also depicted in the plot.

Note, please: The 3 test points with the different crack sizes are assumed average values. (1) In this context, in the HSB sheet the sample size number of tests belonging to one ‘average’ point was not given. (2) Further, an additional fitting process of the foreseen correction function was performed.

Fig.9, left, displays the geometry and the loading of the envisaged HSB-panel. The coordinate x has its origin in the hole center.

Fig.9 right, presents the course of the SIF K and of the net section stress along x .

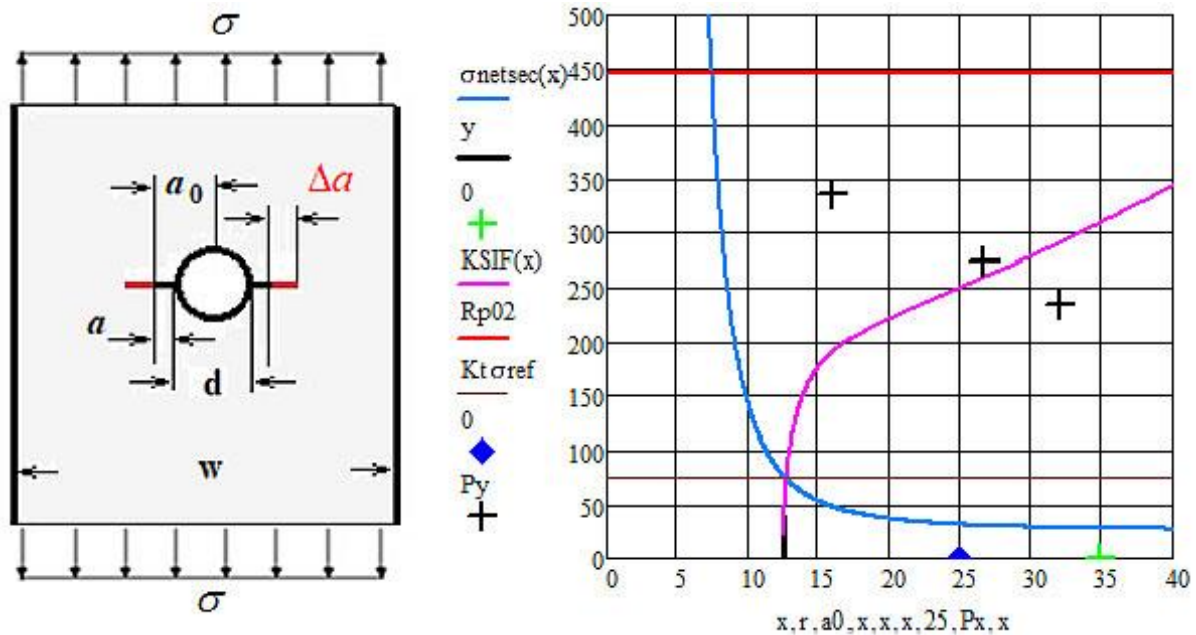


Fig.9: (left) Geometry of the fixed Open Hole Panel and its uniaxial loading. (right) Test points with the courses of the ‘SIF’ and the net section stress in width x -direction $w = 160$ mm, $t = 2$ mm, $d = 25$ mm. AA 7475-7761: $R_{p02} = 445$ MPa, $K_{C, \infty} = 2500$ MPa $\cdot \sqrt{\text{m}}$.

Abscissa points in mm: $x = r = 12.5$, $x = 25$, $x = 35$ $K_{tref} = +$

Result:

Shifting the FFM failure stress point by Δa gives a point a little far from the derived FM-curve, This crack size Δa defines the a_0 when analyzing future loading and crack growth.

In Fig.10 for the given hole, $d = 2 \cdot r$, the computed FFM-linked failure stress point σ_{fail} (bold) is depicted with the generated crack size Δa . The Mathcad computations are presented in Table 3.

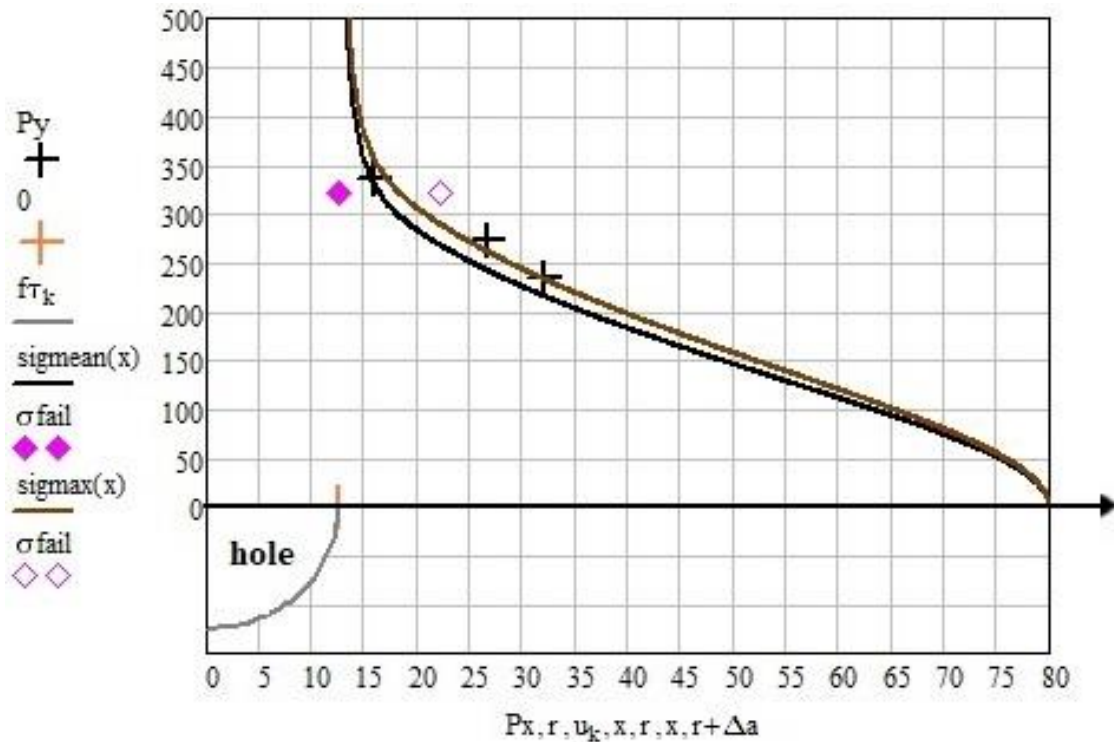


Fig.10: (a) Depiction of the FFM-based failure stress at ‘Onset-of-Cracking’ generating Δa .
 (b) FM-based mapping of the course of the three test points with its initial different crack size a_0 ,
 Application of two different K -values, $r = 12.5$ mm

The Mathcad computation is presented in [Table 3](#).

The upper part depicts the classical FFM procedure and the center the Cuntze procedure with directly using the single equations.

Result: Both, the procedures end with the same numbers.

Also a FM-linked mapping of the three test point examples with its initial crack sizes a_0 was successfully performed, see the bottom of [Table 3](#). Thereby the SIF K was varied, a mean and a maximum assumed value was applied.

This might be of interest for a rework of the ‘Feddersen parameter sheet’ HSB 63321-06.

Result:

Mapping was successful. The difference vanishes at both the ends.

Table 3, Mathcad computations:

(up) Standard FFM procedure (using the square), however solved without necessary iterations, $a_0 = 0$

(center) Cuntze's procedure: separate FFM- and SFC-equation, $a_0 = 0$

(down) FM-based mapping of the three test points with its individual initial cracks a_0

Vorgabe $\Delta a := 11$ $\sigma := 111$

$$\frac{1}{\Delta a} \int_r^{r+\Delta a} \left[\sigma \sqrt{\pi \cdot x} \cdot \sqrt{1 - \frac{r}{x}} \cdot \left[1 + 0.358 \cdot \frac{r}{x} + 1.425 \cdot \left(\frac{r}{x}\right)^2 - 1.578 \cdot \left(\frac{r}{x}\right)^3 + 2.156 \cdot \left(\frac{r}{x}\right)^4 \right] \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot r}{w}\right) \cdot \text{Sek}\left(\frac{\pi \cdot x}{w}\right)} \right]^2 dx = \frac{Kc^2}{Rp02^2}$$

$$\frac{1}{\Delta a} \int_r^{r+\Delta a} \sigma \cdot crw \cdot \left[0.335 + 0.665 \cdot \left(1 + c11 \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c12} + c13 \cdot \left(\frac{x-r}{0.5 \cdot w - r}\right)^4 \right]^2 dx = Rp02$$

$$\frac{1}{\Delta a} \int_r^{r+\Delta a} \sigma \cdot crw \cdot \left[0.335 + 0.665 \cdot \left(1 + c11 \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c12} + c13 \cdot \left(\frac{x-r}{0.5 \cdot w - r}\right)^4 \right] dx = Rp02$$

$\underline{\underline{A}} := \text{Suchen}(\Delta a, \sigma)$ $A = \begin{pmatrix} 9.6 \\ 322.28 \end{pmatrix}$ $\underline{\underline{\Delta a}} := A_0$ $\underline{\underline{\sigma}} := A_1$ $\Delta a = 9.6$ $\sigma = 322$

Vorgabe $\underline{\underline{\Delta a}} := 11$ $\underline{\underline{\sigma}} := 111$

$$\frac{1}{\Delta a} \int_r^{r+\Delta a} \left[\sigma \sqrt{\pi \cdot x} \cdot \sqrt{1 - \frac{r}{x}} \cdot \left[1 + 0.358 \cdot \frac{r}{x} + 1.425 \cdot \left(\frac{r}{x}\right)^2 - 1.578 \cdot \left(\frac{r}{x}\right)^3 + 2.156 \cdot \left(\frac{r}{x}\right)^4 \right] \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot r}{w}\right) \cdot \text{Sek}\left(\frac{\pi \cdot x}{w}\right)} \right]^2 dx = 1$$

$$\frac{Kc^2}{Rp02} \cdot \frac{1}{\Delta a} \int_r^{r+\Delta a} \sigma \cdot crw \cdot \left[0.335 + 0.665 \cdot \left(1 + c11 \cdot \frac{x-r}{0.5 \cdot w - r} \right)^{c12} + c13 \cdot \left(\frac{x-r}{0.5 \cdot w - r}\right)^4 \right] dx = 1$$

$A := \text{Suchen}(\Delta a, \sigma)$ $A = \begin{pmatrix} 9.6 \\ 322.3 \end{pmatrix}$ $\underline{\underline{\Delta a}} := A_0$ $\underline{\underline{\sigma}} := A_1$ $\Delta a = 9.6$ $\sigma = 322$

$\text{sigmean}(x) := \frac{2500}{\left[\sqrt{\pi \cdot x} \cdot \sqrt{1 - \frac{r}{x}} \cdot \left[1 + 0.358 \cdot \frac{r}{x} + 1.425 \cdot \left(\frac{r}{x}\right)^2 - 1.578 \cdot \left(\frac{r}{x}\right)^3 + 2.156 \cdot \left(\frac{r}{x}\right)^4 \right] \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot r}{w}\right) \cdot \text{Sek}\left(\frac{\pi \cdot x}{w}\right)} \right]^2}$

$\underline{\underline{Kc - Variants}}$

$\text{sigmax}(x) := \frac{2700}{\left[\sqrt{\pi \cdot x} \cdot \sqrt{1 - \frac{r}{x}} \cdot \left[1 + 0.358 \cdot \frac{r}{x} + 1.425 \cdot \left(\frac{r}{x}\right)^2 - 1.578 \cdot \left(\frac{r}{x}\right)^3 + 2.156 \cdot \left(\frac{r}{x}\right)^4 \right] \cdot \sqrt{\text{Sek}\left(\frac{\pi \cdot r}{w}\right) \cdot \text{Sek}\left(\frac{\pi \cdot x}{w}\right)} \right]^2}$

8 Conclusions, concerning

Strength criteria alone or energy-based fracture mechanical criteria alone cannot always lead to a reliable fracture failure prediction. Design Verification (DV) by using a coupled criterion will improve the situation and be an aid for understanding the stress state-dependent Onset-of-Cracking. The so-called FFM concept should bring a solution to close the gap. It assumes the formation of cracks of finite size Δa at Onset-of-Cracking.

Fracture Mechanics

The crack-linked residual strength R_{res} is the gross-sectional tensile stress σ at failure of a structural component containing a crack. R of the last fatigue phase is to discriminate from R_{res} in the previous fatigue phase. Thereby, the crack length a_0 at the beginning of the static up-loading will increase to its critical value a_c in general.

A structural component will fail in the case of static loading if the SIF K of a brittle material reaches its critical value at $K = K_c$, termed fracture toughness, which depends on the material behavior. The determination of the K_c values requires in the so-called K -concept used above the fulfilment of a geometric bound in order to achieve a real minimum value by taking a minimum test specimen thickness of

$$t > 2.5 \cdot (K_{Ic} / R_{0.2}^t)^2 \rightarrow \sigma_c = K_{Ic} / (\sqrt{\pi \cdot a_0} \cdot f(a_0)).$$

In the less brittle material case the limit reads $G = G_c$.

The influence of the geometry factor f decreases with the specimen thickness, resulting in fracture toughness independent of the specimen dimensions. For the same materials, the fracture toughness decreases with an increasing yield strength of 0.2 %.

Fig.11 shall illustrate how the failure stress is governed by the crack size. Plastic deformation plays a significant role.

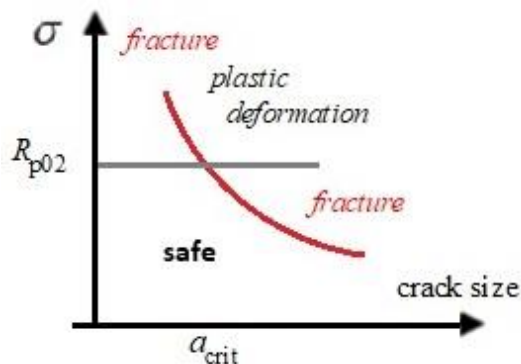


Fig.11: Illustration of the example with the concern plastic yielding

Strength

Dependent on the design requirements the average, the upper or a lower value of the property is used for the various physical properties.

In the case of the resistance property strength a statistically reduced value R is to apply and in order to achieve a reliable design a so-called Strength Design Allowable has to be applied. It is a value,

beyond which at least 99% (“A”-value) or 90% (“B”-value) of the population of values is expected to fall, with a 95% confidence (*on test data achievement*) level, see MIL-HDBK 17.

In this context, note please: Measurement data sets are the result of a Test Agreement (norm or standard), that serve the desire to make a comparability of different test procedure results possible. The Test Agreement consists of test rig, test specification, test specimen, test procedure and the test data evaluation method. Therefore, one could only speak about ‘*exact test results and properties in the frame of the obtained test quality*’.

Test specimens shall be manufactured like the structure, ‘as-built’.

Bearable load(ing)

The provision of bearable load(ing)s requires series tests of the distinctive structural component with statistical evaluation in order to determine a structural ‘load-resistance design allowable’. This is valid for the FFM applications. See the 3 average open-hole dots in *Fig.10*.

Load-defined Reserve Factor RF and design Factor-of-Safety FoS_j

* A RF is usually the result of worst case assumptions that does not take care of the joint actions of the stochastic design parameters and thereby cannot take care of their joint failure action and probability.

* The RF value does not outline a failure probability, and failure probability p_f does not dramatically increase if RF turns slightly below 1.

* A FoS is given and not to calculate such as a the Reserve Factor RF .

Application limits linked to FFM

In Design, as with each criterion, validity limits are faced, such as

- Application-extension of linear structural analysis and high brittleness
- Future task to capture small scale yielding at the crack tip which requires the provision of the associate statistically-based toughness K_{Ic} -values in order to master Design Verification
- The stress in the net-section of the panel should not exceed the tensile yield strength R_{p02} .
- 3D-application.

Many thanks to my friends, Prof. Dr.-Ing. habil. Wilfried Becker and Dr.-Ing. Jürgen Broede for the excellent exchange on this difficult novel topic FFM.

Annex

1. Course of net-section stress

In the context above and because it is necessary for understanding the FFM an illustration of the stress distribution along the net-section is to provide. In *Fig.12* the curves are depicted for the x- and an integration-simplifying normalized ξ -coordinate, proposed in HSB 34112-11. The relationship

reads $x = d/2$:
$$\xi = \frac{2 \cdot x / d - 1}{w / d - 1} = \frac{x - r}{0.5 \cdot w - r} = 0 \quad (\text{hole edge})$$

and $x = a = d/2 + \Delta a$:
$$\xi = \frac{2 \cdot x / d - 1}{w / d - 1} = \frac{2 \cdot \Delta a / d}{w / d - 1} = \Delta \alpha, \text{ abbreviated.}$$

In [10] was given

$$\sigma_y = \sigma_{\text{netsec}}(\xi) = \sigma \cdot K_{t,wd} \cdot [0.335 + 0.665 \cdot (1 + c_{11} \cdot \xi)^{c_{12}} + c_{13} \cdot \xi^4]$$

with the geometry-dependent stress concentration factor $K_t(w, d)$

$$K_t(w, d) = 3.215 - \left(\frac{w}{d}\right)^{-0.5} + 4.294 \cdot \left(\frac{w}{d}\right)^{-1.5} \equiv c_{dw}$$

and the abbreviation functions

$$c_{11} = -3.765 + 2.148 \cdot \left(\frac{w}{d}\right)^{0.879}, \quad c_{12} = -2.552 - 42.894 \cdot \left(\frac{w}{d}\right)^{-3.17}, \quad c_{13} = -0.7497 \cdot \left(\frac{w}{d}\right)^{-1.858}.$$

For the example $w = 300$ mm, $d = 25$ mm, $c_{dw} = 3.03$, $c_{11} = 19.5$, $c_{12} = -2.56$, $c_{13} = -4.9 \cdot 10^{-3}$ follow after normalization by $K_{t, \infty}(w = \infty) = 3$, and setting a reference stress $\sigma = 100$ MPa the following plots:

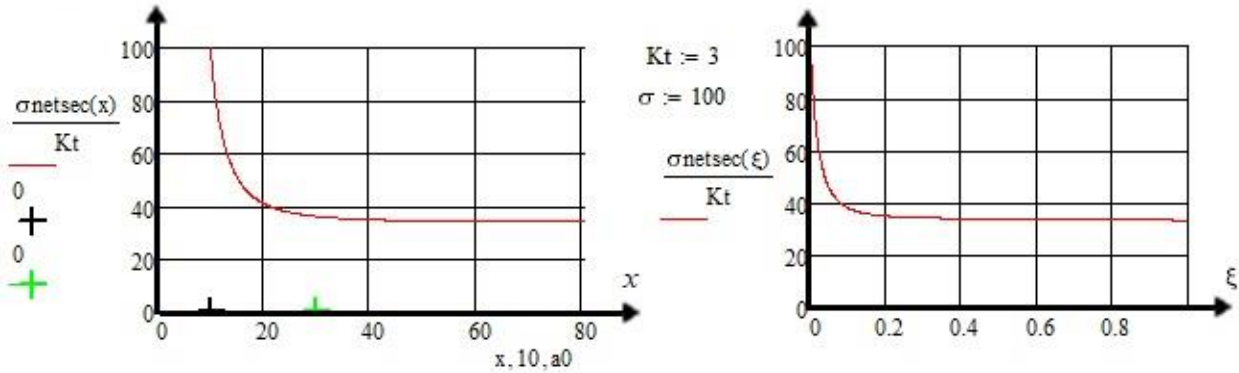


Fig.12: Contour of the stress along the net-section of the panel considering the coordinates x and ξ .

$$\sigma_{\text{ref}} = 100 \text{ MPa}, \quad r = 12.5 \text{ mm}, \quad a_0 = 30 \text{ mm}, \quad K_{t, \infty} = 3, \quad x \text{ width coordinate (ligament)},$$

$$\xi = (x - r) / (0.5 \cdot w - r),$$

Results:

With increasing distance to the hole edge the stresses are monotonically descending whereas the incremental energy release rate \mathcal{G} is monotonically ascending (see Fig.12).

2. Integration of net-section stress

HSB 34112-11 computation, retraced:

Applying the afore mentioned coordinate transformation $x \rightarrow \xi$ enables the following symbolic integration

$$\begin{aligned} \frac{1}{\Delta a} \cdot \int_r^{r+\Delta a} \sigma_y \cdot dx &= \frac{\sigma \cdot c_{wd}}{\Delta \alpha} \cdot \int_0^{\Delta \alpha} [0.335 + 0.665 \cdot (1 + c_{11} \cdot \xi)^{c_{12}} + c_{13} \cdot \xi^4] \cdot d\xi \\ &= \frac{\sigma \cdot c_{wd}}{\Delta \alpha} \cdot \left[0.335 + c_{14} \cdot \frac{(1 + c_{11} \cdot \Delta \alpha)^{c_{15}} - 1}{\Delta \alpha} + c_{16} \cdot \Delta \alpha^4 \right] \\ \text{with} \quad c_{14} &= \frac{0.665}{c_{11} \cdot (c_{12} + 1)}, \quad c_{15} = c_{12} + 1, \quad c_{16} = \frac{c_{13}}{3}. \end{aligned}$$

Variant Cuntze:

Despite of the more complicate integration limit $r + \Delta a$ instead of Δa , the Mathcad solution process allows to stick to the x coordinate, avoiding a mixture of α with a within the solution process. Inserting into the equation above the relationship $\xi = (x - r) / (0.5 \cdot w - r)$ leads to

$$\begin{aligned} \frac{1}{\Delta a} \cdot \int_r^{r+\Delta a} \sigma_y \cdot dx &= \frac{1}{\Delta a} \cdot \int_r^{r+\Delta a} \sigma \cdot c_{wd} \cdot [0.335 + 0.665 \cdot (1 + c_{11} \cdot \xi)^{c_{12}} + c_{13} \cdot \xi^4] \cdot dx \\ &= \frac{1}{\Delta a} \cdot \int_r^{r+\Delta a} \sigma \cdot c_{wd} \cdot [0.335 + 0.665 \cdot (1 + c_{11} \cdot \frac{x-r}{0.5 \cdot w-r})^{c_{12}} + c_{13} \cdot (\frac{x-r}{0.5 \cdot w-r})^4] \cdot dx. \end{aligned}$$

Result:

The solution of the coupled equation delivers the remote failure stress with its associated crack length size Δa , see Table 3, too.

Of interest could be the effect of a varying panel width geometry. Finally Fig.13 plots the influence of the resistance ratio $c_{KR} = K_{app}^2/R_{app}^2$ on the critical crack size Δa_c . The c_{ik} are the variables:

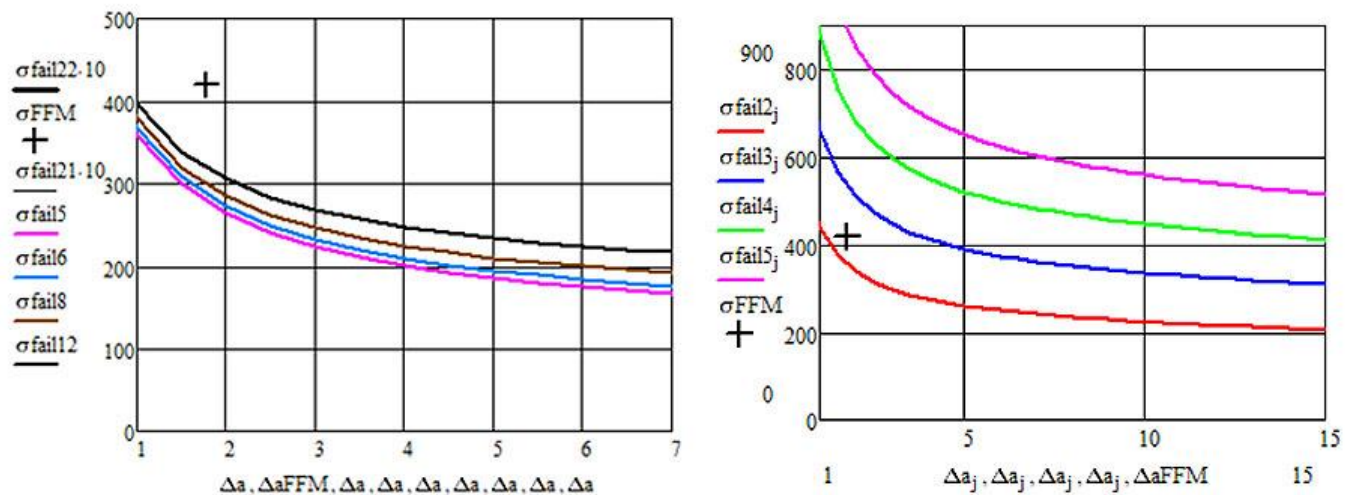


Fig.13 (l left), general, $w = 300\text{mm}$: Effect of different panel geometry, ratios $w/d=5, 6, 8, 12$ as variables.
 Fig.13 (right), general, $w = 300\text{mm}$: Effect of different resistance ratios $K_{Ic}^2/R_m^2 = 2, 3, 4, 5$

Lessons Learned on FFM and its two parts

FFM:

- In the case of plain structural parts ‘Onset-of-Cracking’ in brittle and semi-brittle materials cannot be fully captured by the SFCs, because both a critical energy and a critical stress state must be fulfilled. Therefore, SFCs are ‘just’ necessary but not sufficient for the prediction of strength failure, onset of cracking. Basically, also due to internal flaws, an energy criterion is to apply
- The novel approach ‘Finite Fracture Mechanics (FFM)’ offers a 2D hybrid criterion to more realistically predict the stress-based ‘Onset-of-Cracking’ in brittle isotropic (the focus here) and UD materials.
- FFM enables to predict a hybrid (coupled) failure stress = a resistance quantity on basis of the resistances of the FFM-parts fracture mechanics (FM) and structural strength (SFC)
- FFM is advantageous for the analysis of notched structural parts and captures applications usually treated by the well-known Neuber theory. The coupled FFM-criterion ‘SFC-FM’ can be used with some confidence to predict onset of cracking (failure) in brittle materials in design situations as never could be done before.

- *The FMC-application looks successful for the 'open hole panel' example, a realistic failure stress can be estimated.*
- *Unfortunately there is still a lack of test data sets for the validation of FFM*
- *Multi-axial stress states are captured by the principal stress σ_1*
- *Using a locally evenly distributed stress curve averaged over the finite length Δa is to check*

FM (R-curve):

- *It is to regard, when considering the formulations to be applied: Short Cracks behave differently to Large Cracks*
- *It is unbelievable (see the treated HSB example Feddersen concept) that no test results can be found in literature concerning panels with different ratios 'width/hole radius'. Such tests should have been performed when investigating the Neuber theory*
- *Notch surface quality and the metal homogeneity faced have its impacts on the results.*
- *The R-curve does not depend on a_0 and w .*
- *The fracture stress is to base on $a_e = a + \Delta a + \omega$.*
- *Principal stress-linked.*

SFCs Cuntze:

- *Full 3D- stress state-capable.*

9 ‘Curiosities’ regarding Classical Material Mechanics

Aim: Filling two rooms in the Material Mechanics Building by proving the assumed ‘generic’ number.

Regarding a material ‘generic’ number of 2 to be valid for isotropic materials there are two ‘empty rooms’ in the author-assumed ‘Mechanics Building’ of Isotropic Materials namely Normal Yielding NY and a counterpart of the tensile fracture toughness $K_{Icr}^{(t)}$ in the compressive domain.

9.1 Normal Yielding NY: [CUN22, §4]

Glassy, amorphous polymers like polystyrene (PS), polycarbonate (PC) and PolyMethylMethacrylate (PMMA = plexiglass) are often used structural materials. They experience two different yield failure types, namely crazing under tension (*Fig. 9-1*) and under compression a

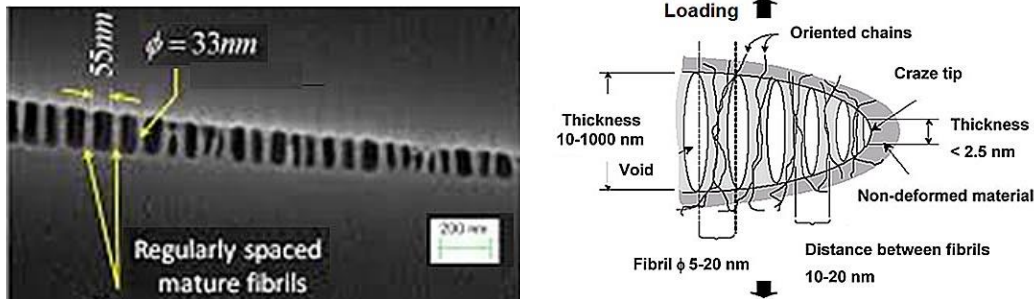


Fig. 9-1: PMMA, SEM image of a craze in Polystyrene Image (created by Y. Arunkumar)

shear stress yielding that is often termed by material specialist ‘shear-banding’.

Crazing can be linked to Normal Yielding (NY) which precedes the crazing-following tensile fracture. Crazing occurs with an increase in volume through the formation of fibrils bridging built

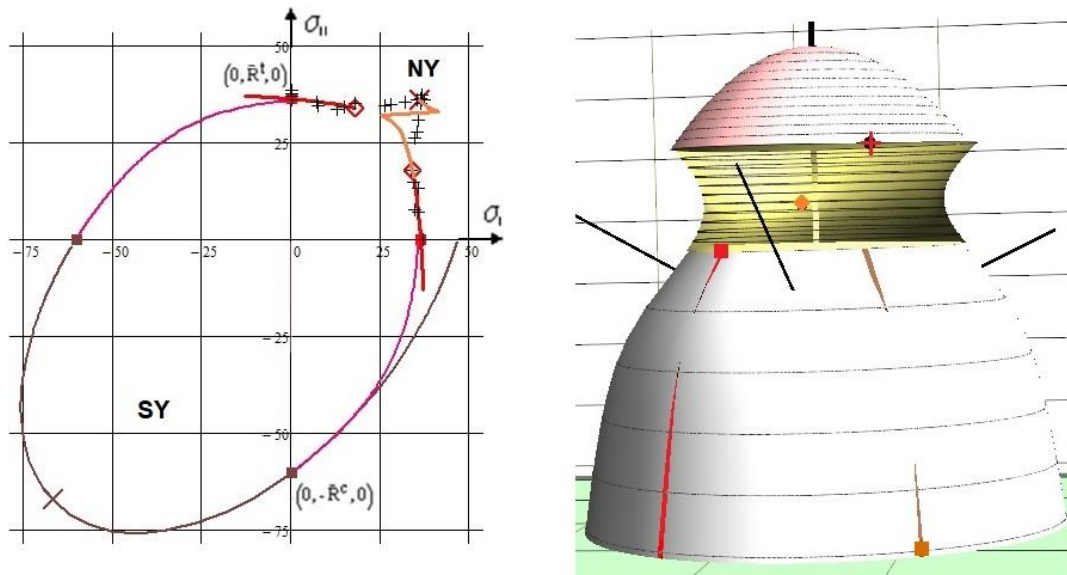


Fig. 9-2, PMMA: (left) Mapping of test data in tension and compression principal stress domain with and without interaction; (right) depiction of the fracture body shape with some representative points. For the validation of the FMC-based SFC for PMMA two data sets were available, one NY-2D-data set from Sternstein-Myers and a SY-3D-data set from Matsushige

[Ste73] Sternstein S S and Myers F A: Yielding of glassy polymers in the second quadrant of principal stress space. J. Macromol. Sci, Phys. B 8 (1973), 539-571

micro-cracks and shear banding keeps volume. Therefore, due to the FMC ‘rules’ the dilatational I_1^2 is to employ in the SFC-approach for tension $I_1 > 0$. Under compression, brittle amorphous polymers classically shear-band (SY) and experience friction. Therefore, I_1 must be employed in the approach for $I_1 < 0$ in order to consider material internal friction. ‘Mises’ means frictionless yielding and therefore it forms a cylinder.

For obtaining the complete yield failure body (*Fig.9-2*) its parts NY and SY are to interact in the transition zone. Doing this the used Mathcad 15 code had no problems to generate the **3D**-failure body, however the **2D**-visualization of the NY failure surface using Mathcad 15 code (*a 35 DIN A4-pages application*) was too challenging for the solver which had to face a concave 2D principal stress plane situation instead of a usual convex one.

LL: *The failure type crazing shows a ‘curiosity’ under tensile stress states: A non-convex shape exists for Onset-of-Crazing (\bar{R}_{NY}^t). This violates the convexity stability postulate of Drucker, meaning “If the stress-strain curve has a negative slope then the material is not Drucker-stable”. The inflection point of the hyperboloid results from the derivation dF/dI_1 of the NF criterion,*

$$\text{neglecting } 120^\circ\text{-symmetry (see later chapter) } F^{NF} = \frac{\sqrt{4J_2 - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t} = 1.$$

9.2 Compressive (shear) Fracture Toughness $K_{IIcr}^{(c)}$, [CUN22,§4.2]

Some reasons caused the author to search a compressive fracture toughness:

- An early citation of A. Carpinteri, that approximately reads: “*With homogeneous isotropic brittle materials there are 2 real energy release rates G_{Icr} , G_{IIcr} , one in tension and one in compression*”
- The number of the (basic) fracture toughness quantities may be *theoretically at least* also 2, namely $K_{Icr}^t \equiv K_{Ic}$ together with K_{IIcr}^c (*Fig.9-3*) and
- The novel approach Finite Fracture Mechanics (FFM) that offers a hybrid criterion to more realistically predict the crack initiation in brittle isotropic and UD materials.

A stringent postulate for the author was crack path stability which can be explained “*Only an angle-stable, self-similar crack growth plane-associated critical Stress Intensity Factor (fracture toughness) is a ‘basic’ property*”. This requires as presumption an *ideally* homogeneous isotropic material in front of the crack-tip. Therefore, the investigation is only for an ideal structural mechanics building of importance, because in practice, there are usually no ideal homogeneous conditions at the crack-tip.

Practically, fracture mechanics is presently only tensile driven performed using $K_{Ic} = K_{Icr}^t$ as a clear critical fracture intensity, where the crack plane does not change (the index cr is necessarily to be taken in this document in order to separate tension ^t from compression ^c). Why shouldn’t there not be a quantity K_{IIcr}^c that fits as an opposite complement to K_{Icr}^t and where, in an ideal case of no flaws in front of the crack tip, the crack plane grows further along the generated shear fracture angle under a compressive fracture load?

The Fracture Mechanics Mode I delivers a real, ‘basic’ fracture resistance property generated under a tensile stress. Both the Modes II K_{IIc} , and III K_{IIIc} do not show a stable crack plane situation but are nevertheless essential FM model parameters to capture ‘mixed mode loading’ for performing a multi-

axial assessment of the far-field stress state. $\rightarrow \bar{R}^t$ and \bar{K}_{Icr}^t correspond! They are ‘just’ very helpful model parameters driving the crack plane in direction of a finally KIc-driven failure.

With the Mode-II compressive fracture toughness K_{IIcr}^c it is like with strength. One says compressive failure, but actually shear (stress) failure is meant, compressive stress is ‘only’ the descriptive term. Therefore the shear index $_{II}$ is to apply with K_{IIcr}^c . One has to keep in mind: In mechanical engineering the structural tasks are usually lie in the tension domain (*index ^t is skipped*), whereas oppositely in civil engineering the compression domain is faced (*index ^c is skipped*):

*Tension domain: One knows from K_{Icr}^t (tension), that – viewing the fracture angle - it corresponds to R^t .

*Compression domain: Above not generally known second basic SIF K_{IIcr}^c seems to exist under ideal conditions. It corresponds to shear fracture SF happening under compressive stress R^c and leading to the angle Θ_{fp}^c . The crack surfaces are closed for K_{IIcr}^c , friction sliding occurs.

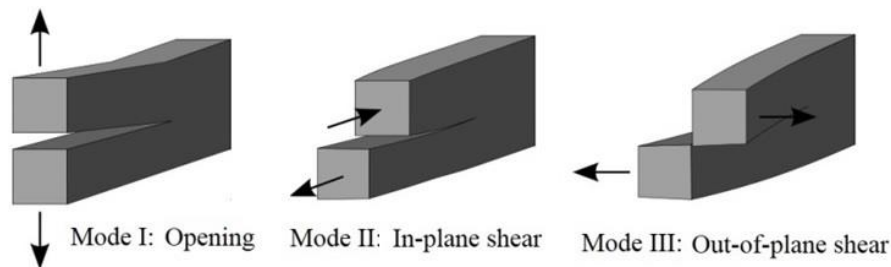


Fig.9-3: Classical Fracture Mechanics modes

Some proof of the author’s postulate could be: There exists a minimum value of the compressive loading at a certain fracture angle. This means that the K_{IIcr}^c becomes a minimum, too. Liu et al performed in [Liu14] tests using a cement mortar material, (Fig.9-4). ► From his measured results, by now, it seems to - theoretically at least - that the ‘generic’ number 2 is met.

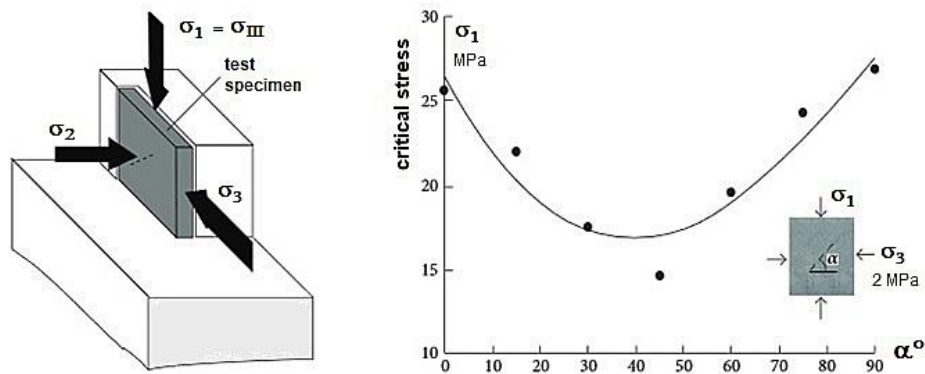


Fig.9-4: Scheme of the test set-up and of the test points obtained for cement mortar [Liu14], σ_1 represents the mathematical stress σ_{III} (largest compressive stress value).

[Liu J, Zhu Z and Wang B: The fracture characteristic of three collinear cracks under true tri-axial compression. The scientific World Journal, V 2014, article ID459025]

For the transversely-isotropic UD lamina materials it seems directly to match: ► 5 fracture toughness properties correspond to 5 strength properties, ‘generic’ number postulate is fulfilled.

LL: **Fracture Mechanics seems to follow material symmetry 'rules' and to possess a 'generic' number, too.*
** Note on K_{IIcr}^c as a design entity: Is of theoretical, but not of practical value due to the usually faced not ideal homogeneous situation of 'isotropic materials' at crack tips.*

10 Automated Generation of Constant Fatigue Life curves considering Mean Stress Effect

Aim: Automated derivation of the Constant Life Curve with discussion of the Mean Stress Correction

Generally, in Design Verification (DV) it is to demonstrate that “No relevant limit failure state is met considering all Dimensioning Load Cases (DLCs)”. This involves cyclic DLCs, focusing lifetime with non-cracked and cracked structural parts (*the latter would require Damage Tolerance tools*). Methods for the prediction of durability, regarding the lifespan of the structural material and thereby of the structural part, involves long time static loading which is linked to ‘static fatigue’ and in particular to ‘cyclic fatigue’. Fatigue failure requires a procedure for the Fatigue Life Estimation necessary to meet above cyclic DV.

Domains of Fatigue Scenarios and Analyses are:

LCF: high stressing and straining

HCF: intermediate stressing $10.000 < n < 2.000.000$ cycles (*rotor tubes, bridges, towers, off-shore structures, planes, etc.*)

VHCF: low stress and low strain amplitudes (*see SPP1466 Very High Cycle Fatigue > 10^7 cycles (in centrifuges, wind energy rotor blades, etc.)*).

Principally, in order to avoid either to be too conservative or too un-conservative, a separation of the always needed ‘analysis of the average structural behaviour’ in Design Dimensioning (*using average properties and average stress-strain curves*) in order to obtain the best structural information (= 50% expectation value) is required from the mandatory single DV-analysis of the final design, where statistically minimum values for strength and minimum, or mean and maximum values for other task-demanded properties are applied as Design Values.

10.1 Fatigue Micro-Damage Drivers of Ductile and Brittle behaving Materials, see [Cun23b]

There are strain-life (*plastic deformation decisive, plastic strain-based $\epsilon_p(N)$*) and stress-life models (*SN*) used. For ductile materials, strain-life models are applied because a single yield mechanism dominates and the alternating stress amplitude counts. For brittle materials, the elastic strain amplitude becomes dominant and stress-life models are applied. With brittle materials inelastic micro-damage mechanisms drive fatigue failure and several fracture mechanisms may come to act. This asks for a modal approach that captures all failure modes which are now fracture modes.

Above two models can be depicted in a Goodman diagram and in a Haigh diagram. The Haigh diagram (σ_a, σ_m) will be applied here because the often used Goodman employs just one quantity σ_a or $\Delta\sigma = 2 \cdot \sigma_a$ or σ_{\max} which is not sufficient. A Haigh Diagram represents all available SN curve information by its ‘Constant Fatigue Life (CFL) curves, being the focus here and using the two quantities σ_a, R .

Basic differences between ductile and brittle materials are the following ones:

- Ductile Material Behavior, isotropic materials: mild steel
 - 1 *micro-damage* mechanism acts \equiv “*slip band shear yielding*“ and drives micro-damage under tensile, compressive, shear and torsional cyclic stresses: *This single mechanism is primarily described by 1 SFC, yield failure condition (HMH, ‘Mises’)*!
- Brittle Material Behavior, isotropic materials: concrete, grey cast iron, etc.
 - 2 *micro-damage* driving mechanisms act \equiv 2 *fracture failure modes Normal Fracture failure*

(NF) and Shear Fracture failure (SF) under compression described by 2 fracture conditions, the 2 SFCs for NF and SF, where porosity is always to consider

- Brittle Material Behavior, transversely-isotropic UD-materials:
5 micro-damage driving fracture failure mechanisms act \equiv 5 fracture failure modes described by 5 SFCs or strength fracture failure conditions.

A very essential topic is the so-called ‘Mean stress sensitivity’: Within [Cun23b] the author attempts to redirect the ‘Thinking, resulting from ductile material behavior using ‘Mean stress influence correction factors’, which in reality means ‘Walking on crutches’, into a direct ‘Thinking with fracture modes facing a realistic brittle material behavior’.

Not fully ductile isotropic materials show an influence of the mean stress on the fatigue strength depending on the (static) strength ratio R^c/R^t and the material type. Mean stresses in the tensile range, $\sigma_m > 0$ MPa, lead to a lower permanently sustainable amplitude, whereas compressive mean stresses $\sigma_m < 0$ MPa increase the permanently sustainable amplitude or in other words.

LL:

- * A tensile mean stress lowers the fatigue strength and a compressive mean stress increases the fatigue strength
- * If it is a pretty ductile material one has one mode ‘yielding’ and if the material is pretty brittle then many ‘fracture modes’ are to consider
- * Brittle materials like the transversely-isotropic UD material with its five fracture failure modes possess strong mean stress sensitivity, a brittle steel material just 2 modes
- * Whether a material has an endurance fatigue limit is usually open regarding the lack of VHCF tests. The strength at $2 \cdot 10^6$ cycles might be only termed apparent fatigue strength (scheinbare Dauerfestigkeit). However, e.g. CFRP could possess a high fatigue limit.
- * Whether the material’s micro-damage driver remains the same from LCF until VHCF is questionable and must be verified in each given design case (continuum micro-damage mechanics is asked here)
- * The ‘ductile material behavior thinking’ in ‘Mean stress influence’ is to redirect for brittle materials into a thinking in fracture modes.

10.2 Mapping Challenge of the decisive Transition Zone in the Haigh diagram [Cun23b]

The course of the test data in the transition zone determines the grade of the mean stress sensitivity. In Fig.10-1, at first all essential quantities are illustrated. Further, two Constant Fatigue Life (CFL)-**curves** of a brittle material are displayed, for the envelopes $N = 1$ and $N = 10^7$. The pure mode domains are colored and the so-called transition zone is separated by R_{trans} into two influence parts. The course of the R-value in the Haigh diagram is represented by the bold dark blue lines. The CFL curve $N = 1$ is curved at top because 2 modes act in the case of brittle materials! This is in contrast to uniaxial static loading, depicted by the straight static envelopes, $N \neq N_f$: One micro-damage cycle results from the sum of 2 micro-damage portions, one comes from uploading and one from unloading! For fully ductile materials practically no transition zone between 2 modes exists, because just one single mode reigns, namely ‘shear yielding’. Therefore, it is no mean stress effect to correct in this case!

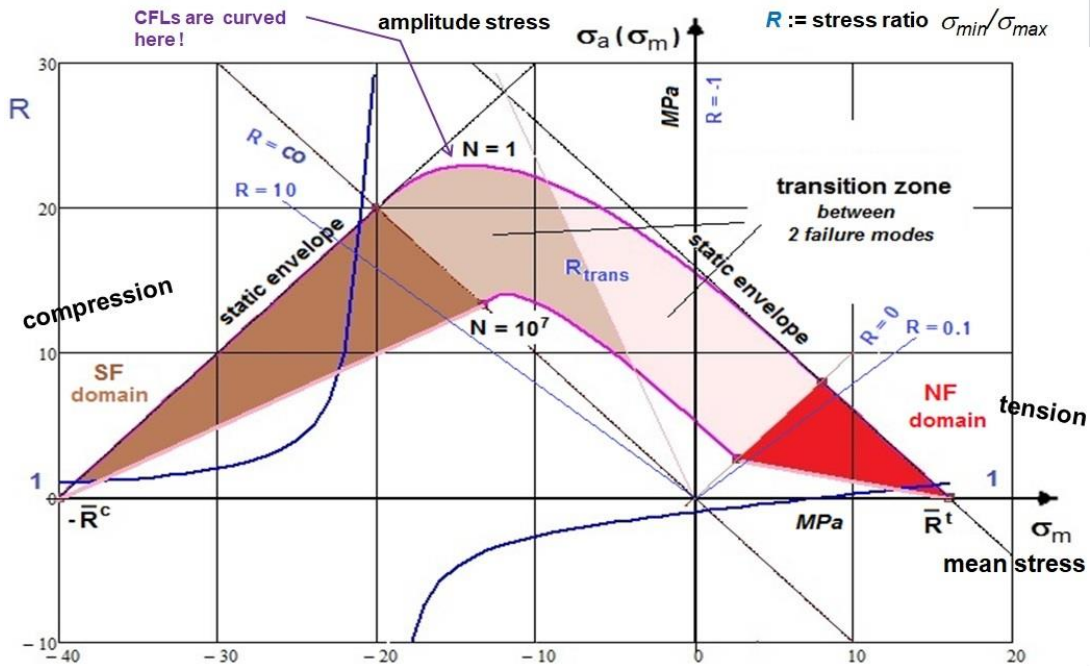


Fig.10-1, Haigh Diagrams: Scheme of pure mode domains, course of R and transition zone parts .
 (a:= amplitude, m:= mean, N := number of fracture cycles, \bar{R}^c := strength and $R := \sigma_{min}/\sigma_{max}$)

The quality of mapping the course of data in the transition zone is practically checked by “How good is the more or less steep course along the stress ratio R_{trans} -line mapped?” This is performed by following the physical reality, that the pure SF-domain is fully decoupled from the NF-domain, and employing oppositely running decay functions f_d , see Fig. 10-2.

$$Eff = [(Eff^{NF})^m + (Eff^{SF})^m]^{m^{-1}} = 100\% \text{ or}$$

$$\left(\frac{-(\sigma_{2m} - \sigma_{2a}) + |\sigma_{2m} - \sigma_{2a}|}{2 \cdot \bar{R}_\perp^c \cdot f_d} \right)^m + \left(\frac{\sigma_{2m} + \sigma_{2a} + |\sigma_{2m} + \sigma_{2a}|}{2 \cdot \bar{R}_\perp^t \cdot f_d} \right)^m = 1$$

$$\rightarrow f_d = 1 / [1 + \exp(\frac{c_1 + \sigma_m}{c_2})].$$

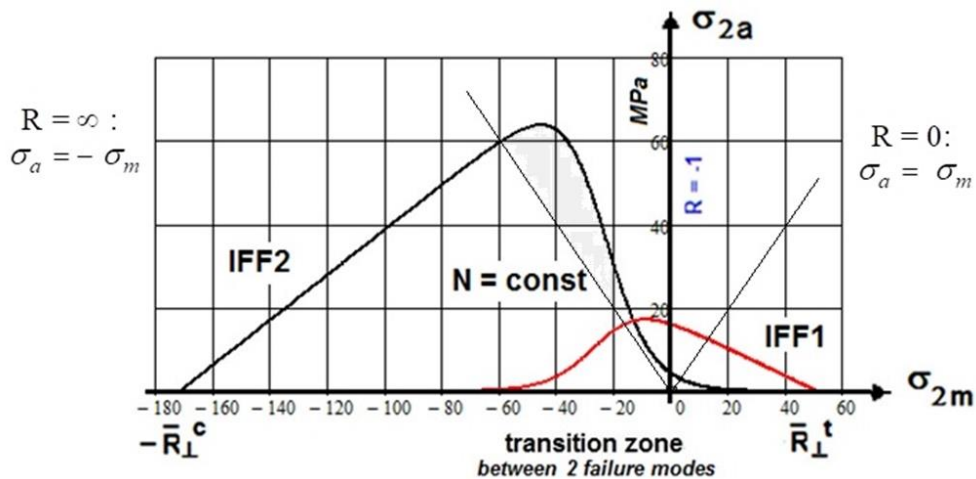


Fig.10-2, example UD material: Course of the decay functions in the transition zone $-\infty < R < 0$

Fig.10-2 illustrates the course of the mode decay functions f_d for the tension and the compression domain. The straight lines in the figure present the extreme SN curve beams, $R = \infty$ for the SF domain and $R = 0$ for the NF domain. In between, the envisaged slightly colored transition zone ($-\infty < R < 0$) is located. Mean stress sensitivity of brittle materials is demonstrated very impressively if the so-called ‘strength ratio’ = compressive strength / tensile strength R^c/R^t is high. The two plots in Fig.10-3 will clearly document this.

LL:

- * A large strength ratio R^c/R^t stands for a large mean stress sensitivity
- * A steep decay cannot be captured by a ‘mean stress correction factor’ as can be still performed with not fully ductile materials

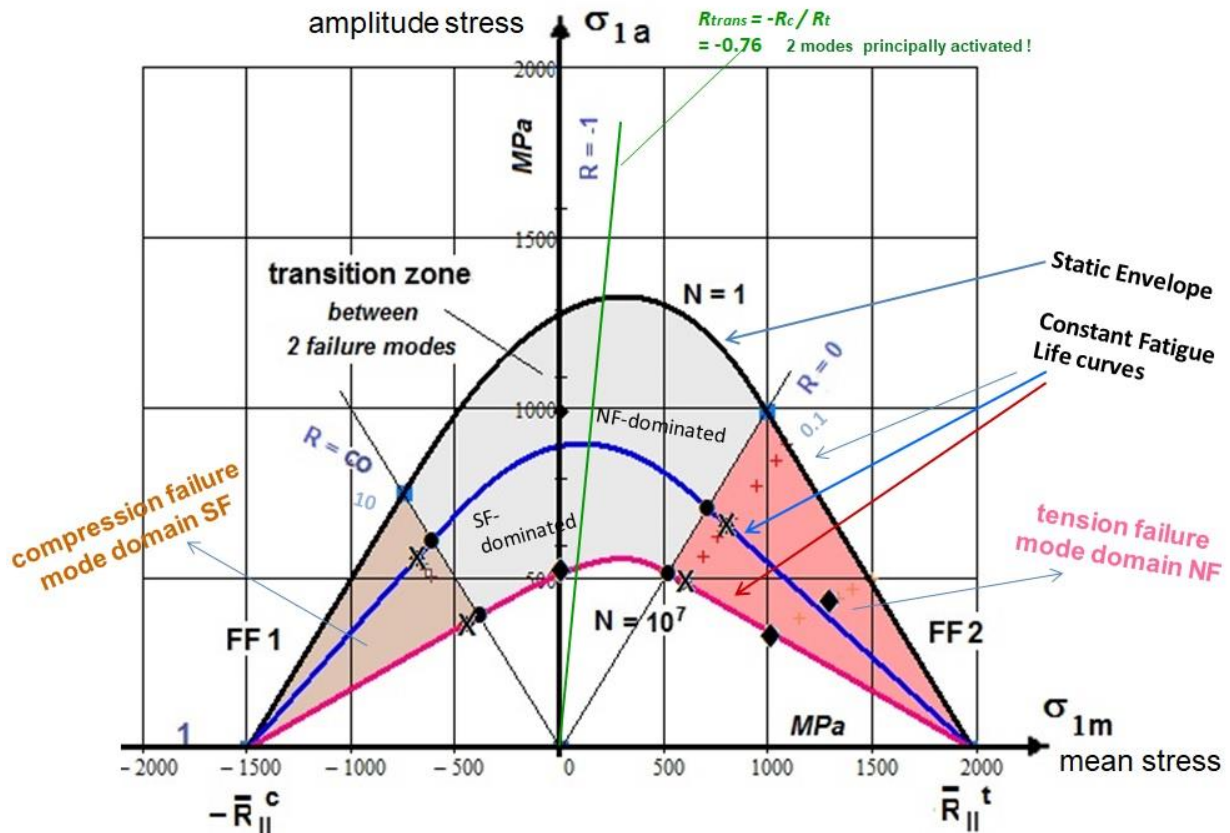
10.3 Estimation of the cyclic Micro-damage Portions of Brittle Materials

A very essential question in the estimation of the lifetime of brittle materials is a means to assess the micro-damage portions occurring under cycling. Here, for brittle behavior the response from practice is: It is permitted to apply validated static SFCs due to the experienced fact:

“If the failure mechanism of a mode cyclically remains the same as in the static case, then the fatigue micro-damage-driving failure parameters are the same and the applicability of static SFCs is allowed for quantifying micro-damage portions”. This is supported because FMC-based static SFCs apply equivalent stresses of a mode SF or NF. See again Fig.10-2 above.

10.4 Automatic Establishment of Constant Fatigue Life Curves (for details , see [Cun23b])

For a decade the author’s intensive concern was to automatically generate Constant Fatigue Life curves on basis of just a few tested Master SN curves coupled to an appropriate physically based model. Such a model the author obtained when M. Kawai gave a presentation during the author’s



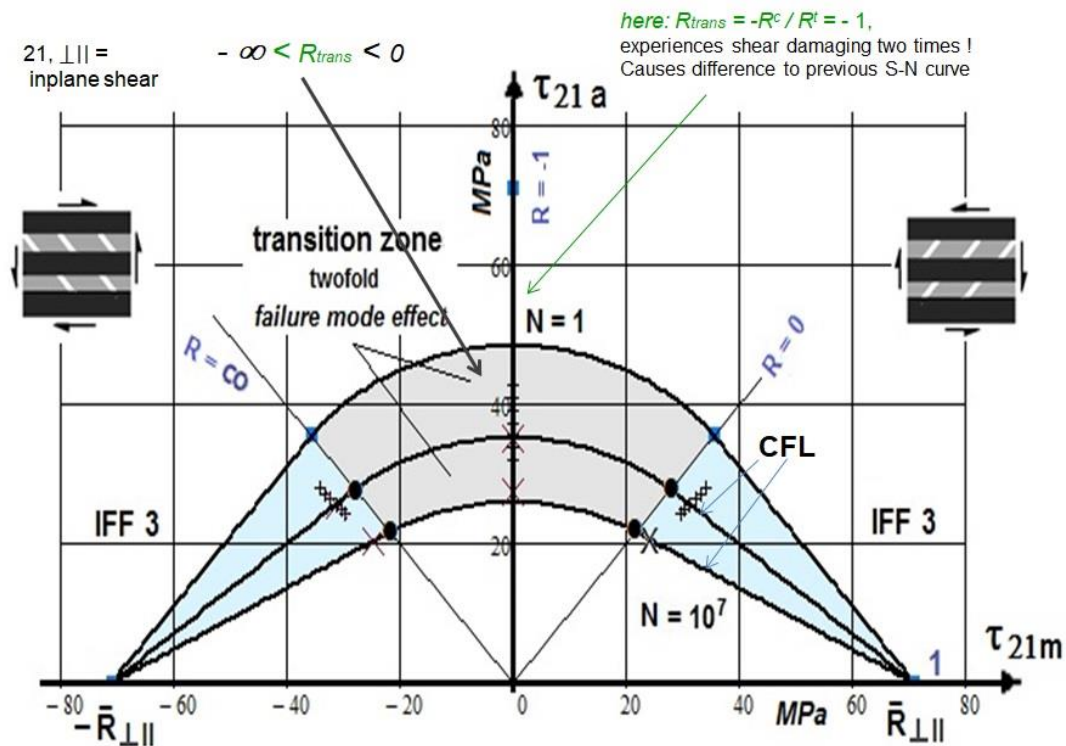


Fig.10-3, UD Haigh diagram: (up) FF with low strength ratio as with ductile materials. Rigorous Interpretation of the Haigh diagram for the UD-example FF1-FF2 displaying failure mode domains and transition zone [16], CFRP/EP, $\bar{R}_I^t = 1980$, $\bar{R}_I^c = 1500$, $\bar{R}_\perp = 51$, $\bar{R}_\perp^c = 172$, $\bar{R}_{\perp II} = 71$ [MPa].

(down) IFF with high strength ratio as with brittle materials Display of a two-fold mode effect ($a :=$ amplitude, $m :=$ mean, $N :=$ number of fracture cycles, $\bar{R} :=$ strength and $R := \sigma_{min}/\sigma_{max}$). Test data CF/EP, courtesy Clemens Hahne, AUDI

conference on composite fatigue in 2010 at CU Augsburg. Kawai's so-called 'Modified fatigue strength ratio' Ψ - model was the fruitful tool found. Kawai's presented procedure was a novelty and is applicable to brittle materials such like UD plies (depicted later in Fig.10-4) and isotropic concrete material as well.

Fig.10-3 (left) displays the differently-colored failure mode domains FF1-FF2 in a UD FF Haigh diagram and (right) IFF1-IFF2 in a UD IFF Haigh diagram. The available test data set along R_{trans} in the transition zone is represented by the crosses.

The decay model quality in Fig.10-3(right) proves the efficiency of the decay functions in the transition zone. For proving this the author is very thankful because this was only possible because he got access to the test results of C. Hahne, AUDI.

In Fig.10-4 the course of the cyclic failure test data can be well mapped by the 4-parameter Weibull formula $R = \text{constant} : \sigma_{max}(R, N) = c_1 + (c_2 - c_1) / \exp(\log N / c_3)^{c_4}$.

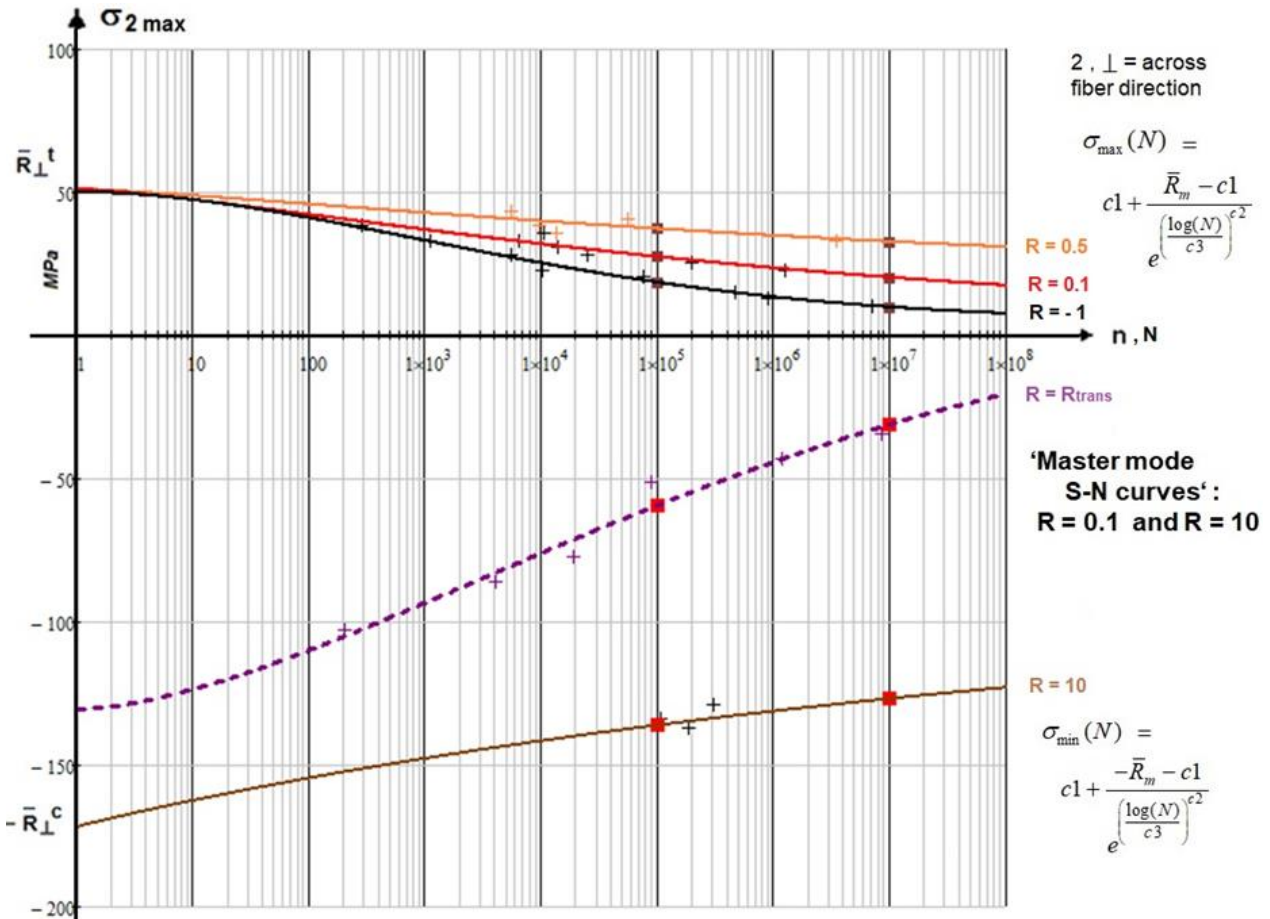


Fig.10-4: SN-curve, lin-log displayed IFF1-IFF2-linked SN curves [test data, courtesy C. Hahne, AUDI]
 [Kawai M: A phenomenological model for off-axis fatigue behavior of uni-directional polymer matrix composites under different stress ratios. Composites Part A 35 (2004), 955-963]

10.4 Lifetime Estimation

The so-called Palmgren-Miner rule is applied for summing up the cyclic micro-damage portions. Statistical analyses in the German aeronautical handbook HSB have shown that the fatigue life estimation using the linear accumulation method of Palmgren-Miner tends to be too optimistic. However a satisfactory reason with correction could not yet found:

- One explanation is the 'Right use of the right SFC: Mises is not anymore fully applicable?'
- A more severe second explanation is the loss of the loading sequence, an effect which is different for ductile and brittle materials. This inaccuracy is practically considered in design by the application of the so-called Relative Miner with defining a D_{feasible} and which must be $< 100\%$.

In the case of variable amplitude loading several SN curves are needed. An example for the computation of the lifetime estimation is displayed by Fig.10-5.

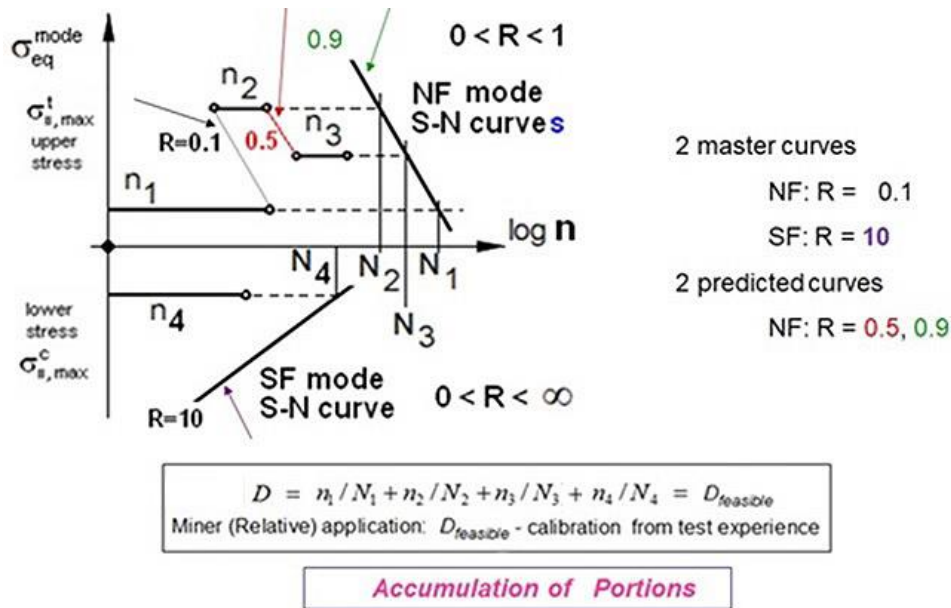


Fig.10-5: Lifetime Prediction (estimation) Method .Summing up of micro-damage portions by application of the Palmgren-Miner rule. Schematic application of a simple example, 4 blocks.

$D_{feasible}$ from test experience

LL:

- * A 'closed CFL-procedure' - as a coupled method - could be found to generate mandatory test data-based Constant Life Fatigue curves by using a Master SN curve plus the supporting model to determine other required SN-curves employing Kawai's Ψ -model
- * The challenging decay along $R_{trans} = -R^c / R'$ could be modelled (strength has a bias letter)
- * Test data along R_{trans} are more helpful than for $R = -1$, which is standard with ductile behavior
- * Right use of the right SFC. One cannot blame 'Mises' if yielding is not anymore decisive for the creation of the micro-damage portions
- * The Palmgren-Miner rule cannot account for loading sequence effects, residual stresses, and for stresses below the fatigue limit (life $\rightarrow \infty$?)
- * Viewing brittle materials, all the SN curves have their physical origin in the strength points.
- The author would like to recommend: Redirect the traditional 'Thinking, resulting from ductile material behavior regarding Mean stress correction' into a 'Thinking with fracture modes' in the case of the usually not fully ductile structural materials.

11 Evidencing 120°-symmetrical Failure Bodies of Brittle and Ductile Isotropic Materials

Aim: Structural Materials Building, Proof that ‘All isotropic materials possess 120° rotational symmetry’ with presentation of 3D-SFCs for isotropic, transversely isotropic UD-materials and orthotropic ones.

11.1 General

From experiments is known, that brittle isotropic materials possess a so-called 120°-axially symmetric fracture failure body in the compressive domain. The question arises: Should ductile materials in the tensile domain not also possess a 120°-axially symmetric yield loci envelope instead of having just the rotationally symmetric ‘Mises cylinder’?

According to the French saying ” Les extrêmes se touchent” and based on his FMC-thinking the author assumed that there is a large similarity in the description of the behavior of very ductile and very brittle materials. Also with ductile materials a 120°-rotational symmetry should be found. In order to prove the 120°-rotational symmetry, test results from bi-axially measuring test specimens are necessary, such as a cruciform or a cylinder.

Searched is the description of a complete failure body. This requires that the SFC captures both the positive and the negative I_1 -domain. Further, the 120°- rotational symmetry should be mapped by the SFC approach (*use of J_3*), too.

Thereby, brittle and ductile material behaviors are to discriminate:

Brittle: In order to show the difference of brittle to ductile materials *Fig. 11-1* outlines the brittle material with its features $\bar{R}^{tt} < \bar{R}^t$ and $\bar{R}^{cc} > \bar{R}^c$. (Probably not considering the natural flaws in concrete, in [Lem08] was published $\bar{R}^{tt} > \bar{R}^t$ which is physically not explainable and might be the consequence of the difficult measurement).

Ductile: Deformation measurements prove that for the same strain value of the growing yield surface it holds that equi-biaxial stress $\bar{\sigma}^{tt}(2D) > \bar{\sigma}^t(1D)$. This is similar to brittle concrete in the compressive domain where $\bar{R}^{cc} > \bar{R}^c$ and demonstrates the validity of the 120°-axial symmetry here, too.

Note:

Brittle: bi-axial tension = ‘weakest link failure behavior’ (schwächstes Glied'-Versagen)

Brittle: bi-axial compression = redundant (benign) failure behavior (Stützwirkung)

Ductile: bi-axial compression = redundant (benign) failure behavior (Stützwirkung).

11.2 Brittle Isotropic Materials (Metals, Glass, Ceramics, Concrete, Soil, ..)

2 modes → 2 SFCs, which is in line with the ‘generic’ number 2 according to the FMC.

3D-SFCs of Isotropic Dense Materials

| |
|--|
| <p>* Normal Fracture NF for $I_1 > 0$ \Leftarrow SFCs \Rightarrow Shear Fracture SF for $I_1 < 0$</p> $F^{NF} = c_{\Theta}^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t} = 1 \quad \leftrightarrow \quad F^{SF} = c_{\Theta}^{SF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{SF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^c} = 1$ <p style="text-align: center;">after inserting $\sigma = R \cdot Eff$ and dissolving for Eff follows</p> $Eff^{NF} = c_{\Theta}^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t} = \frac{\sigma_{eq}^{NF}}{\bar{R}^t} \quad \leftrightarrow \quad Eff^{SF} = c_{\Theta}^{SF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{SF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^c} = \frac{\sigma_{eq}^{SF}}{\bar{R}^c}$ |
|--|

The formulation of F^{NF} generates a straight line in the principal stress plane!

3D-SFCs of Isotropic Porous Materials with model parameter determination

* **Normal Fracture NF** for $I_1 > 0$ \Leftarrow SFCs \Rightarrow **Crushing Fracture CrF** for $I_1 < 0$

$$F^{NF} = c_{\Theta}^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t} = 1 \quad \leftrightarrow \quad F^{CrF} = c_{\Theta}^{CrF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{CrF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^c} = 1$$

after inserting $\sigma = R \cdot Eff$ and dissolving for Eff follows

$$Eff^{NF} = c_{\Theta}^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t} = \frac{\sigma_{eq}^{NF}}{\bar{R}^t} \quad \leftrightarrow \quad Eff^{CrF} = c_{\Theta}^{CrF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{CrF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^c} = \frac{\sigma_{eq}^{CrF}}{\bar{R}^c}$$

with $I_1 = (\sigma_I + \sigma_{II} + \sigma_{III}) = f(\boldsymbol{\sigma})$, $6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2 = f(\boldsymbol{\tau})$

$$27J_3 = (2\sigma_I - \sigma_{II} - \sigma_{III}) \cdot (2\sigma_{II} - \sigma_I - \sigma_{III}) \cdot (2\sigma_{III} - \sigma_I - \sigma_{II})$$

If a failure body is rotationally symmetric, then $\Theta = 1$ like for the neutral or shear meridian, respectively.

A 2-fold acting mode makes the rotationally symmetric fracture body 120°-symmetric and is modelled by using the invariant J_3 and Θ as non-circularity function with d as non-circularity parameter

$$\Theta^{NF} = \sqrt[3]{1 + d^{NF} \cdot \sin(3\vartheta)} = \sqrt[3]{1 + d^{NF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}} \quad \leftrightarrow \quad \Theta^{CrF} = \sqrt[3]{1 + d^{CrF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}}$$

Lode angle ϑ , here set as $\sin(3 \cdot \vartheta)$ with ‘neutral’ (shear meridian) angle $\vartheta = 0^\circ$ ($\rightarrow \Theta = 1, d = 0$);

tensile meridian angle $30^\circ \rightarrow \Theta^{NF} = \sqrt[3]{1 + d^{NF} \cdot (+1)}$; compr. mer. angle $-30^\circ \rightarrow \Theta^{CrF} = \sqrt[3]{1 + d^{CrF} \cdot (-1)}$.

Mode interaction \rightarrow Equation of the fracture body: $Eff = [(Eff^{NF})^m + (Eff^{CrF})^m]^{m^{-1}} = 1 = 100\%$

$$Eff = \sqrt[m]{\left(c_{1\Theta}^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t}\right)^m + \left(c_{\Theta}^{CrF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{CrF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^c}\right)^m} = 1$$

* 120°-rotat. symmetric $\Theta \neq 1$:

$c_{\Theta}^{NF} \rightarrow c^{NF} = 1$ ($\Theta^{NF} = 1$ in practice chosen).

c_{Θ}^{NF}, d^{NF} from the 2 points $(\bar{R}^t, 0, 0) \rightarrow c_{\Theta}^{NF}$ and $(\bar{R}^{tt}, \bar{R}^{tt}, 0) \rightarrow d^{NF}$ or min.error fit of data course

$c_{\Theta}^{CrF}, d^{CrF}$ from the 2 points $(-\bar{R}^c, 0, 0) \rightarrow c_{\Theta}^{CrF}$ and $(-\bar{R}^{cc}, -\bar{R}^{cc}, 0) \rightarrow d^{CrF}$.

The failure surface is closed at both the ends! A paraboloid serves as closing cap and bottom

$$\frac{I_1}{\sqrt{3} \cdot \bar{R}^t} = s^{cap} \cdot \left(\frac{\sqrt{2J_2 \cdot \Theta^{NF}}}{\bar{R}^t}\right)^2 + \frac{\max I_1}{\sqrt{3} \cdot \bar{R}^t}, \quad \frac{I_1}{\sqrt{3} \cdot \bar{R}^t} = s^{bot} \cdot \left(\frac{\sqrt{2J_2 \cdot \Theta^{CrF}}}{\bar{R}^t}\right)^2 + \frac{\min I_1}{\sqrt{3} \cdot \bar{R}^t}$$

Slope parameters s are determined connecting the respective hydrostatic strength point with the

associated point on the tensile and compressive meridian, $\max I_1$ must be assessed whereas $\min I_1$

can be measured. \bar{R}^t works as normalization strength. [CUN22, §5].

[Lem08] Lemnitzer L, Eckfeld L, Lindorf A and Curbach M (IfM TU Dresden): *Bi-axial tensile strength of concrete – Answers from statistics*. In: Walraven, J. C.; Stoelhorst, D. (Hrsg.): Tailor made concrete structures. New solutions for our society. Amsterdam, The Netherlands: CRC Press / Balkema, 2008, S. 1101-1102

In order to illustrate the various SFCs a 3D-concrete Fracture Body is presented: (more pictures of such fracture bodies are found in [CUN22]).

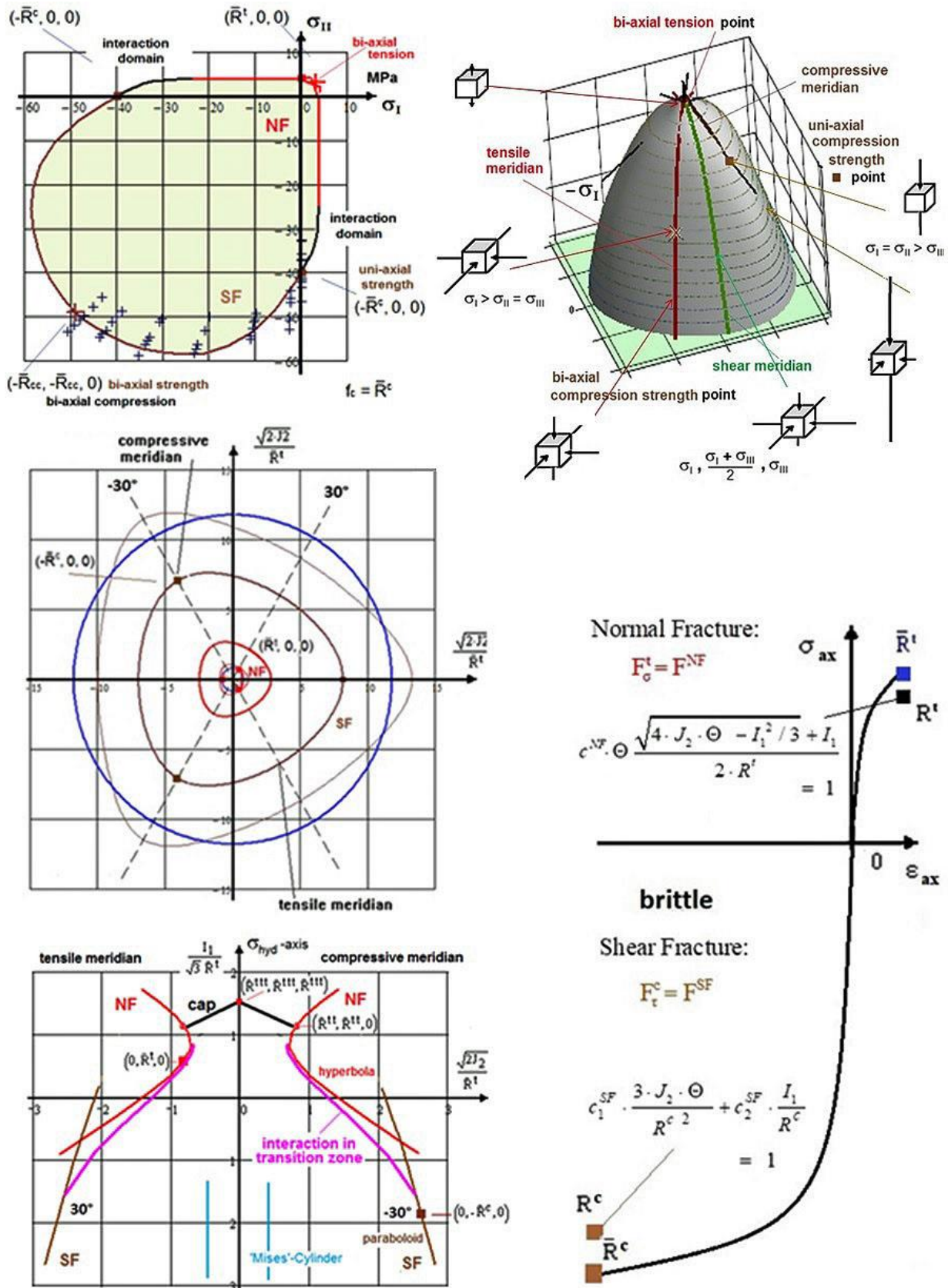


Fig.11-1: Visualization of the behavior of a brittle material (Normal Concrete) considering 1D stress-strain curve with 2D- and 3D-fracture failure curves and fracture body (surface). 120°-rotationally-symmetric

11.5 Ductile Materials, Metal

In *Fig.11-2(left)*, the failure body is presented with its meridians as axial lines. The center figure fully proves the general isotropic 120°-material rotational symmetry which is supported by the Mises ellipse being the *inclined cross-section of the Mises cylinder failure body* is added. The right octahedral figure shows the inner green curve with the Mises circle at the ‘Onset-of-yielding’ and the *outer one* at tensile strength R^t .

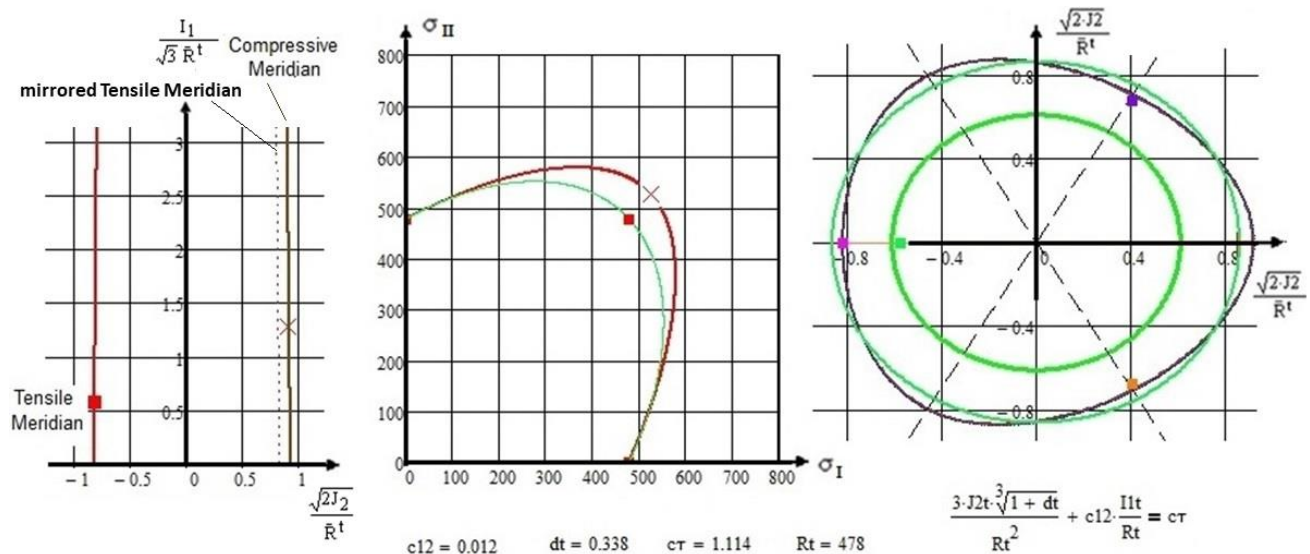


Fig.11-2, isotropic steel AA5182-0: Visualization of the behavior of a ductile material. (left) Yield body in Haigh-Lode-Wintergaard coordinates; (center) 120°-symmetry, visualized in the principal stress plane; (right) 120°-symmetry, visualized in the octahedral stress plane

The 120°-rotational symmetry can be best displayed in the octahedral stress plane which is a ‘horizontal’ cross-section of the failure body at a distinct I_1 , *Fig.11-2(right)*. The points and curves on the spatial body (*left figure*) are projected onto the octahedral plane (*right figure*). Since they depend on I_1 , they have different cross-section heights I_1 , such as the uniaxial tensile strength point which is located higher than the equi-biaxial strength point x .

In the center figure, Mises is the green curve; red square: the tensile strength point; cross: the equi-biaxial tensile strength point ductile ($trueR^{tt}$, $trueR^{tt}$, 0), i. e. the cross x . In the case of ductile metals it can be assumed $R^{tt} \cong 1.1 \cdot R^t$.

An elaboration of four materials with the Mathcad calculation program leads to the *Fig.11-3* below: *Fig.11-3(left)* presents curves through the uniaxial tensile strength points and the equi-biaxial strength R^{tt} . The curves are inclined cross-sections of the failure body. *Fig.11-3(right)*, for completion, displays the Beltrami potential surface (egg shaped), the ‘Mises’ cylinder and the three principal axes.

The figure shows extreme curve examples at $trueR^t$ level in the positive principal stress range. The red curve is occupied by the data of Kuwabara given below in the table, shown within *Fig.11-4*. The metal test data AA 5182-0 are from [Kuw98] T. Kuwabara et al: *Journal of Materials Processing Technology* 80–81 (1998) 517–523. Gotoh’s biquadratic yield criterion (not given here) was used to map the test data of the cold-rolled low-carbon steel AA 5182-0 sheets.

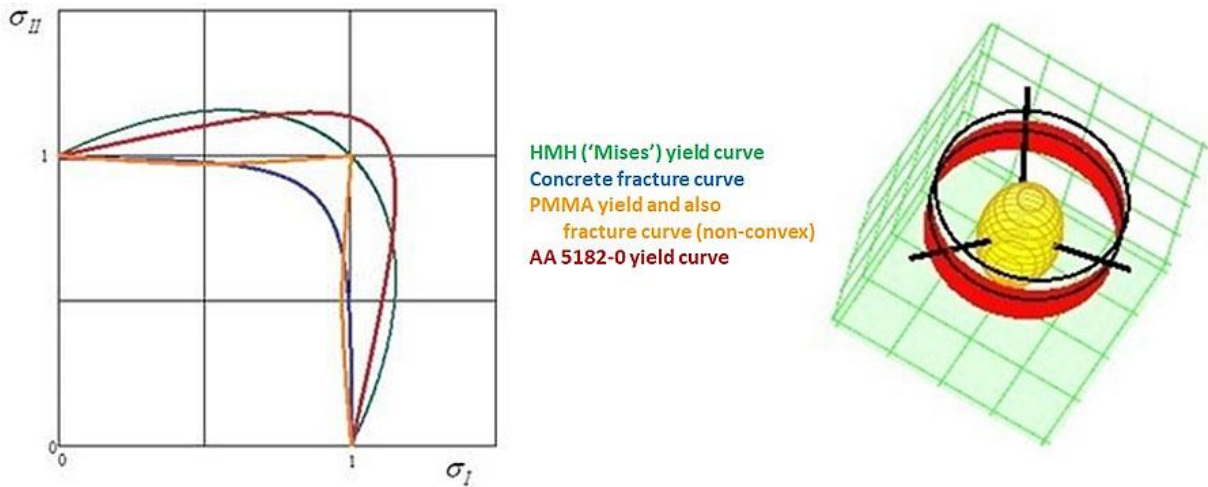


Fig.11-3: (left) Normalized principal stress plane failure curves of a set of fully different isotropic materials. (right) Failure body surface

Fig.11-4 depicts several failure cross-sections of an isotropic ductile steel demonstrating 120°-rotational symmetry like the brittle isotropic materials such as concrete in the compression domain and other ductile ones in the tensile domain.

For the generation of Fig.11-4 biaxial tensile tests of cold-rolled low-carbon steel sheet were carried out using flat cruciform specimens with the biaxial loads maintained in fixed proportion. Contours of plastic work (of flow potential) were determined in stress space under the shown strain range.

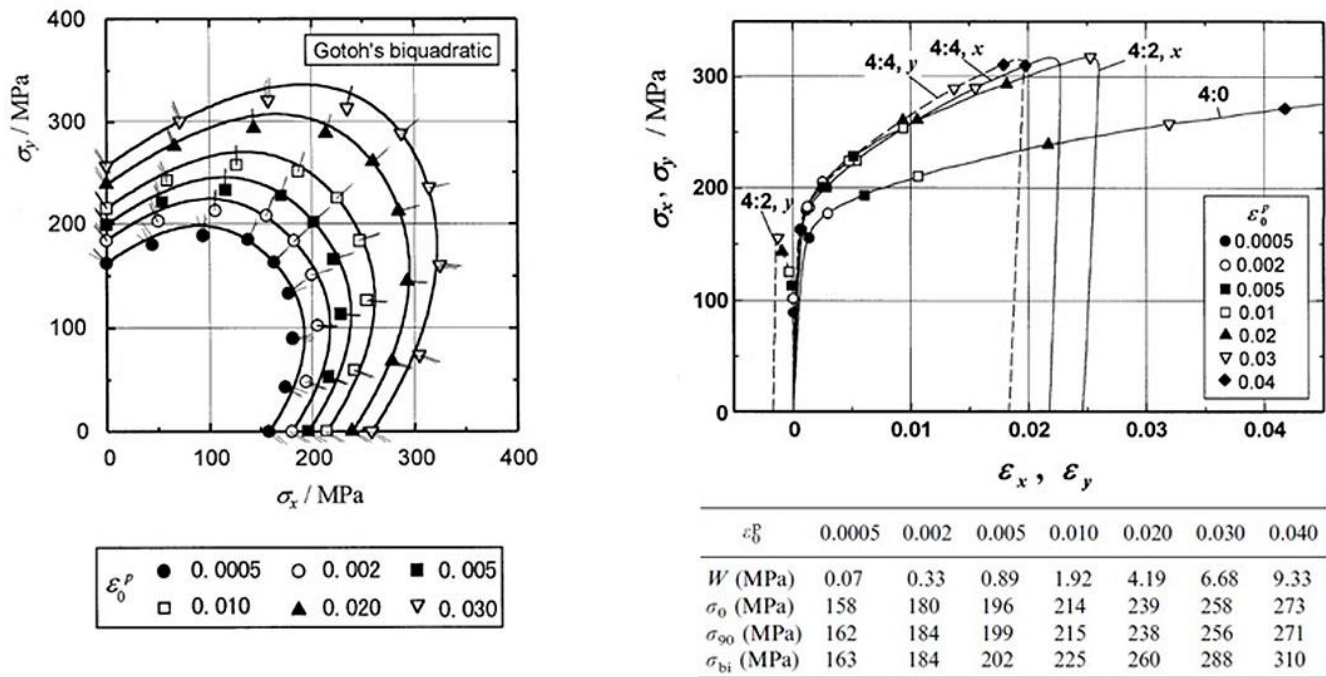


Fig.11-4: (left) Test points as function of the experienced plastic straining ϵ_0^{pl} ; mapping by using Gotoh's biquadratic criterion. (right) True stress-true strain curves for different biaxial loadings = different stress ratios. Measured values using r_0, r_{45}, r_{90} . $T = 1mm$, flat cruciform

LL:

- * *The author was able to map the course of all the corresponding courses of test data points with his isotropic SFC models.*
- * *Also for the ductile materials, the 120°-rotational symmetry was demonstrated, see further [CUN22, §5.8].*
- * *The 120°-rotational symmetry of isotropic materials is nothing else than a 'double mode effect, a two-fold danger'.*
- * *This effect is faced with all isotropic materials independent whether they are ductile or brittle.*

Reminder to illustrate elastic and plastic behavior:

- * Elastic deformation of crystalline structures occurs on the atomic scale: The bonds of the atoms in the crystal lattice are stretched. When de-loading, the energy stored within these bonds can be reversed. The material behaves elastic.
- * Plastic deformation or sliding occurs along gliding planes inter-crystalline or intra-crystalline and is permanent (plastic). No volumetric change is faced. 'Mises' applied.

12 Completion of the Strength Mechanics Building

Aim: Completion of a material-‘generic’ number driven Strength Mechanics Building

In the frame of his material symmetry-driven thoughts the author intended to test-proof some ideas that help to complete the Strength Mechanics Building by finding missing links and by providing engineering-practical strength criteria (SFCs), the parameters of which are directly measurable.

All this supports the assumption of a ‘generic’ number for the smeared-modelled materials.

The obtained Strength Mechanics Building matured, became clearer and more complete.

LL:

- ✓ *Beside the standard Shear (band) Yielding SY there also exists Normal Yielding NY analogous to the failure modes Shear Fracture SF and Normal Fracture NF (author assumption proven)*
- ✓ *120°-rotational symmetry is inherent to brittle and ductile isotropic materials (author assumption proven)*
- ✓ *Generic number 2, K_{Icr}^I with $K_{IIcr}^c : K_{IIcr}^c$ was theoretically proven for the non-real, ideal case of no flaws in front of crack tip*
- ✓ *Also in consequence of above building: Different but similar behaving materials can be basically treated with the same SFC. Examples are: Concrete ↔ foam, different fabrics.*

Material Symmetry seems to tell:

“In the case of ideally homogeneous materials a generic number is inherent. This is valid for elastic entities, yield modes and fracture modes, for yield strengths R_{02} and fracture strengths R , fracture toughness entities K_{cr} and for the invariants used to generate strength criteria”.

This **generic number** is
2 for isotropic and 5 for transversely–isotropic materials,

One might think:

“Mother Nature gives Strength Mechanics a mathematical order ! ?”

13 Safety Concept in Structural Engineering Disciplines

Aim: Providing basic knowledge for modeling, in order to pace the required finally necessary design verification of a component.

Exemplarily, the *designer* of a structure (e.g. aerospace) has to demonstrate to the *operator* (airline) and the *regulator* (airworthiness authority) compliance with the design requirements concerning Structural Integrity of flight hardware components such as: Stiffness, strength, vibration, fracture behaviour as well as to material selection, manufacturing process, hardware tests, inspection methods, quality assurance and documentation. This procedure is principally valid for other disciplines like civil engineering, too.

Structural Integrity of Hardware shall be proved by analyses and verified by tests under mission environmental conditions considering the complete life history of each item.

13.1 General with Mentioning the Old safety Concept

A *Safety Concept* means to implement reliability into the structural component by ‘capturing’ the uncertainty of the design parameters! It can just provide an unknown safety distance between load (‘stress’ S) and load resistance (‘strength’ R). *FoS* capture uncertainties, small inaccuracies, and simplifications in analyses w.r.t. manufacturing process, tolerances, loadings, material properties (strength, elasticity etc.), structural analysis, geometry, strength failure conditions. *FoS* do not capture missing accuracies in modeling, analysis, test data generation and test data evaluation!

In the *deterministic concepts* or *formats*, respectively, the worst case scenario is usually applied for loadings considering temperature, moisture, undetected damage. Further, a load is to increase by a ‘Design *FoS*’ and the resistances are to decrease. For the decrease of the strength, statistical distributions are used. If the loading is also based on a statistical distribution, then one speaks about a *semi-probabilistic format*.

Design Development was the basic work of the author in industry. This is why at first the Flow Chart below shall remind of the structural analysis tasks. There are basically four blocks, where – after the material Model Validations - the fulfillment of the Design Requirements has to be demonstrated for obtaining Design Verification as precondition of the final Certification Procedure.

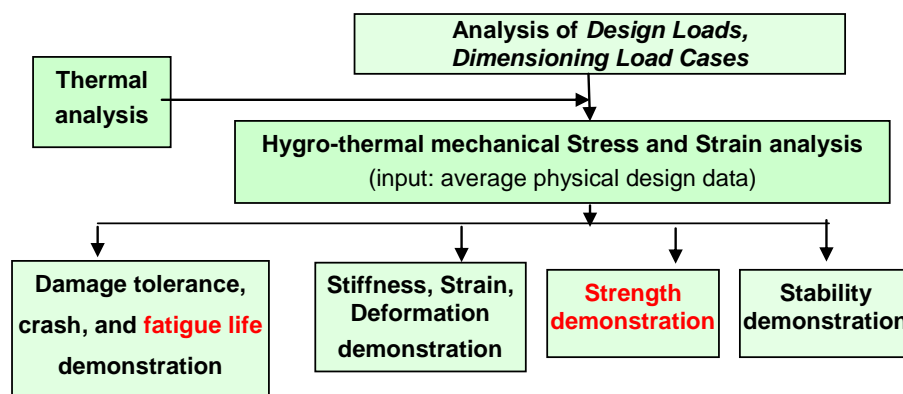


Fig13-1: Structural Design-Analysis Flow Chart

Essential question of engineers in mechanical and in civil engineering is: “How much could one further increase the loading“. Which is the reserve?

Old Safety Concept of Allowable Stresses:

At least since 1926 the civil engineer M. Mayer questioned the old safety concept, which used allowable stresses, meaning: resistance was reduced by a design safety factor.

This gives no accurate results in the case of non-linear behavior. In construction this was replaced since some decades in DIN 1054 by the Partial Safety Factor concept, which applies design safety factors and combination factors for general service loads, live loads, snow, ice loads, and wind loads. Temperature effects are specified in DIN 1055-100.

Material resistance must be generally demonstrated by a positive Margin of Safety MoS or a Reserve Factor $RF = MoS - 1 > 1$ in order to achieve Structural Integrity for the envisaged Design Limit State! A FoS is given and not to calculate (as it is too often to read even in FEA code manuals) like the Margin of Safety MoS or the Reserve Factor $RF = MoS + 1$.

Fig.13-2 visualizes the stress-strength distribution which outlines that the crossing over will determine the probability of failure p_f . Its value is the area of the p_f -distribution within the overlapping (gusset) of the stress and the strength distribution tails, see for details [CUN22, §16]

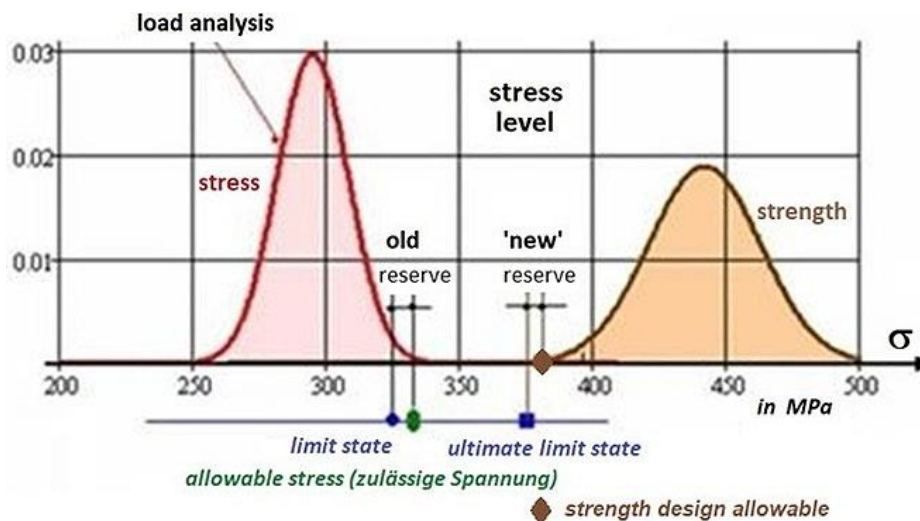


Fig.13-2: Visualization of the present ('new') and the old safety concept

LL:

The citation of the term 'allowable stress' is restricted to the former 'Concept of Allowable Stresses' and shall be not applied within present concepts anymore. Why? The usual application of the abbreviating term 'allowable' instead of 'strength design allowable may not confuse, but 'allowable stress' is error-prone because the relation below is valid:

$$j \cdot \text{allowable stress} = \text{strength design allowable} !! \text{ (see again the figure above) !}$$

13.2 Global (lumped) Factor of Safety Concept ('deterministic format') on Loading

Concept, that deterministically accounts for design uncertainties in a lumped (global) manner by enlarging the 'design limit loads' through multiplication with a design Factor of Safety FoS_j .

As still mentioned, FoS are applied to decrease the chance of failure by capturing the uncertainties of all the given variables outside the control of the designer. In the design process the scatter of individual values and parameters is usually treated by using fixed deterministic FoS , which act as load increasing multiplying factors FoS and should be called, more correctly, *Design FoS* .



*Personal Experience:
A safety distance pays off “.*

Comodo waran \approx 80 kg

Presently, in mechanical engineering the loading is increased by one lumped (global) FoS j , and in civil engineering the procedure was improved by using several partial Design FoS γ for the uncertain stochastic design variables. These *FoS* are based on long term minimum risk experience with structural testing. Depending on the risk consequences different classes of *FoS* are applied, e.g. for manned space-crafts higher *FoS* are used than for unmanned space-crafts.

Present spacecraft safety concept is an *improved global deterministic format (intention: semi-probabilistic)* = ‘Simplest’ Partial Safety Factor concept: It discriminates load model uncertainties considering factors (K_{Model} , $K_{Project}$) from design uncertainties which are considered by one global FoS j !

The to be applied values j for the *FoS* are risk or task driven. Facts to consider are:

- As mentioned exemplarily: Different application in cases of manned, un-manned spacecraft
- Design verification by ‘Analysis only’ (*by the way this is the usual case in construction*)
- Different risk acceptance attitude of the various industries.

Example: $DUL = j_{ult} \cdot \text{design limit load } DLL$

Mind: The virtual design value must be written DUL, because is the real test fracture load.

Different loading (action) FoS in aircraft and space engineering:

The first task in aerospace industry is load analysis. In any load analysis there are to establish all load events the structure is likely to experience in later application. This includes as well the estimation of loadings induced by the hygro-thermal, the mechanical (*static, cyclic and impact*) and the acoustical environment of the structure as further the corresponding lifetime requirements (*duration, number of cycles*), specified by an authority or a standard. Then, the so-called Design Limit Load values are determined, usually derived from mission simulations utilizing the so-called mathematical models of the full structure (*dynamical analyses, at first on basis of the preliminary design*).

When preparing the HSB sheet [Cun12] the author sorted out, that there practically is no different risk view between air-craft and space-craft:

- * Spacecraft: using a *dynamic Limit Load* model obtaining a basic load prediction dLL considering a load model uncertainty considering factor $j_{LM} = 1.2$. This delivers a Design Limit Load $DLL = 1.2 \cdot dLL$, and from this follows $DUL = dLL \cdot j_{LM} \cdot j_{ult}$, with $1.2 \cdot 1.25 = 1.5$! The DLL level is applied in spacecraft in fatigue life demonstration.

* Aircraft: Definition of a so-called (design) Limit Load LL delivering $DUL = LL \cdot 1.5$.

LL: *The author could conclude after comparing the ESA/ESTEC aerospace Standards (the author had to work on), that the DUL-value is practically the same value in aircraft and in spacecraft !*

The resistance strength and the bearable loads (at joints etc ...):

Dependent on the design requirements the average, the upper or a lower value of the property is used for the various properties. In the case of strength a statistically reduced value R . To achieve a reliable design the so-called Design Allowable has to be applied. It is a value, beyond which at least 99% (“A”-value) or 90% (“B”-value) of the population of values is expected to fall, with a 95% confidence (*on test data achievement*) level, see MIL-HDBK 17. A “B”-value is permitted to use for multi-layered, redundant laminates.

Bearable loads require series tests of the distinctive structural component with statistical evaluation in order to determine the ‘load-resistance design allowables’.

Measurement data sets are the result of a Test Agreement (norm or standard), that serve the desire to make a comparability of different test procedure results possible. The Test Agreement consists of test rig, test specification, test specimen and test data evaluation method and the Test Procedure. Therefore, one can only speak about ‘*exact test results in the frame of the obtained test quality*’. Hence, there are no exact property values.

Test specimens shall be manufactured like the structure (‘as-built’).

Considering property input: When applying test data from ‘isolated lamina’ test specimens (*like tensile coupons*) to an embedded lamina of a laminate one should consider that coupon test deliver tests results of ‘weakest link’ type. An embedded or even an only one-sided constrained lamina, however, possesses redundant behavior → “B”-values permitted.

Reserve Factor RF and Margin of Safety MoS : Formulas:

Linear analysis is sufficient (presumption): $\sigma \sim \text{load} \Rightarrow RF \equiv f_{RF} = 1 / Eff$

$$\text{Material Reserve Factor} \quad f_{RF, ult} = \frac{\text{Strength Design Allowable } R}{\text{Stress at } j_{ult} \cdot \text{Design Limit Load}} > 1,$$

Non-linear analysis required: σ not proportional to load

$$\text{Reserve Factor (load-defined)} \quad RF_{ult} = \frac{\text{Predicted Failure Load at } Eff = 100\%}{j_{ult} \cdot \text{Design Limit Load}} > 1.$$

LL:

- * A *FoS* is given and not to calculate such as a *Margin of Safety MoS* or the *Reserve Factor RF* = $MoS + 1$.
- * A *MoS* is usually the result of worst case assumptions that does not take care of the joint actions of the stochastic design parameters and thereby cannot take care of their joint failure action and probability. This failure probability is a ‘joint failure probability’ because it considers the probability of joint acting
- * A material with a high coefficient of variation *CoV* disqualifies itself, when computing the statistically-based strength design allowable values. Therefore, one must not penalize it further as performed in some standards in the past in the case of new materials.
- * Both, an increasing mean value and a decreasing standard deviation will lower p_f

- * The MoS value does not outline a failure probability. Failure probability p_f does not dramatically increase if MoS turns slightly negative
- * A local safety measure of MoS = -1 % is no problem in design development if a 'Think (about) Uncertainties' attitude is developed in order to recognize the main driving design parameters and to reduce the scatter (uncertainty) of them
- * Nowadays often non-linear analyses are performed, delivering true quantities, however Design Verification is executed with engineering strength values R . Why do we not use in such a case the true tensile strength, but calculate f_{RF} with four numbers accuracy?
- * Fig.13-3 (left) visualizes strength distribution, Eff versus micro-damage growth and material reserve factor f_{RF}
- * True-in requires True-out and an assessment by $true\bar{R}^t$. The Fig.13-3 (right) shows for an aluminum alloy a difference between the mean (material model) strength values $eng\bar{R}^t \rightarrow true\bar{R}^t$ of 8%.

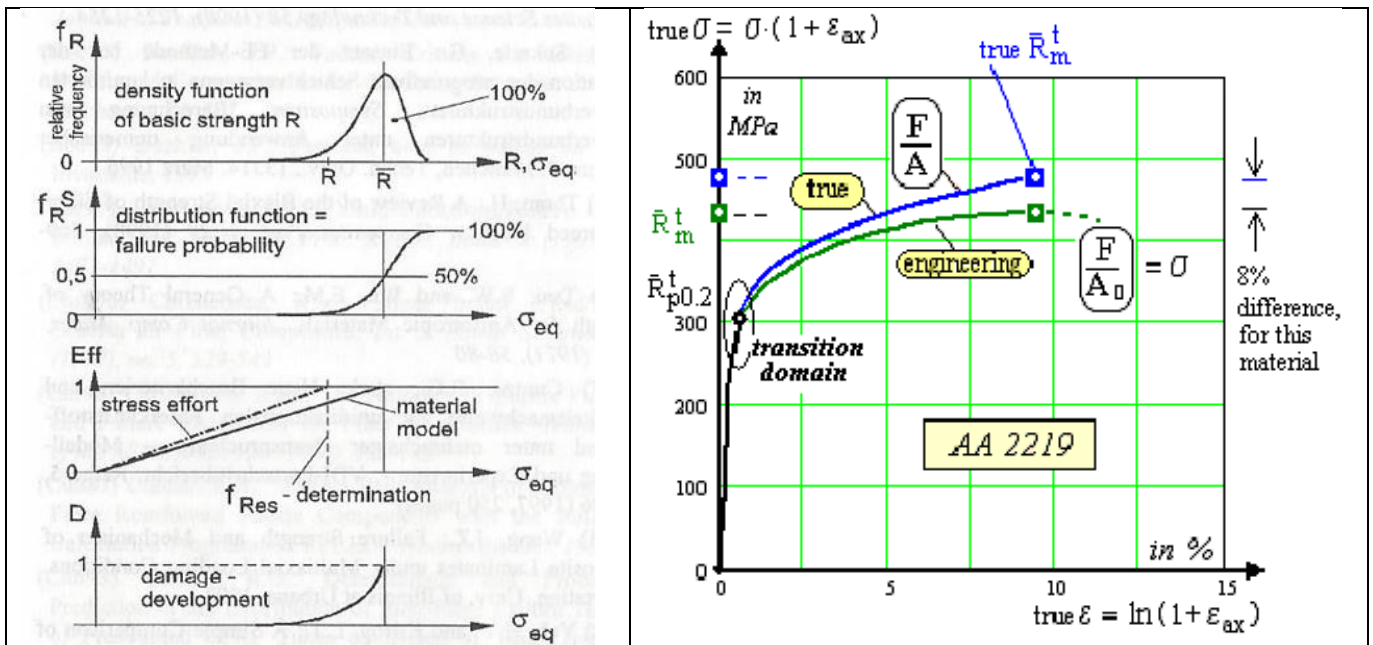


Fig.13-3: (left) Design quantities when approaching failure in Design Verification . (right) Difference engineering and true tensile strength of AA2219

Robust Design Requirements:

The goal of any design engineer should be to end up with a robust design. In order to achieve this, the main stochastic design parameters have to be used to outline the robustness of the design against the envisaged actual failure mode by firstly computing the sensitivity measures α and then investigating the reduction of the design's sensitivity to changes of X_j while keeping p_f at the prescribed level. This is important for the production tolerances. Probabilistic design may be used as an assessment of the deterministic design or is necessary as design method if a *reliability* target \mathcal{R} is assigned instead of a *FoS*. or its complement, the probability of failure p_f .

A structural reliability analysis in a Hot Spot reveals the influence of each stochastic design parameter on the distinct failure mode by means of the sensitivity measures. Robust designs (*robust*

to later changes of the design parameters) are required with identification of the most sensitive design parameters!

For better illustration of the Safety Concepts from [CUN22, §12] the Fig.13-4 is included. It clearly depicts the definition of the failure probability in this two-parameter case.

Design advantages found with the Ariane Booster design, when using a probabilistic tool:

Two advantageous applications of the probabilistic tool shall be shortly demonstrated where probabilistic modelling and computation were successfully applied:

* A reduced production tolerance width leads to a reduced mass which sequentially reduced further fuel mass savings. Improved production reduced the wall thickness tolerance from 8.2 +/- 0.20 mm to 8.2 +/- 0.05 mm. Keeping the same given reliability value $\mathfrak{R} = 1 - p_f = 1 - 5 \cdot 10^{-6}$ the nominal wall thickness could be set $\rightarrow 8.1 \pm 0.05$ mm leading to mass and fuel savings. *(As early as 1985 for our pre-design of the Ariane 5 launcher so-called target survival probabilities \mathfrak{R} were fixed for the several structural parts!)*

* Probabilistic modelling of the geometrical tolerances of bore hole, pin, position (pitch) and strength minimum restrains with minimum residual stresses could be achieved, for the pin connection an optimum number of pins of 130 pins for a simpler assembly process and for reduced mounting stresses.

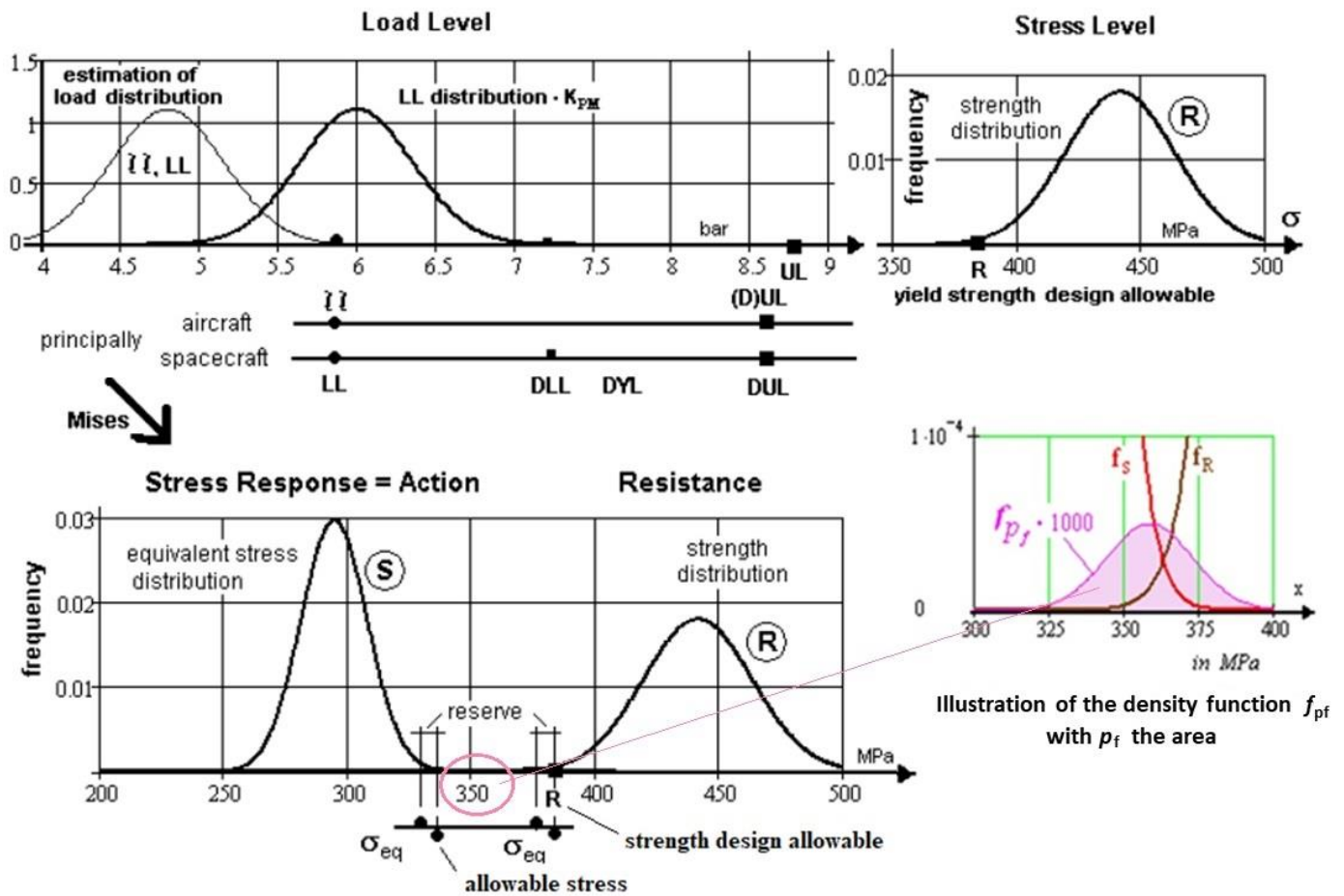


Fig.13-4: Visualization of the difference of the aerospace load terms used in the Strength Design Allowable Safety Concept and of the 'hopefully forgotten' Allowable Stress Safety Concept

Fig.13-5 presents a numerical example how the reserve factor RF is to compute.

Assumption: Linear analysis permitted, design FoS $j_{ult} = 1.25$

* Design loading (action): $\{\sigma\}_{design} = \{\sigma\} \cdot j_{ult}$

* 2D-stress state: $\{\sigma\}_{design} = (\sigma_1, \sigma_2, \sigma_3, \tau_{23}, \tau_{31}, \tau_{21})^T \cdot j_{ult} = (0, -76, 0, 0, 0, 52)^T$ MPa

* Residual stresses: 0 (effect vanishes with increasing micro-cracking)

* Strengths (resistance) : $\{\bar{R}\} = (1378, 950, 40, 125, 97)^T$ MPa average from measurement
 statistically reduced $\{R\} = (R_{||}^t, R_{||}^c, R_{\perp}^t, R_{\perp}^c, R_{\perp||})^T = (1050, 725, 32, 112, 79)^T$ MPa

* Friction value(s) : $\mu_{\perp||} = 0.3, (\mu_{\perp\perp} = 0.35)$, Mode interaction exponent: $m = 2.7$

$\{Eff^{mode}\} = (Eff^{||\sigma}, Eff^{||\tau}, Eff^{\perp\sigma}, Eff^{\perp\tau}, Eff^{\perp||})^T = (0.88, 0, 0, 0.21, 0.20)^T$

$Eff^m = (Eff^{||\sigma})^m + (Eff^{||\tau})^m + (Eff^{\perp\sigma})^m + (Eff^{\perp\tau})^m + (Eff^{\perp||})^m = 100\%$.

The results above deliver the following material reserve factor $f_{RF} = 1 / Eff$

* $Eff^{\perp\sigma} = \frac{\sigma_2 + |\sigma_2|}{2 \cdot R_{\perp}^t} = 0, \quad Eff^{\perp\tau} = \frac{-\sigma_2 + |\sigma_2|}{2 \cdot R_{\perp}^c} = 0.60, \quad Eff^{\perp||} = \frac{|\tau_{21}|}{\bar{R}_{\perp||} - \mu_{\perp||} \cdot \sigma_2} = 0.55$

$Eff = [(Eff^{\perp\sigma})^m + (Eff^{\perp\tau})^m + (Eff^{\perp||})^m]^{1/m} = 0.80.$

$\Rightarrow f_{RF} = 1 / Eff = 1.25 \rightarrow RF = f_{RF}$ (if linearity permitted) $\rightarrow MoS = RF - 1 = 0.25 > 0 !$

Fig.13-5: Computation of a Reserve Factor RF

14 Nonlinear Stress-Strain relationships, Beltrami Theory with Change of Poisson's Ratio ν

Aim: Provision of a Basis to generate an 'Extended Mises' model as a simplified 'Gurson' model.

14.0 General on Stress-Strain curves $\sigma(\epsilon)$, Strengths R and Poisson's Ratio ν

There are two different stress-strain curves existing: the *monotonic* and the *cyclic stress-strain curve*. The first curve is derived by the static tests, whereas the second one is generated by fatigue tests. Strain-controlled cyclic hysteresis loops (*Fig.14-1, left down*) are performed on different strain levels with several test specimens. Dependent on hardening and softening behavior of the actual material these two curves may discriminate significantly. Monotonic stress-strain curves have long been used to obtain design parameters for limitation of the stresses in engineering structures subjected to static loading. Similarly, cyclic stress-strain curves are useful for assessing the durability of structures subjected to repeated loading.

Further, in the case of monotonic σ - ϵ -curves there are very different, material-specific stress-strain curves in the elastic-plastic transition domain, see *Fig.14-1, left up and right*. Some show an 'Onset-of-yield' at an upper yield stress level \bar{R}_e^{upper} and others at a lower yield strength \bar{R}_e^{lower} . In this case usually the lower yield point is taken as the yield strength of the metal.

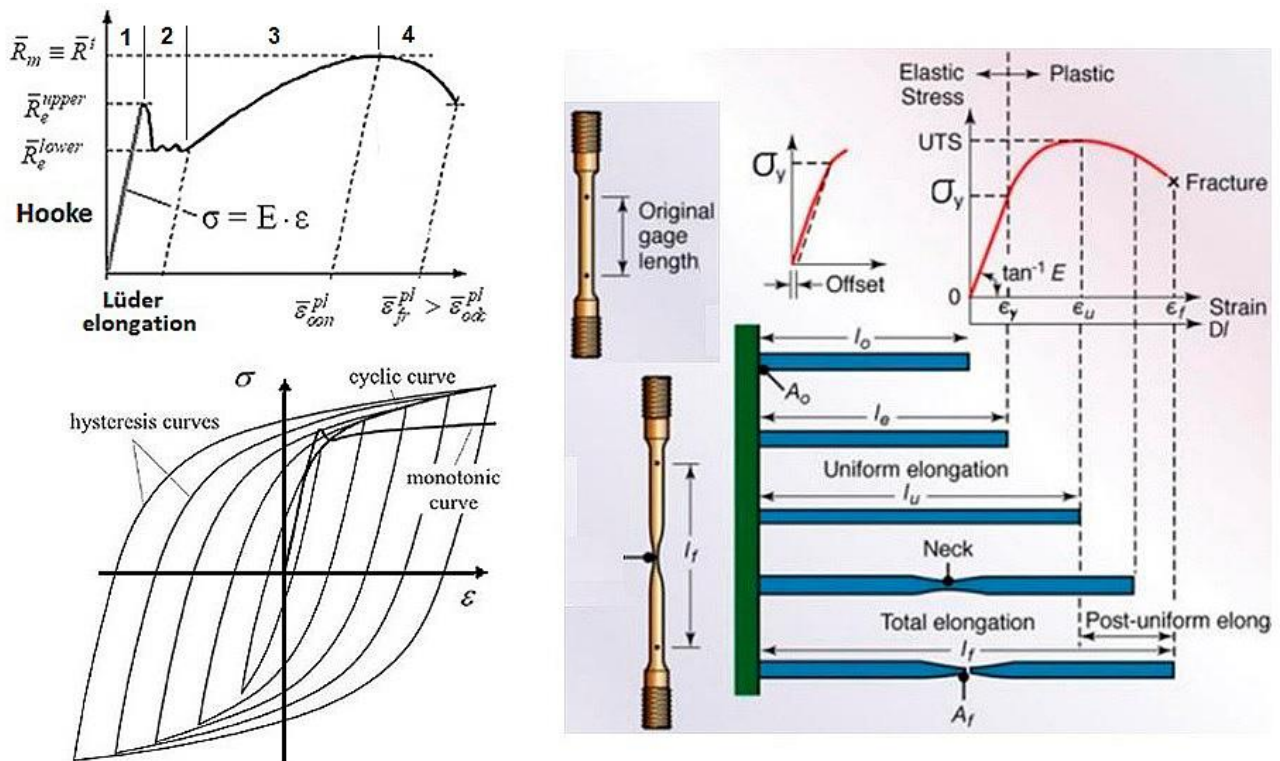


Fig.14-1, engineering quantities. modelling: (left,up) Discontinuous yielding, mean curve for mild steel showing the yield point phenomenon, termed Lüder's elongation effect. (left, down) Cyclic curves. (right) Tensile-test specimen with gage length, elongation before and after testing and finally after rupture (from Kalpakjian S and Schmid S: Evaluation of the Possibility of Estimating Cyclic Stress-strain Para-meters and Curves from Monotonic Properties of Steels. Manufacturing Engineering & Technology. 2013

For the 'left up'- metals in the paper of Hai Qiu and Tadanobu Inoue: *Evolution of Poisson's Ratio in the Tension Process of Low-Carbon Hot-Rolled Steel with Discontinuous Yielding. Metals* **2023**, *13*, 562. <https://doi.org/10.3390/met13030562> four different regimes are distinguished: Phase 1: Uniform elastic elongation, Phase 2: Discontinuous yielding, Phase 3 beyond R_{02} : Uniform elongation in the

hardening regime, Phase 4 beyond \bar{R}^t : Macroscopic plastic-strain localization experiencing radial deformation. Low-alloy iron usually has such an upper yield limit R_e^{upper} (R_{eH} , *Streckgrenze*). If it is stretched during the tensile test, a spontaneous yielding in the crystals-compound takes place under loading. This so-called Lüder's elongation effect of mild metals as a part of plastic stretching disappears until all crystals are finally commonly stretched. Austenitic steels do not have a pronounced yield strength.

Essential for an accurate analysis is a stress-strain curve which is derived from a set of test curves, delivering distributions for the *design parameters* $R_{p0.2}$, R_m , ε_{on}^{pl} and ε_{fr}^{pl} .

The yield strength is a material property defined as the stress at which a material begins to deform plastically. If it is not well-defined (*remind Lüder*) on the stress-strain curve, it is difficult to determine a precise onset-of-yield point. In general, discriminating the proportional tensile limit R_{prop} and $R_{p0.2}$ ($\equiv R_{0.2}^t$), the offset yield point is taken as the stress at which 0.2% plastic deformation remains (*in English literature* $R_{p0.2}$ is termed *proof stress*). The mean stress at Onset-of-Yielding, denoted $\bar{R}_{0.2}$ will be applied for ductile modeling. The stress $\sigma(\varepsilon_{pl})$, considering only the plastic deformation or plastic flow of the material, is termed Flow stress σ_F .

By the way, the actual 'Onset-of-yielding at $R_{prop} \equiv \sigma_{prop}$ can be determined by a temperature measurement. If a metallic material is subjected to tensile stress, it first cools down in the area of elastic elongation analogous to an ideal gas, thermo-elastic effect. With onset of plasticization heat is released, which leads to an increase in temperature. This temperature is measurable with glued thermocouples. In other words: The proportionality stress σ_{prop} can be allocated to that applied stress level, where the test specimen experiences a temperature increase due to internal dislocations.

Regarding not only metals - for a conflict-free understanding – it will be denoted $R_{p0.2}$ ($\rightarrow R_{0.2}^t$) and $R_{c0.2}$ ($\rightarrow R_{0.2}^c$) in the body text from now on. At the maximum of the curve, characterized by the so-called 'End-of-uniform elongation' = 'Onset-of-(ductile) necking' in the ductile material case, the tensile strength R_m ($\rightarrow R^t$) is given. For very ductile materials is valid $R_{0.2}^c \cong R_{0.2}^t$.

Beyond the tensile strength R^t a multiaxial state of stress follows in the tensioned ductile behaving test specimen. Therefore, the index ax holds up to the 'End-of-uniform elongation' (Gleichmaßdehnung) at R^t (*index pl for plastic strain, on for Onset-of-(ductile) necking, and odc for Onset-of-ductile cracking located before rupture = plastic collapse*). In this respect, any formulations in this domain afford equivalent quantities in order to perform an accurate non-linear analysis with a correct $\sigma(\varepsilon)$ -input.

14.2 Engineering and True Stress and Strain Quantities

The larger the strains the more the engineering quantities lose their applicability in structural dimensioning. Therefore, logarithmic (*usually termed true*) strains have to be used in an accurate dimensioning process. The derivation of these quantities is collected in [Table 14-1](#).

[Fig.14-2](#) contains a true and an engineering stress-strain curve. The figure presents a general view and uses classical Ramberg-Osgood mapping. Mapping of the course of stress-strain data in the non-linear domain is well performed by taking the usually applied Ramberg-Osgood equation for the true stress-true strain curve (*maps the true curve better than the engineering curve*)

$$\text{eng}\varepsilon = \varepsilon = \varepsilon^{\text{el}} + \varepsilon^{\text{pl}} = \frac{\sigma}{E_0} + 0.002 \cdot \left(\frac{\sigma}{\bar{R}_{0.2}} \right)^{\bar{n}} \quad \text{and} \quad \text{true}\varepsilon = \frac{\text{true}\sigma}{E_0} + 0.002 \cdot \left(\frac{\text{true}\sigma}{\text{true}\bar{R}_{0.2}} \right)^{\text{true}\bar{n}}$$

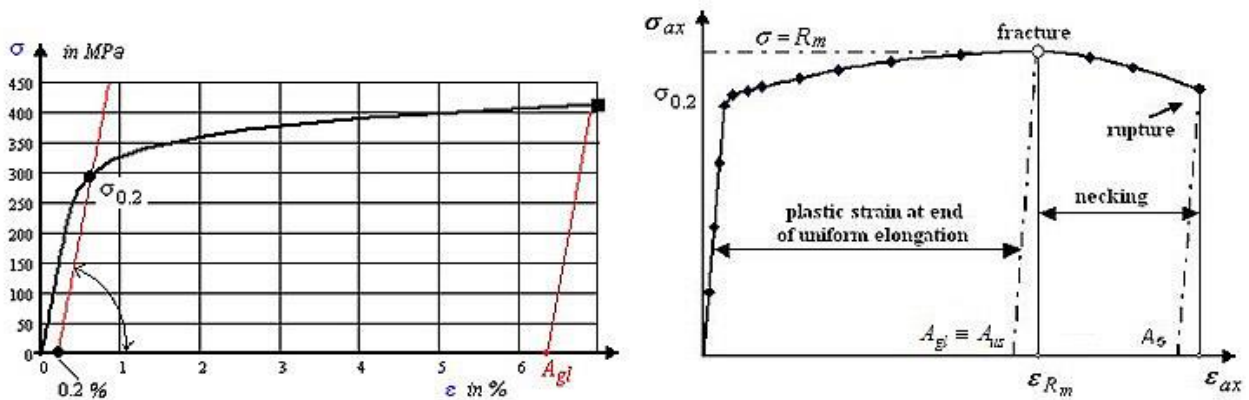


Fig.14-2: R-O mapping of a single engineering measurement test results, $A_{gl} = \min \varepsilon_{oon}^{pl}$.

Typical (mean) engineering stress-strain curve of a distinct ductile metal material. End of uniform elongation (Gleichmassdehnung ε_{gl})

Table 14-1 presents the derivation of true stresses and true strains in the ‘Mises’-validity domain. In Fig.13-4 the difference between the mean strength values $\text{eng}\bar{R}^t \rightarrow \text{true}\bar{R}^t$ was shown to be 8% for AA2219! Fig.14-3(left) depicts the linear elastic proportional domain and the hardening domain. Fig.14-3(right) presents stress-strain measurement with Ramberg-Osgood mapping. The course of the area reduction would show a slight kink beginning at ‘Onset-of-ductile cracking odc ’ (= onset-of-localized necking) according to the deteriorating effect of the void coalescence.

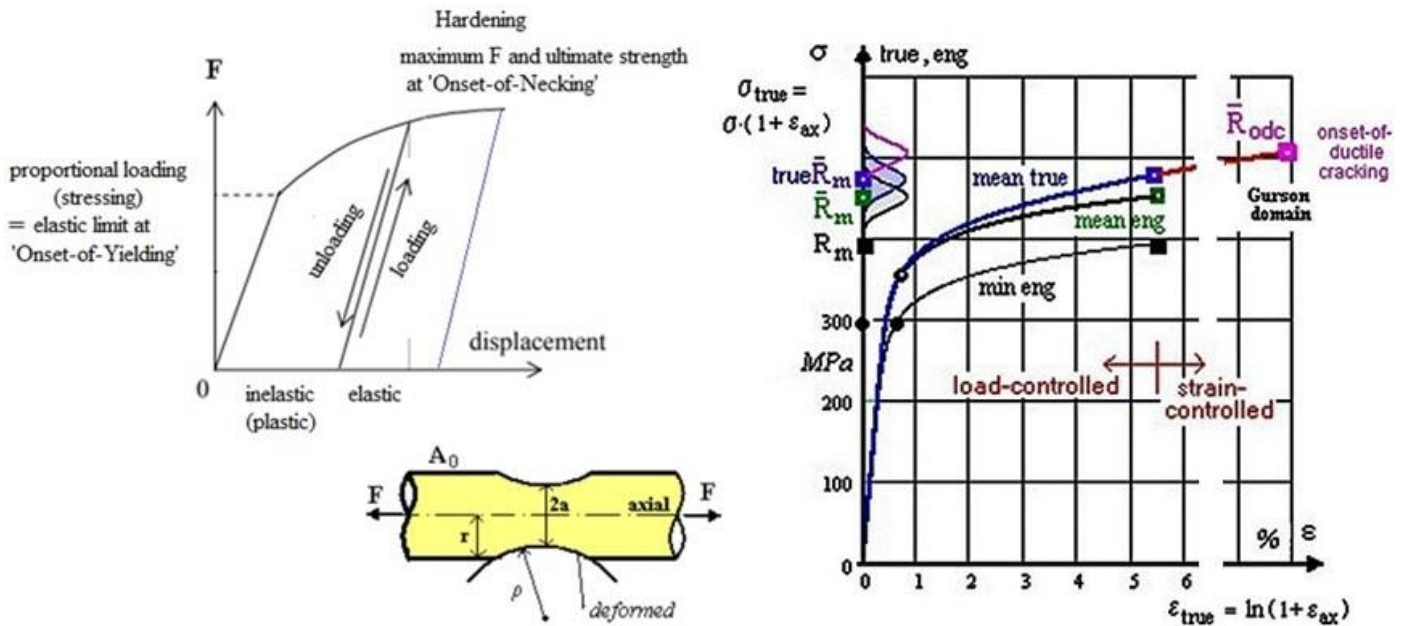


Fig. 14-3, modelling: (left) Display of proportional domain and hardening domain with the tensile rod test specimen. (right) Ramberg-Osgood-mapped true and engineering stress-strain curves of AA2219. F := Force F_{ax} A_0 := original cross-section, A := actual cross section of the necked rod. $\bar{R}^t = \max F / A_0$, $\varepsilon \leq \bar{A}_{gl}$ (permanent strain linked to load-controlled fracture at \bar{R}^t). Necking radius is ρ . A bar over \bar{R} indicates a mean (average) value of a sufficiently large test data set, and no bar over R will generally mean strength and later indicate a ‘strength design allowable’.

Table 14-1: Derivation of true stresses and true strains in the 'Mises'-validity domain

True Strains (logarithmic strains):

The application of engineering strain cannot be correct for larger strains, since it is based on the original gage length ℓ_0 , whereas the length is continuously growing. Ludwik [Lud09] therefore introduced the true strain (logarithmic strain), the increment of which for a given length is defined as $d(\text{true}\varepsilon) = d\ell / \ell$ and the total true strain, integrated from ℓ_0 to current length ℓ , is

$$\text{true}\varepsilon_{ax} = \int_{\ell_0}^{\ell} d\ell / \ell = \ln(\ell / \ell_0) = \ln(1 + \text{eng}\varepsilon_{ax}).$$

Above equation delivers an accurate value up to 'onset-of-necking' or \bar{R}^t .

The replacement of the logarithmic function by a Taylor series

$$\text{true}\varepsilon_{ax} = \text{eng}\varepsilon_{ax} - \text{eng}\varepsilon_{ax}^2 / 2 + \text{eng}\varepsilon_{ax}^3 / 3 - \dots$$

clearly shows that identity is given for small strains, only. Applying the true strain has a physical and a numerical advantage: The incompressibility equation really becomes zero

$$\sum \text{true}\varepsilon_i = \text{true}\varepsilon_I + \text{true}\varepsilon_{II} + \text{true}\varepsilon_{III} = 0,$$

whereas in terms of engineering strains the correct equation from solid geometry reads

$$(1 + \text{eng}\varepsilon_I) \cdot (1 + \text{eng}\varepsilon_{II}) \cdot (1 + \text{eng}\varepsilon_{III}) - 1 = 0,$$

which reduces to 0 for negligible strains, only.

Once necking starts most of the deformation occurs in the smallest cross section. The longer the gage length used the smaller the percent elongation will be. Therefore, a better procedure is the measurement of the reduction of the cross-section. → Beyond \bar{R}^t , the true σ - ε curve can be more accurately obtained by measuring the radial strain

$$\text{eng}\varepsilon_{radial} = (r - r_0) / r_0 = r / r_0 - 1 \quad \text{and} \quad \text{true}\varepsilon_{radial} = -\ln(1 + \text{eng}\varepsilon_{radial}) = -\ln(r / r_0),$$

provided, the tensile test specimen has a circular cross-section, a rod. In this case $\varepsilon_{radial} = \varepsilon_{hoop}$

$$\text{true}\varepsilon_{ax} + \text{true}\varepsilon_{radial} + \text{true}\varepsilon_{hoop} = 0 \quad \text{and it holds} \quad \text{true}\varepsilon_{ax} = -2\text{true}\varepsilon_{radial} = 2\ln(r / r_0),$$

which delivers an accurate value above 'onset-of-necking'. The equivalent strain in the center reads

$$\begin{aligned} \text{true}\varepsilon_{eq} &= \frac{\sqrt{2}}{3} \cdot \sqrt{(\text{true}\varepsilon_{ax} - \text{true}\varepsilon_{hoop})^2 + 0 + (\text{true}\varepsilon_{hoop} - \text{true}\varepsilon_{ax})^2} \\ &= \frac{\sqrt{2}}{3} \cdot \sqrt{2(2 - (-1))^2} \cdot \text{true}\varepsilon_{radial} = \frac{2}{3} \cdot \sqrt{3^2} \cdot \text{true}\varepsilon_{radial} = 2\ln(r / r_0) \end{aligned}$$

$$\Rightarrow \text{Transferring strain data:} \quad \text{true}\varepsilon = \ln(1 + \text{eng}\varepsilon), \quad \text{eng}\varepsilon = e^{\text{true}\varepsilon} - 1.$$

True Stresses

True σ can be obtained from $\text{eng}\sigma$, if the small changes in volume at the end of the transition domain are neglected. Then, incompressibility $\sum \varepsilon_i^{pl} = 0$ can be assumed and it follows:

$$\text{eng}\sigma = F / A_0, \quad \text{true}\sigma = F / A \quad \text{with} \quad A \cdot \ell = A_0 \cdot \ell_0, \quad F: = \text{load } F_{ax}$$

wherein ℓ_0 := original gage length, and A, ℓ current values of the necking cross-section.

Introducing the equation $\varepsilon_{ax} = (\ell - \ell_0) / \ell_0$ derived above, the true stress is linked by

$$\text{true}\sigma_{ax} = F / A = (F / A_0) \cdot (\ell / \ell_0) = \text{eng}\sigma \cdot (1 + \text{eng}\varepsilon_{ax}) = \sigma_{ax} \cdot (1 + \varepsilon_{ax}) \quad \text{usually written}$$

$$\Rightarrow \text{Transferring stress data:} \quad \text{true}\sigma = \text{eng}\sigma \cdot (1 + \text{eng}\varepsilon) \quad \text{and} \quad \text{eng}\sigma = \text{true}\sigma / \exp(\text{true}\varepsilon).$$

Fig.14-4 (left) shows an experiment in the elastic-plastic transition region, carried out by O. Mahrenholtz /H. Ismar. The test was a flat compression test of a cube: One side constrained, one free, one compressed \rightarrow Principal stress state ($\sigma_I = \sigma_{\text{action}}, \sigma_{II} = \sigma_{I(\text{re-action})}, \sigma_{III} = 0$) \rightarrow principal strain $\rightarrow v$. It turns out that R_{p01} is approximately $v = 0.4$. The value at R_{p02} in Lode coordinates is $0.82 = \sqrt{2/3} = \sqrt{2J_2} / R_{02}$, with $J_2 = 2R_{02} / 6$ (left, down). Poisson's ratio, determined by a coupon measurement, reads $v = -\epsilon_{\text{lat}}/\epsilon_{\text{ax}}$ or $v = -(\Delta d/d)/(\Delta \ell/\ell)$.

Concerning sheet test specimens the measurement problem increases because localized necking will occur at onset-of-ductile cracking and this depends on the thickness of the test specimen.

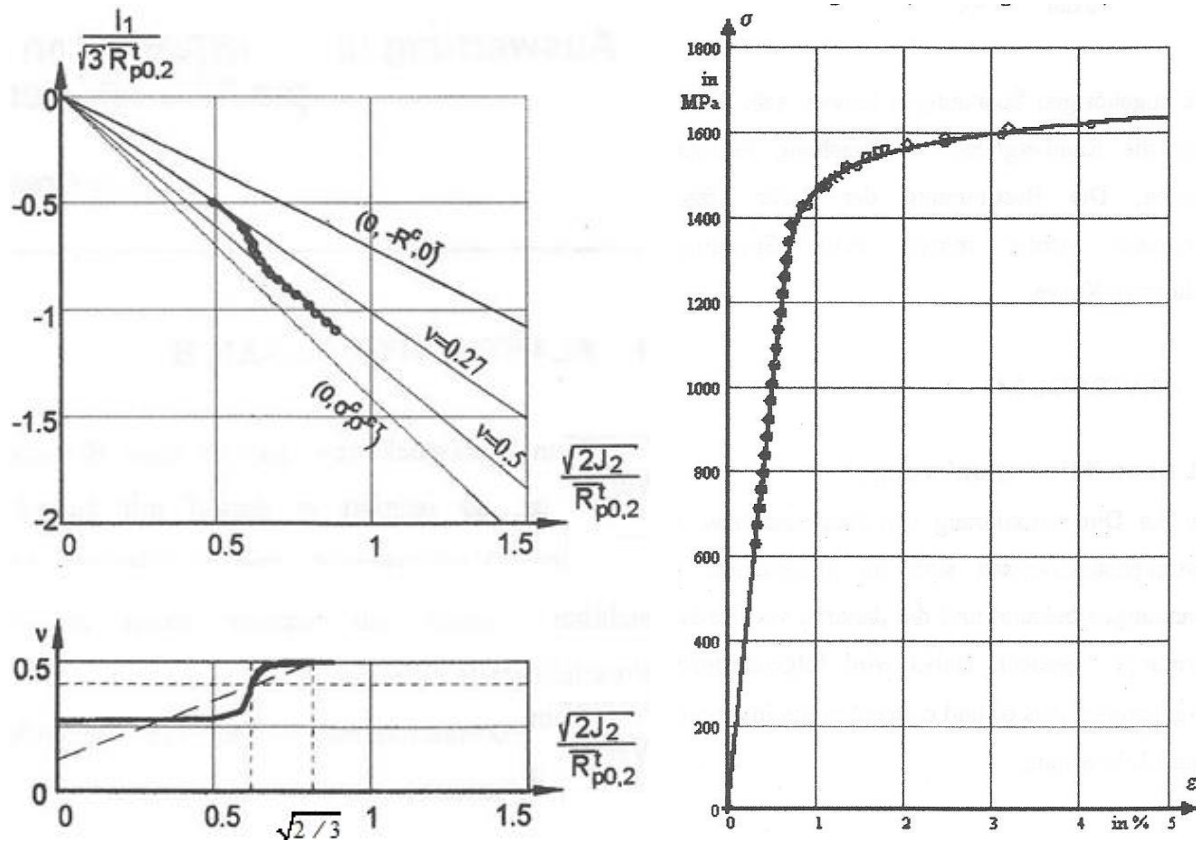


Fig. 14-4: (left) St37 Development of ν in Beltrami's elastic-plastic transition regime, a cube plane compression test. (right) D6AC, Ariane 5 Booster) Stress-strain measurement points with a Ramberg-Osgood engineering stress-strain data mapping curve under axial tension

14.4 Mapping of the measured stress-strain curve by the Ramberg-Osgood Model

In a contract of MAN-NT with the institute IWF at Freiburg all standard model-required properties have been determined. For completion, hopefully in a material-handbook given will be in addition the plastic strain A_5 and also the final necking value Z , being usually minimum and not average values. $A_{fr} \equiv A_{\text{rupture}}$ comes from measurement of A_5 (type: $L_0 = 5 \cdot d_0$, original length L_0 and initial diameter d_0) as plastic or permanent change in length, measured on the load-controlled broken test specimen and Z the radial plastic necking A-reduction ratio value, in % (Unfortunately, material mechanics also uses the letter A for this strain property).

Table 14-2 lists analysis-relevant quantities (in MPa and %) to be applied in a Ramberg-Osgood curve modelling.

Table 14-2: AA2219 material properties and Ramberg-Osgood parameters. Isotropic materials, in MPa and %), $d = 4.0 \text{ mm}$. Regarding \bar{R}_{odc} , see the following Sub-chapter 14-6.

| $\bar{R}_{p0.2}$ | \bar{R}_m | A_{gl} | $\text{true}\bar{R}_m$ | $\text{true}A_{gl}$ | \bar{R}_{odc} | $A_{fr} = A_{rupt}$ | $\text{true}A_{fr}$ | Z | \bar{n} | $\text{true}\bar{n}$ | $R_{p0.2}$ | R_m | R_{odc} |
|---|-------------|----------|------------------------|---------------------|-----------------|---------------------|---------------------|-----|-----------|----------------------|-------------------|-------|-----------|
| 352 | 453 | 4.9 | 478 | 4.8 | 535 | 7.7 | 7.5 | 20 | 12.7 | 10.6 | 297 | 417 | 492 |
| MPa | MPa | % | MPa | % | MPa | % | % | % | - | - | MPa | MPa | MPa |
| average (mean, typical, characteristic) values for best mapping | | | | | | | | | | | Design Allowables | | |

$$\text{true}\varepsilon = \frac{\text{true}\sigma}{E_0} + 0.002 \cdot \left(\frac{\text{true}\sigma}{\text{true}\bar{R}_{0.2}} \right)^{\text{true}\bar{n}} = \text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl},$$

$$E_{\text{sec}}^{\text{hard}} = \frac{\sigma}{\varepsilon} = \frac{\sigma}{\frac{\sigma}{E_0} + 0.002 \cdot \left(\frac{\sigma}{\bar{R}_{0.2}} \right)^{\bar{n}}} = \frac{E_0}{1 + 0.002 \cdot \frac{E_0}{\bar{R}_{0.2}} \cdot \left(\frac{\sigma}{\bar{R}_{0.2}} \right)^{\bar{n}-1}}, \quad E_{\text{tan}}^{\text{hard}} = \frac{d\sigma}{d\varepsilon} = \frac{E_0}{1 + 0.002 \cdot \bar{n} \cdot \frac{E_0}{\bar{R}_{0.2}} \cdot \left(\frac{\sigma}{\bar{R}_{0.2}} \right)^{\bar{n}-1}}.$$

14.5 Poisson's ratio

If analytically necessary the value of Poisson's ratio ν , which increases when stresses narrow the plastic regime, can be determined for stability analyses as a function of the stress. The formula, which uses quantities of the R-O-mapped true stress-true strain curve, is derived in Table 14-3.

Table 14-3: Derivation of a formula for Poisson's ratio

$$\text{true}\varepsilon = \text{true}\varepsilon^{el} + \text{true}\varepsilon^{pl} \text{ with } \text{true}\varepsilon_{ax} = \text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}, \quad \text{true}\varepsilon_{lat} = \text{true}\varepsilon_{lat}^{el} + \text{true}\varepsilon_{lat}^{pl}, \quad \nu_0 = \frac{-\text{true}\varepsilon_{lat}^{el}}{\text{true}\varepsilon_{ax}^{el}}$$

from incompressibility in the plastic range (\equiv volume conservation law) $\frac{V}{V_0} = \frac{\ell}{\ell_0} \cdot \frac{A}{A_0} = 1$

follows $\text{true}\varepsilon_{ax}^{pl} + 2 \cdot \text{true}\varepsilon_{lat}^{pl} = 0$ and $\text{true}\varepsilon_{lat}^{el} = -\nu_0 \cdot \text{true}\varepsilon_{ax}^{el}$, which gives after insertion of above relations

$$\text{true}\nu = \frac{\text{true}\varepsilon_{lat}}{\text{true}\varepsilon_{ax}} = -\frac{\text{true}\varepsilon_{lat}^{el} + \text{true}\varepsilon_{lat}^{pl}}{\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}} = -\frac{\text{true}\varepsilon_{lat}^{el} - 0.5 \cdot \text{true}\varepsilon_{ax}^{pl}}{\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}} = -\frac{\text{true}\varepsilon_{ax}^{el}}{\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}} \cdot \left(-\nu_0 - 0.5 \cdot \frac{\text{true}\varepsilon_{ax}^{pl}}{\text{true}\varepsilon_{ax}^{el}} \right)$$

$$= -\frac{\text{true}\varepsilon_{lat}^{el} - 0.5 \cdot \text{true}\varepsilon_{ax}^{pl}}{\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}} = -\frac{-\nu_0 \cdot \text{true}\varepsilon_{ax}^{el} - 0.5 \cdot \text{true}\varepsilon_{ax}^{pl}}{\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}} = -\frac{-\nu_0 \cdot \text{true}\varepsilon_{ax}^{el} - 0.5 \cdot (-\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl})}{\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}}$$

$$= -\frac{-\nu_0 \cdot \text{true}\varepsilon_{ax}^{el} - 0.5 \cdot (-\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl})}{\text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl}} = 0.5 + \frac{-\nu_0 \cdot \text{true}\varepsilon_{ax}^{el} - 0.5 \cdot (-\text{true}\varepsilon_{ax}^{el})}{-\text{true}\varepsilon_{ax}^{pl}}$$

$$= 0.5 - \frac{\text{true}\varepsilon_{ax}^{el}}{\text{true}\varepsilon_{ax}^{pl}} \cdot (0.5 - \nu_0), \text{ see. 14.5.}$$

However, this formula does not fully lead to $\nu = 0.5$ at R_{02} as can be seen in Fig.14-5. A better approximation $\nu = 0.5 - (E_{\text{tan}}^{\text{hard}} / E_0) \cdot (0.5 - \nu_0) \equiv \text{true}\nu$ is usually applied in the elastic-plastic domain in stability analysis employing the tangent modulus function above in order to approximately consider the changing ν in analysis.

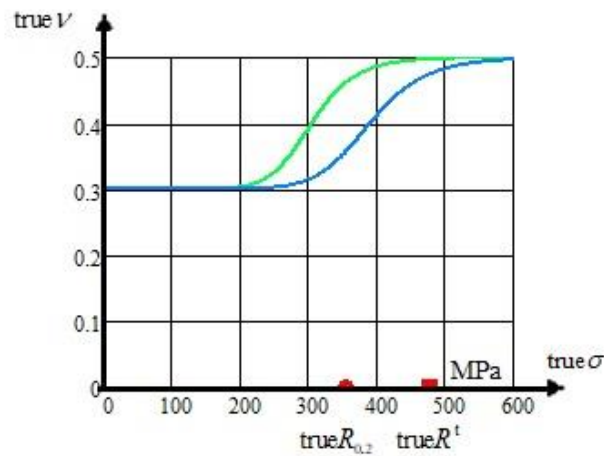


Fig.14-5: Course of Poisson's ratio in the elastic-plastic domain, determined with several formulas

LL:

- * The determination of the properties of a solid material requires a force-elongation curve which is then accurately to transfer into a stress-strain curve that is independent from the tested specimen type rod, sheet, coupon, cube.
- * Before any performance of a non-linear analysis is executed it is to check whether true or engineering curve quantities are to provide for numerical input. This then fixes the output
- * Beyond R^t necking occurs generating a hydrostatic stress σ_{hyd} in the tensile rod, which lowers the stress-strain curve (see Chapter 15) in the high plastic regime
- * Poisson's ratio can only approach the limiting points $0.5 > \nu > (-1, \text{principally.})$ So-called auxetic materials possess a negative ν . Being strained, the transverse strain in the material will also be positive
- * UD-materials have different ν -values in the directions of anisotropy
- * True strains can be added while engineering strains can not!

In Fig.14-6 the different growth of the engineering and the true stress-strain curve is displayed up to the tensile strength point at the 'End-of-uniform elongation'. Beyond R^t , in test data evaluation the axial stress has to be replaced by the equivalent stress because necking in the test specimen activates a hydrostatic residual stress state, dependent on the test specimen used.

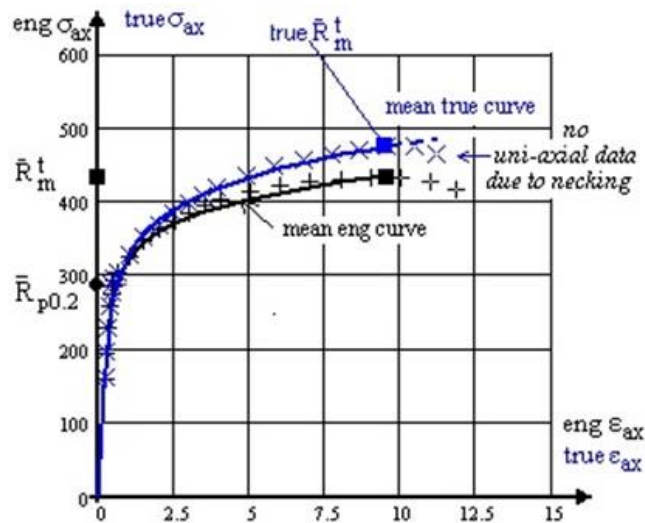


Fig.14-6, AA 2219:

Differences in R-O-mapping of engineering and true stress-strain curve, single measurement.. Bar over R indicates a mean value. F/A_0 at 'End of uniform elongation' = 'Onset-of-(diffuse) Necking'

In *Fig.14-7* the full stress-strain curve is presented and associated significant points including strength design allowables points are depicted. Additionally for ‘Onset-of-yielding’ the Margin of Safety is rendered in order to visualize the size of the fulfillment of the ‘Design Yield’ Limit State.

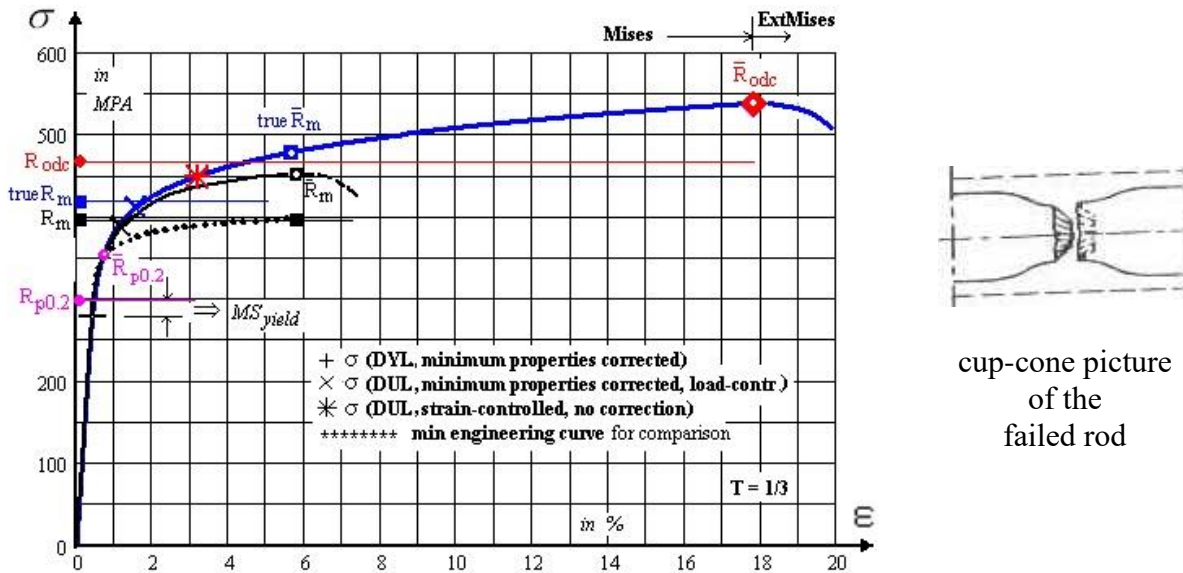


Fig. 14-7: Equivalent true stress-equivalent true strain curve. Proposed local strain-controlled extended stress-strain curve incl. mean fracture points and strength design allowables (no bar over)

The full curve ends with reaching the ‘onset-of-ductile cracking’ point at the associated strength R_{odc} .

LL:

- * Opposite to some regulations it is to note “In general, it can be not correct to use a minimum engineering curve in order to obtain the desired realistic structural behavior because structures are usually statically indeterminate”.
- * The elliptical shape of the ‘Beltrami egg’ and its surface potential description will be used in the ‘Gurson domain’ too, next chapter.

14.6 Estimation of the Strength \bar{R}_{odc}

Beyond ‘Onset-of-diffuse necking’ the axial strain measurement becomes senseless, only representative is the rod radius-decrease measurement to investigate in this full plastic domain the influence of the hydrostatic stress. From the measured plastic cross-section reduction the plastic portion ϵ_{odc}^{pl} can be estimated and the ‘plastic’ curve point \bar{R}_{odc} computed if the only counting associated plastic strain is known, fixed by the diameter reduction. Because the R/O-model excellently maps the true strength course of test data, its plastic part is employed to estimate a value for the plastic point $\bar{R}_{odc} \equiv$ ‘Onset-of-ductile-cracking’, which is of interest for plastic structural design.

This can be executed by using volume constancy applying the measured reduction of the initial radius $a = d/2$ of the tensile rod. With $Z(\bar{R}_{rupt})$ taken as $Z(\bar{R}_{odc})$ the estimation of \bar{R}_{odc} at $true \epsilon_{odc}$ from the Ramberg-Osgood curve is performed as shown in *Table 14-4*.

Ductile collapse or rupture \bar{R}_{rupt} , respectively, is just of theoretical interest.

Table 14-4: Derivation of an estimate value for the Strength \bar{R}_{odc}

At \bar{R}^t 'Onset-of- (diffuse) necking' $d_{oon} = 3.89$ mm, at 'Onset-of-ductile cracking' $d_{odc} = 3.78$ mm.

$$\varepsilon^{pl} = 0.002 \left(\frac{\sigma}{\bar{R}_{0.2}} \right)^n, \quad \varepsilon_{rad}^{pl} = \ln\left(\frac{r}{a}\right), \quad \frac{\Delta A}{A_0} = \frac{A_0 - A}{A_0} = 1 - \frac{A}{A_0} \rightarrow \frac{A_{rupt}}{A_0} = 1 - Z = \frac{r^2}{a^2}$$

$$\varepsilon_{ax,00n}^{pl} = -2 \cdot \varepsilon_{rad}^{pl} \text{ at } \bar{R}^t \quad \text{and delivers} \quad \text{true}\bar{n} = \frac{\ln\left(\varepsilon_{ax,00n}^{pl} / 0,2\%\right)^n}{\ln\left(\bar{R}^t / \bar{R}_{p0.2}\right)} .$$

With known $\varepsilon_{rad}^{pl} = \ln\left(\frac{r}{a}\right) = \ln(\sqrt{1-Z}) = \ln(\sqrt{1-0.20}) = -11.2\%$ and $\varepsilon_{ax}^{pl} = -2 \cdot \varepsilon_{rad}^{pl}$

follow for the non-corrected odc-point $\rightarrow \text{true}\varepsilon_{ax}^{pl} = 0.002 \cdot \left(\frac{\bar{R}_{odc}}{\bar{R}_{0.2}}\right)^{\text{true}\bar{n}}$

$$\Rightarrow \bar{R}_{odc} \cong \bar{R}_{0.2} \cdot \sqrt[\text{true}\bar{n}]{\varepsilon_{ax}^{pl} / 0.002} = 542 \text{ MPa} \quad \text{and}$$

$$\text{true}\varepsilon_{odc} = \text{true}\varepsilon_{ax}^{el} + \text{true}\varepsilon_{ax}^{pl} = \frac{\bar{R}_{odc}}{E} + 0.002 \left(\frac{\bar{R}_{odc}}{\bar{R}_{0.2}}\right)^{\text{true}\bar{n}} .$$

14.7 Beltrami's Potential Surfaces in the Elastic-plastic and as Idea for the Porous Regime

From previous investigations the author knows, that any volume change, due to the FMC 'rules', is to describe by the term I_1^2 . If a shape change occurs then the invariant J_2 is required.

Elastic-plastic transition regime:

Beltrami cites: "*The deformation of a material consists of two parts, a shape and a volume change*". Based on this, one can formulate for the elastic-plastic transition regime

$$\frac{(2+2\nu) \cdot 3J_2}{\bar{R}^2} \quad \text{and} \quad \frac{(1-2\nu) \cdot I_1^2}{\bar{R}^2} \rightarrow \frac{3J_2}{\bar{R}^2} + \kappa \cdot \frac{I_1^2}{\bar{R}^2} = c^{\text{Bel}} \quad \text{with} \quad \kappa = \frac{1-2\nu}{2+2\nu} .$$

Into this formulation a normalizing strength is inserted: $I_1 = \bar{R}$, $J_2 = 2\bar{R} / 6 \rightarrow c^{\text{Bel}} = 1 + \kappa$ and

for the special yield potential surface ($\nu=0.5$) yields $\frac{3J_2}{\bar{R}_{02}^2} + 0 \cdot \frac{I_1^2}{\bar{R}_{02}^2} = 1 + 0$ ('Mises' cylinder).

Beltrami bridges the elastic domain with the plastic domain ($3 \cdot J_2$ is Mises part). His formulation is not a failure function but a descriptive function to predict subsequent Beltrami surfaces $\nu(\bar{R})$, which are surfaces of equal potential. This means: A pair (ν, \bar{R}) must be given for each desired ν -curve of the subsequent potential surfaces are obtained, *see Fig.14-8 left*. This part figure shows the change of the potential surface of the growing 'Yield' body with increasing ν in the elastic-plastic transition domain. The two center figures show the cross-section using the principal stresses and below the development of the yield body from the yellow egg ($\nu = \nu_0$) up to full yielding ($\nu = 0.5$) rendered by the 'Mises cylinder' \rightarrow *Poisson's ratio ν drives the elliptic shaping!*

Plastic porosity affected regime: an anticipation, considering Chapter 15

Porosity causes a volume increase. This works oppositely as in the elastic-plastic transition regime, which can be described by Beltrami, too. Increasing porosity f means a decreasing Poisson's ratio ν

and a more elliptic shape. In the outer figures of *Fig.14-7* both the regimes of the changing Poisson's ratio are displayed. The right part figure, modelled by Beltrami, pre-informs (see §15) how the surface of the yield body changes its shape with decreasing ν according to the increasing porosity f .

Fig.14-7(right) displays the development of the subsequent failure surfaces whereby an increasing true stress is considered. This is relevant for the critical material location. After achieving the tensile strength a small further radial increase of the surface is obvious together with the initiation of an increasing elliptic failure surface. With increasing degradation the subsequent surfaces become more and more elliptical. This is the opposite process regarding Beltrami in the elastic-plastic transition regime. A growing f means higher true stress but less cross-section or load-carrying material in the strain-controlled 'hot spot'.

The Beltrami formulation delivers an *Idea for the ductile porous regime* and is intended to replace the 'Gurson' formulation by Cuntze's so-called 'Extended Mises' one, reading

$$\frac{3J_2}{\bar{R}_{0.2}^t} + \frac{1-2\nu}{2+2\nu} \cdot \frac{I_1^2}{\bar{R}_{0.2}^t} = c^{\text{Bel}} \Rightarrow \frac{3J_2}{\bar{R}_{0.2}^t} + c_{12} \cdot \frac{I_1^2}{\bar{R}_{0.2}^t} = c^{\text{ExtMises}} .$$

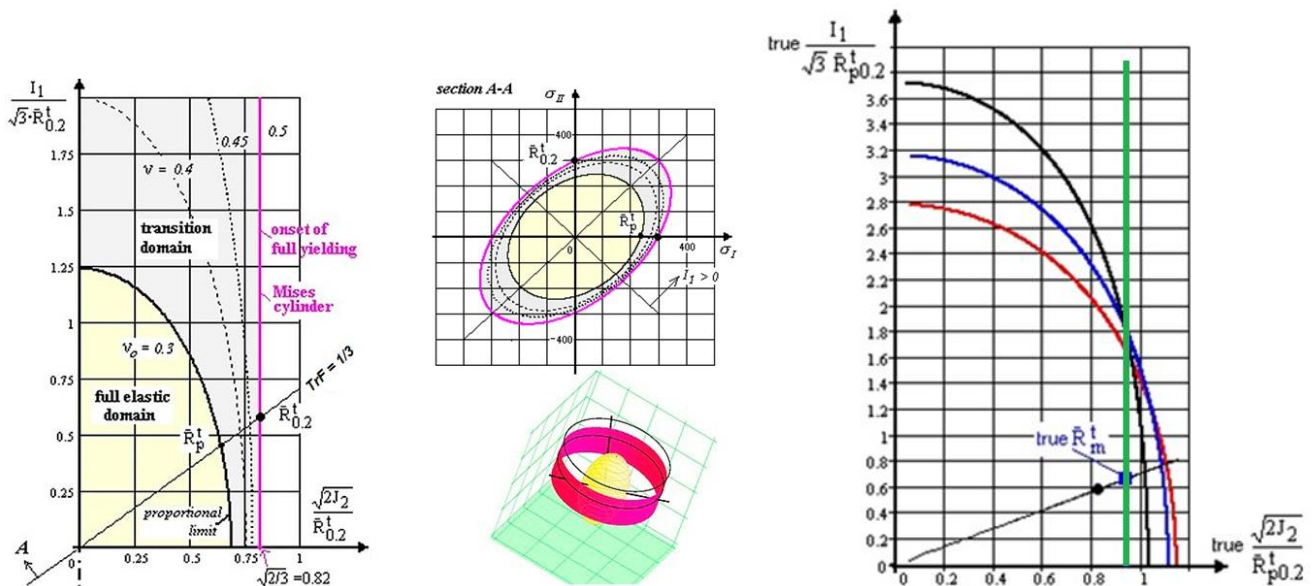


Fig.14-8: (left) Elastic-plastic transition domain, development of the Beltrami surfaces from egg shape (growing yield potential surface with $\nu_0 = 0.3$ for metals (0 for foam = sphere) $< \nu < 0.5$ ('Mises cylinder' $\rightarrow J_2 = \text{constant} = \text{incompressibility}$) depicted in Lode-Westergaard coordinates. (center) visualization of the Beltrami potential surfaces. (right) Change of potential surfaces in the porous domain computed with the Extended Mises formulation (see [CUN22, §17]), $f = 0, 0.1, 0.2, 0.3$

Also here, the yield strength can be used for normalization. The parameters c^{Bel} , c^{ExtMises} mark the size parameter of the changing potential surface (see survey in *Table 15-4*).

In order to understand the chosen Haigh-Lode-Westergaard coordinates *Fig.14-9* is provided below. The vector $\{\sigma_{\text{prin}}\} = (\sigma_I, \sigma_{II}, \sigma_{III})^T$ is a vector-addition of the principal stresses. The cone angle between all principal axes and I_1 is 54.75° .

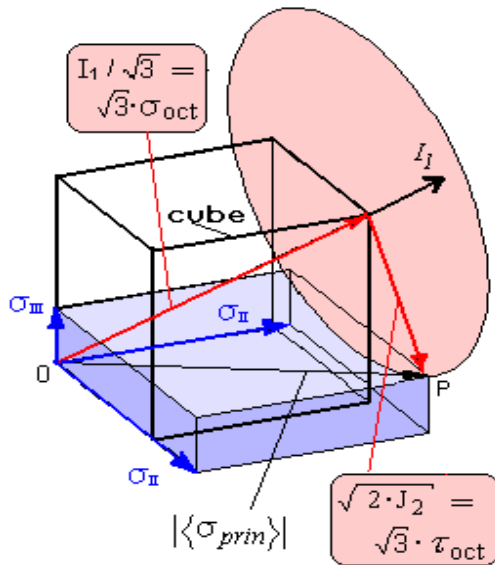


Fig.14-9:
Visualization of the used Lode-Westergaard coordinates by the principal stresses acting at a material cube.

Octahedral stresses:

$$\sigma_{oct} = I_1 / 3 \quad \text{with } I_1 = \sigma_I + \sigma_{II} + \sigma_{III}$$

$$\sigma_{eq}^{Mises} = \sqrt{3 \cdot J_2} = f(\tau), \quad \text{with}$$

$$6 \cdot J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2$$

$$\tau_{oct} = \sqrt{6 J_2} / 3 -$$

To make more familiar with potential surfaces Fig.14-10 presents two potential surfaces dedicated to different Effs, for fracture $Eff = 100\%$ and for a loading that generates $Eff = 50\%$.

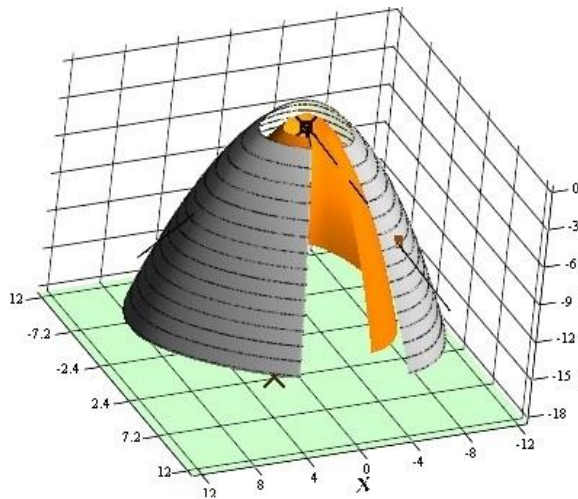


Fig.14-10: Two potential surfaces. Eff is the measure for the distinct potential surface with $Eff=1=100\%$ the fracture surface. The potential surfaces are $Eff^{SF}=50\%$ and $Eff^{cSF}=100\%$, fracture.

Indicated are the failure stress points $\bar{R}' = 4 \text{ MPa}$, $\bar{R}'' = 3 \text{ MPa}$, $\bar{R}^c = 40 \text{ MPa}$, $\bar{R}^{cc} = 49 \text{ MPa}$ and the principle stress axes.

'Normal Concrete', 3D test data available

$$Eff^{SF} = c^{SF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{SF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^c}$$

LL:

- * The shape of the potential surfaces in the plastic porosity regime changes oppositely to the shape in the elastic-plastic regime. Both the surface shapes one can dedicate to the change of the Poisson ratio ν
- * In structural analysis the stresses are most-often determined in the elastic-plastic regime. This is performed very accurately, sometimes over-precise. However in this domain the Poisson's ratio changes significantly, which should be considered.

15 A measurable parameters'-based 'Extended-Mises' Model instead of a 'Gurson' Model?

Aim: De-complication of highly non-linear plastic analyses by generation of a simplified model to perform Design Verification in a Ductile Metal's high Porous Regime

15.1 Introduction

There is stress- and strain-controlled behavior. Strain-controlled locations in a structure will not break, when the stress level reaches tensile strength R^t . A fuel-outlet hole in the upper tank of the Ariane 5 central stage was such a strain-controlled case at MAN, where the vicinity of the 'overstrained' critical material location takes over the reduced loading capability, no direct fracture is to face.

Such a (seldom) task caused MAN-Technologie to let perform an analysis together with IWM Freiburg applying a multi-parameter 'Gurson' yield model. Its model parameters cannot be measured directly, but are usually determined by a FE analysis which best models the deformation of the test specimen, a classical simulation process. An example for such a multi-parameter set, determined for the aluminum alloy AA2219 and by using the tensile rod test specimen, is given in the table below [IWM Freiburg]:

| f_0 | f_n | f_c | f_F | q_1 | q_2 | ϵ_n | s_n |
|-------|-------|-------|-------|-------|-------|--------------|-------|
| 0.00 | 0.05 | 0.04 | 0.15 | 1.5 | 1.0 | 0.20 | 0.01 |

The applied 'Gurson'-model (such a model is a model of the *Continuum (micro-)Damage Mechanics theory in the ductile materials regime*) of the IWM was a refined one. Refinement means that more parameters are to determine than for a simpler 'Gurson' model. Therefore, the optimal model parameter set of a 'Gurson' model depends on the mesh fineness and has to be inversely determined by an excellent simulation of the test specimen's behavior, see [Fig.15-1 left](#) for the tensioned rod

(Gurson A L: *Continuum Theory of Ductile Rupture by Void Nucleation and Growth. Part 1: Yield criteria and flow rules for porous ductile media*. J. Eng. Mater. Techn.99 (1977), 2-15)

Using 'Gurson' model results, the responsible design engineer must ask:

What about the scatter of the simulation-won parameters which are to insert in the analysis?

Without knowledge of the scatter there is not a generally accepted design verification possible. Might it be not better to apply a simpler model with 2 or 3 parameters at dispense of the little gain of the last load carrying portion after coalescence at 'onset-of-ductile cracking' marked by the corresponding strength value R_{odc} ? This is the 'technically relevant point', where the coalescence of voids begins. Only a reduced procedure with directly measurable model parameters has the chance to capture the statistical Design Verification requirements.

In the context above the question comes up:

"How much Gurson material modelling is necessary to achieve a reliable prediction of the local design-deciding ductile fracture level of the structure?"

This failure mode 'ductile fracture' is defined here to be met at 'onset-of-ductile cracking' and it shall correspond to Design Ultimate Load. Such an application is a seldom case, where the deformation-controlled strength value $R_{odc}^t > R^t$ is used to *save the final design* not anymore possible via the load-controlled strength value R^t . A simpler model is required. Two challenging parts tasks are thereby faced:

(1) Creation of a model simpler than a multiple-parameter ‘Gurson’ model, and
 (2) to capture the porosity f in the equivalent σ - ε -curve, to be provided, whereby f is an additional but measurable model parameter transferring the ‘Mises’ model to the ‘Extended Mises’ model. For its derivation, the various micromechanical mechanisms during ductile fracture are of basic interest:

- * Void nucleation in the test rod at so-called second phase particles by debonding
- * Void growth, controlled by stress Triaxiality Factor TrF and growing plastic strain ε_{eq}^{pl} , and
- * Coalescence of voids by internal shear stress-driven rod necking with final ductile rupture.

For the evaluation of the usual rod test results, the widely used correction formula of P.W. Bridgman is employed. *Fig.15-1(left)* presents the dependency of the rod’s diameter reduction on the load F and further shows simulation curve and test curve. The measurement of the diameter reduction is mandatory beyond the ‘end of uniform elongation’ at the tensile strength point $\bar{R}^t = \max F / A$, depicting the ‘onset-of-diffuse necking σ_{on} point and experiencing full plasticity. Beyond \bar{R}^t only true values represent the reality. Mind: $F(\Delta d)$ is not completely of the same shape like $true\sigma(true\varepsilon)$.

In the load-controlled regime axial strain measurements are performed whereas in the transversal, plastic strain-controlled necking regime diameter reduction measurements are to execute. In the *Fig.15-1(right)* attention is drawn to the various stress-strain curves used and to the associated strengths. Displayed are the mean technical and mean true strengths together with the associated Design Allowables.

If materials do **not fail** when the tensile strength is reached, then this is accompanied by the fact (*Fig.15-1, left*) that $\max F$ does not essentially change over a certain range of the strain because hardening still **works until a slight kink will** occur due to void coalescence and destruction of piled-up dislocations. Degradation wins over hardening at the ‘onset of ductile cracking’ strength point R_{odc} . R_{odc} and marks the coalescence-linked kink and is defined here as the critical strength.

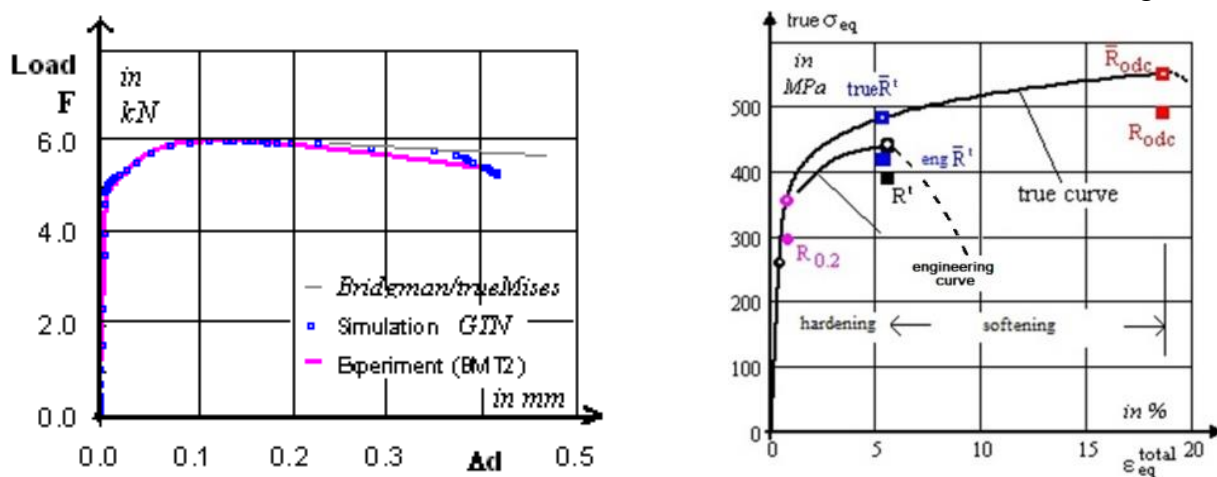


Fig.15-1: (left) Dependency of diameter reduction Δd on the applied load F . Comparison of global simulation and test results (IWM Freiburg, Dr. Sun). (right) Ramberg-Osgood-mapped true and engineering stress-strain curves of AA2219A bar over R indicates a mean value, no bar over R indicates a ‘design allowable’

15.2 Bridgman-3D Correction of the true σ - ε -Curve, employing ‘Mises’

Equivalent stress: $true\sigma_{ax} \rightarrow true\sigma_{eq}$

The validity of the uniaxial stress-strain curve measured in the smooth tensile rod test is terminated at the load-controlled strength point $\max \sigma_{\text{true}} = \bar{R}^t = \max F / A$, which corresponds to the maximum load F and to the actual minimum cross section of the neck. However, beyond \bar{R}^t ('end-of-uniform-elongation') at the 'onset-of-diffuse necking odn ' point the 1D-stress situation in the tensile rod becomes a 3D one and an equivalent stress $\sigma_{eq}^{\text{Mises}}$ has to be considered in order to capture spatial stress tasks.

Under tensioning, in the plastic regime the lateral contraction of the material at the center of the neck is impeded by neighboring material leading to a 3D-stress state. Hence, a simple extrapolation of the F/A (σ - ε)-curve beyond \bar{R}^t cannot provide a physically accurate curve, because the necking-generated 3D-residual stress state σ_{hyd} is to consider in the evaluation of the tensile rod test results in order to obtain a real σ_{eq} . The three stresses within σ_{eq} reach their maximum values at the center of the rod's cross-section with approximately equal values $\sigma_{\text{radial}} = \sigma_{\text{hoop}}$, except close to the surface, as depicted in *Fig.15-2(left)* below. The values of σ_{radial} , σ_{hoop} and of the created necking radius ρ raise with σ_{ax} . The former F/A -quantified capacity becomes continuously reduced with increasing necking. Hence, the true stress-strain F/A curve is to correct to obtain a realistic equivalent stress. In the center of the rod an increasing stress Triaxiality Factor TrF is faced. Assuming a constant σ over

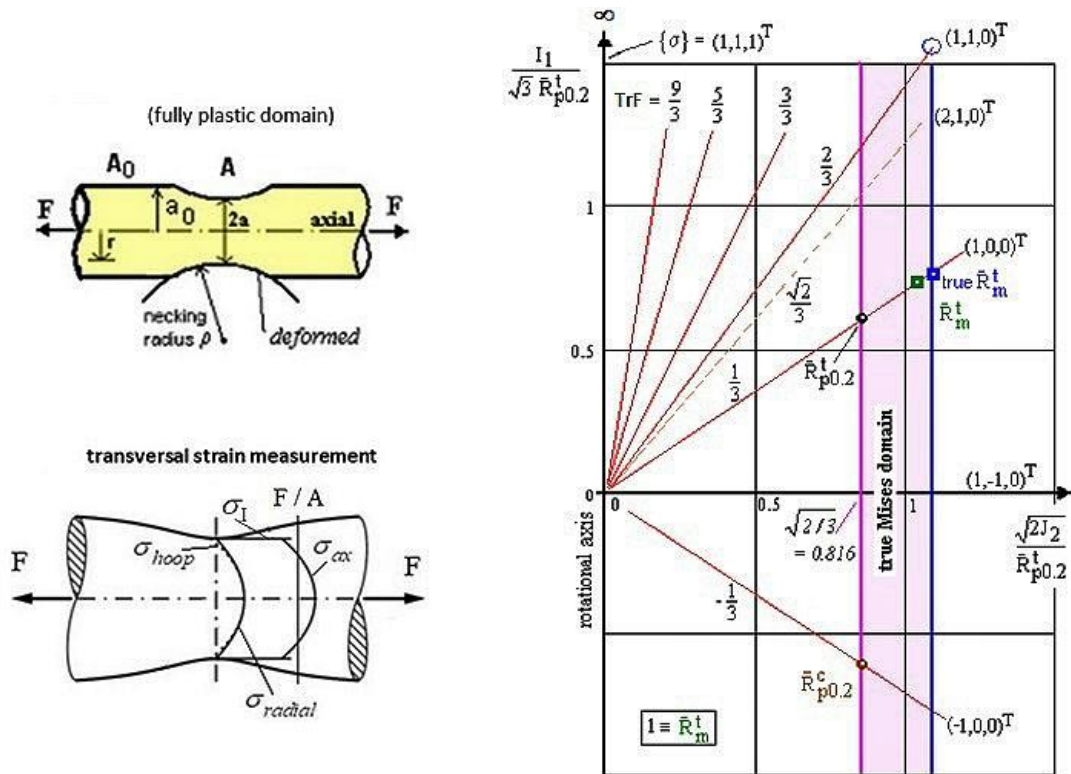


Fig.15-2: (left) Stresses and transversal (radial) strain measurement of the necked round tensile rod. F :=force, A := minimum actual cross section of the neck. F :=Force F_{ax} , A_0 := original cross-section.

$\bar{R}^t = \max F / A_0$, $\varepsilon \leq \bar{\varepsilon}_{gl}$ (permanent strain linked to load-controlled fracture at \bar{R}^t). Necking radius is ρ .

(right) Schematic visualization of the Triaxiality Factor TrF , responsible for failure in the rod center

$$\{\sigma\} = \{\sigma_I, \sigma_{II}, \sigma_{III}\}^T, TrF \{\sigma_I, \sigma_{II} = \sigma_I, 0\}^T = 2/3.$$

the rod's cross-section, *Fig.15-2(right)* illustrates by a variety of TrF -beams that values higher than $2/3$ (bi-axial stressing) are practically not possible. Assuming constancy is not anymore the case for a plastic rod neck, where the failure decisive location is the center of the cross-section with also there facing $maxTrF$. Notched test specimens are applied to capture higher multi-axial stress states, $TrF = 1$, values $> 1/3$.

Fig. 15-3 shows the void volume fraction in the necking region at failure. The highest values are reached in the center of the specimen (Element 20) as expected, TrF highest. From the central region micro-damage spreads out over the whole cross section.

Basic task now will be the necessary transfer from the uniaxial $true\sigma_{ax}(true\epsilon_{ax}) \rightarrow$ tri-axial $true\sigma_{eq}(true\epsilon_{eq})$ in the diffuse necking regime.

Bridgman provided a correction means how to adjust $true\sigma_{ax}$, but had to make some essential assumptions:

- (1) The cross section of the necked region remains angular (like the 'Mises' cylinder, assuming a rotationally symmetric yield body).
- (2) The inner axial contour of the neck can be approximated by the arc of a circle with the radius ρ .
- (3) 'Mises' can be applied (effect of growing voids is therefore not considered).

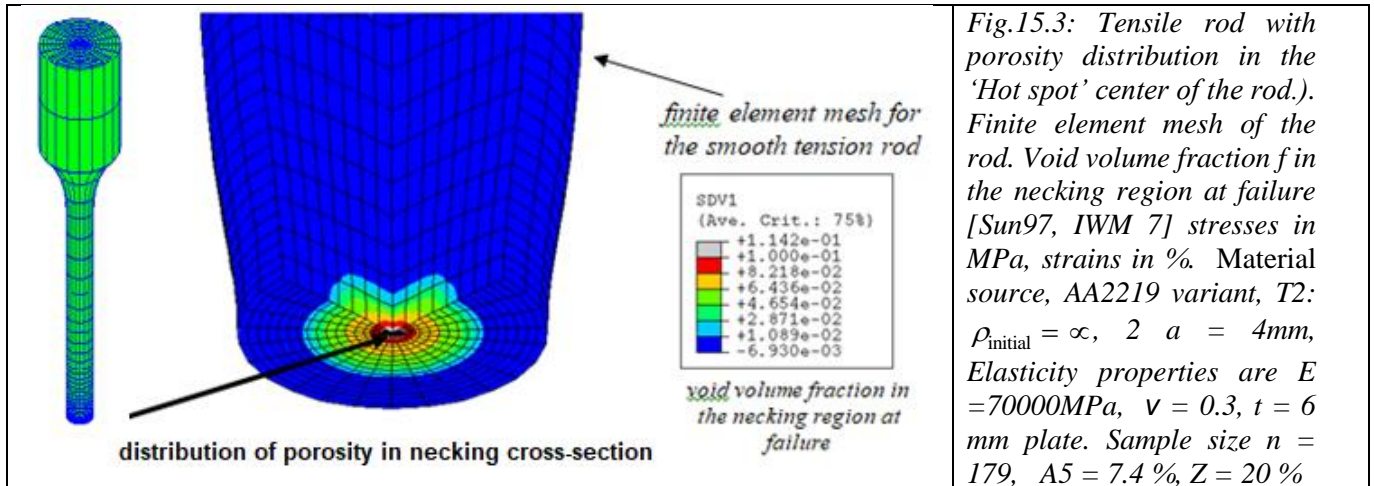


Fig.15.3: Tensile rod with porosity distribution in the 'Hot spot' center of the rod.). Finite element mesh of the rod. Void volume fraction f in the necking region at failure [Sun97, IWM 7] stresses in MPa, strains in %. Material source, AA2219 variant, T2: $\rho_{initial} = \infty$, $2 a = 4mm$, Elasticity properties are $E = 70000MPa$, $\nu = 0.3$, $t = 6 mm$ plate. Sample size $n = 179$, $A5 = 7.4 \%$, $Z = 20 \%$

Due to the diffuse necking, an axial load increase-caused internal hydrostatic *tensile* stress state σ_{hyd} is generated, representing a deformation-dependent residual stress state. Its radial distribution can be Mises-based estimated - under the axial loading $\{\sigma\} = (\sigma_I, \sigma_{II}, \sigma_{III})^T = (F/A, 0, 0)^T$ -

after Bridgman by
$$\sigma_{hyd}(r) \approx \sigma_I \cdot \ln\left(1 + \frac{a^2 - r^2}{2 \cdot a\rho}\right) \quad \text{with} \quad \sigma_I < \frac{F}{A} = \frac{F}{\pi \cdot a^2}, \quad \text{Fig.15-2}$$

with $F :=$ load, $a :=$ radius of actual cross section of neck, $\rho :=$ radius of neck curvature and F/A an integral quantity capturing the external loading F . The full set of relevant relations then reads:

$$\sigma_{ax}^t(r) = \sigma_I + \sigma_{hyd}(r) \quad \text{and} \quad \sigma_{radial}(r) \approx \sigma_{hoop}(r) = \sigma_{hyd}(r) \quad \text{and as equivalent stress follows}$$

$$\text{for a single stress} \quad \rightarrow \sigma_{eq}^{Mises} = \sqrt{3 \cdot J_2} = \sqrt{\frac{1}{2} \cdot \sqrt{[(\sigma_I)^2 + (0)^2 + (-\sigma_I)^2]}} = \sigma_I \quad \text{and also}$$

$$\text{for a superimposed } \sigma_{hyd} \rightarrow \sigma_{eq}^{Mises} = \sqrt{3 \cdot J_2} = \sqrt{\frac{1}{2} \cdot \sqrt{[(\sigma_I - \sigma_{hyd})^2 + (\sigma_{hyd} - \sigma_{hyd})^2 + (\sigma_{hyd} - \sigma_I)^2]}} = \sigma_I$$

$$I_1 = \max \sigma_{ax}^t + \max \sigma_{radial} + \max \sigma_{hoop} = \sqrt{3 \cdot J_2} + 3 \cdot \sigma_{hyd}, \quad \sqrt{3 \cdot J_2} = \sigma_I \quad (\leftarrow \text{no } \sigma_{hyd} \text{ effect}).$$

$$TrF(r) = \text{true} \sigma_{mean} / \text{true} \sigma_{eq}^{Mises} = (I_1 / 3) / \sqrt{3 J_2} = [\sqrt{2} / 3] \cdot (I_1 / \sqrt{3}) / \sqrt{2 J_2} = \frac{1}{3} + \frac{\sigma_{hyd}}{\sigma_I}.$$

Decisive for failure in the rod is the still mentioned Triaxiality Factor TrF , which increases with the true axial loading. Its maximum is in the center, the ‘hot spot’ at $r = 0$. In this micro-damage critical cup-cone center the 3D-state of stresses reads

$$\max \sigma_{hyd}(r=0) = \sigma_I \cdot \ln\left(1 + \frac{a}{2 \cdot \rho}\right), \quad \max \sigma_{ax}^t(r=0) = \sigma_I + \max \sigma_{hyd}$$

$$\text{with the stress state in the rod's center } \{\sigma\} = (\sigma_I + \max \sigma_{hyd}, \max \sigma_{hyd}, \max \sigma_{hyd})^T.$$

In the necessary adjusting process of the F/A-curve in the diffuse necking regime (Phase 3) the first step is to integrate the axial stress, which varies over the radius. From load balance the following relations are yielded in Table 15-1.

The last unknown is the neck radius ρ . It could be computed during testing by measuring the shape change of the neck via a real-time Digital Image Correlation (DIC) 3D full-field measurement optical technique of the surface strains and an associated surface geometry model.

Table 15-1: Bridgman-Derivation of the cross-section quantities of the tensioned rod

| | |
|---|--|
| $\frac{F}{A} = 2 \cdot \int_0^a \sigma_{ax}^t \cdot \pi \cdot r \cdot dr / (\pi \cdot a^2)$ $= 2 \cdot \int_0^a (\sigma_I + \sigma_I(r)) \cdot \pi \cdot r \cdot dr / (\pi \cdot a^2)$ $= 2 \cdot \int_0^a \left(\sigma_I + \sigma_I \cdot \ln\left(1 + \frac{a^2 - r^2}{2 \cdot a \cdot \rho}\right) \right) \cdot \frac{\pi \cdot r \cdot dr}{\pi \cdot a^2}$ | <p>The diagram shows a cylindrical rod under tension. Two forces F are applied at the ends, pulling the rod apart. A cross-section is shown with a dashed line through the center. The axial stress is labeled as σ_{ax}. The hoop stress is labeled as σ_{hoop} and the radial stress as σ_{radial}. The force per unit area is labeled as F/A.</p> |
| <p>integrated follows $\frac{F}{A} = \sigma_I \cdot \left(1 + \frac{2\rho}{a}\right) \cdot \ln\left(1 + \frac{a}{2\rho}\right)$ with $\sigma_{eq}^{Mises} = \sqrt{3 \cdot J_2} = \sigma_I$</p> $\sigma_I = \sqrt{3 \cdot J_2} = \sigma_{eq}^{MisBri} = \frac{F}{A} / \left(1 + 2 \cdot \rho/a\right) \cdot \ln\left(1 + a/2 \cdot \rho\right), \quad \text{valid } > \bar{R}^t \text{ or } A < A_{oon},$ <p>an equation, in which the ratio a/ρ is not known.</p> <p>If no test result is available, then Lorrek-Hill's approach for rupture is applied at \bar{R}_{rod}:</p> $\text{maximum } \frac{a}{\rho} = \sqrt{\ln\left(\frac{A_0}{A_{rupt}}\right) - \ln\left(\frac{A_0}{A_{oon}}\right)}, \quad \frac{A_{rupt}}{A_0} = 1 - Z \quad \text{with a}$ <p>given cross section reduction Z in % at maximum necking at \bar{R}_{rupt}</p> <p>and A_{oon} the cross section at $\bar{R}^t \equiv \bar{R}_m^t$, being 'Onset-of-(diffuse) necking'.</p> $\frac{a}{\rho} = c_1 + c_2 + \text{true} \mathcal{E}_{ax} R_{odc}^2 + c_3 \cdot \text{true} \mathcal{E}_{ax} R_{odc}^3.$ | |

Equivalent strain: $\text{true}\varepsilon_{ax} \rightarrow \text{true}\varepsilon_{eq}$

For the Mises equivalent strain is valid in the plastic domain (elastic part is negligible):

$$\varepsilon_{eq}^{Mises} = \frac{\sqrt{2}}{3} \cdot \sqrt{(\varepsilon_I - \varepsilon_{II})^2 + (\varepsilon_{II} - \varepsilon_{III})^2 + (\varepsilon_{III} - \varepsilon_I)^2} = \frac{\sqrt{2}}{3} \cdot \sqrt{(\varepsilon_I^{pl} - \varepsilon_{II}^{pl})^2 + (\varepsilon_{II}^{pl} - \varepsilon_{III}^{pl})^2 + (\varepsilon_{III}^{pl} - \varepsilon_I^{pl})^2}$$

considers plastic volume constancy (incompressibility) $\Sigma \varepsilon_i^{pl} = 0$ during plastic deformation it becomes

$$\varepsilon_I^{pl}/2 = -\varepsilon_{rad}^{pl} = -\varepsilon_{tan}^{pl} \text{ and } \varepsilon_{rad}^{pl} = \ln(r/a) = \varepsilon_{tan}^{pl} \Rightarrow \varepsilon_I^{pl} = -2 \cdot \ln(r/a) \text{ and it reads}$$

$$\text{true}\varepsilon_{eq}^{Mises} = \frac{\sqrt{2}}{3} \cdot \sqrt{((\varepsilon_I^{pl} + \varepsilon_{rad}^{pl}) - \varepsilon_{rad}^{pl})^2 + 0 + (\varepsilon_{rad}^{pl} - (\varepsilon_I^{pl} + \varepsilon_{rad}^{pl}))^2} = \frac{\sqrt{2}}{3} \cdot \sqrt{2\varepsilon_I^{pl/2}} = \frac{2}{3} \cdot \varepsilon_I^{pl}$$

LL: * *Bridgman correction = approach, which considers the varying stress over the rod's cross-section regarding that the center is the critical line*

* *Lorrek-Hill = approach, which formulates a final value for the change of the curvature radius under loading. The increasing curvature triggers the increasing hydrostatic stress and this is to map*

* *Measured ratio $F/A = \text{stress capacity smeared over the cross-section} = \text{load ability-quantity}$, which represents an effective (smeared) value, which decays with increasing axial strain*

* $\sigma_1 \equiv \sqrt{3J_2} = \text{constant basic stress quantity of the Bridgman approach, see Table 15-1}$

* *The applicability of axial measurement ends with 'End-of-uniform elongation' at \bar{R}^t*

* *Bridgman model application is limited to about 30% cross-section reduction, due to not considering the coalescence of the voids*

* *The Bridgman-correction is applied by using the 'Mises' yield function and not a 'Gurson'-type void growth-capturing (porosity f) yield function. This led the author 20 years ago to propose his so-called 'Extended Mises' yield condition at the end of a joint Research program MAN with IWF-Freiburg.*

Idea:

The replacement of a 'Mises'-based Bridgman correction by a porosity-considering one should lead to a more realistic stress-strain curve and should offer the advantage to escape in the analysis from the high number of non-measurable 'Gurson' model parameters except from f . In order to consider the void growth, the author proposes to replace the Bridgman-corrected Mises-model by the mentioned '*Gurson' model-linked Extended Mises-model*'.

15.3 Porosity-improved Bridgman 3D-Correction of true σ - ε -Curve employing 'Extended Mises'

Porosity means volume change due to void coalescence. Such a volume change can be transferred to a decaying Poisson's ratio as it is known from Beltrami. The author experienced, that the usual 'Gurson'-analyses base on a 'Mises'-linked equivalent stress-equivalent strain curve. This should be improved when considering the porosity f . The author's hypothesis from 2002 reads:

- * Formulation of an egg-shaped yield model, termed Extended Mises, with
- * Simplification to 1 measurable 'Gurson' parameter f , only
- * Improvement of this simpler model idea by applying a porosity-capturing equivalent $\sigma - \varepsilon$ curve
- * Taking a simple 'Gurson' yield model to obtain via a 'comparison of coefficients' a relation to the porosity f in the simple 'Gurson' -model from Gurson-Tvergaard-Needleman, index ^{GTN}
- * Probable 120°-material symmetry in the high porosity regime is not documented and therefore not

considered. It can be captured by replacing $\frac{3J_2}{R^2}$ through $\frac{3J_2}{R^2} \cdot \Theta$ (see *Chapter 11*).

LL:

* *The 'Mises' cylinder is a simplification (remember: §11, 120°-symmetry, $\Theta=1$)*

* *Increasing porosity also means decreasing Poisson's ratio ν and an increasing elliptic shape.*

From knowledge in *Chapter 13* is known: Values for the increasing porosity f are strain-controlled detectable. The effect of a probably initially not pore-free material is captured in the initial property values.

14.1 Measurement of rod failure stresses and estimation of the vertex of the failure body

Even for a porous plastic failure body its vertex should be known from theoretical reasons. A vertex represents the equi-triaxial tensile strength capacity of a load-controlled strength situation, remind *Fig.15-2*. Because the vertex stress state $\{\sigma\} = (\text{true}\bar{R}^{\text{III}}, \text{true}\bar{R}^{\text{III}}, \text{true}\bar{R}^{\text{III}})$ with $TrF = \infty$ practically cannot be measured as best substitute a 3D-stress state - closest possible to the vertex - must be employed. Realistic is a stress state $(\text{true}\sigma_{ax} + \sigma_{hyd}, \sigma_{hyd}, \sigma_{hyd})$ by investigating the center of an un-notched tension rod test specimen, being the 'hot spot' in this test specimen.

In such un-notched rods a neck radius builds up and increases with further increasing axial tensile stress. Due to the diameter reduction a hydrostatic stress state is generated and can be determined from the zero volume strain regime faced in the minimum neck cross-section. Hereby, difference due to rolling of the sheet material and how the test specimen is cut out are neglected and full isotropy assumed.

From the test rig loading comes the subsequently effective stress 'true σ_{ax} ', whereas the remaining neck cross-section experiences in the center the multi-axial stress state $(\text{true}\sigma_{ax} + \sigma_{hyd}, \sigma_{hyd}, \sigma_{hyd})$, estimated by the Bridgman model. In order to better understand the stress situation in the rod center the effect of increasing σ_{hyd} is of interest, depicted below. It is to conclude from mechanics, that a hydrostatic stress does not change Mises's representative invariant J_2 for shape deformation of the solid. However, σ_{hyd} affects the tri-axiality value TrF which might be interpreted to cause some quasi-embrittlement of the material:

$$\begin{aligned}
 I_1 &= (\sigma_I + \sigma_{II} + \sigma_{III}) = f(\sigma), \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2 = f(\tau) \\
 &\quad (\text{true}\sigma_{ax} + \sigma_{hyd}, \sigma_{hyd}, \sigma_{hyd}), \quad \sigma_{ax}^I(r) = \sigma_I + \sigma_{hyd}(r) \\
 \sigma_{eq}^{Mises} &= \sqrt{3J_2} = \sqrt{3} \cdot \sqrt{(\sigma_1^M - \sigma_h - \sigma_h)^2 + 0 + (\sigma_h - \sigma_1^M - \sigma_h)^2} \rightarrow \sigma_{eq}^{Mises} = \sigma_1^{Mises} \\
 TrF &= \sigma_{mean} / \sigma_{eq}^{Mises} = (I_1 / 3) / \sqrt{3J_2} = [\sqrt{2} / 3] \cdot (I_1 / \sqrt{3}) / \sqrt{2J_2} \\
 I_1 &= (\text{true}\sigma_{ax} + 3\sigma_{hyd}) = f(\sigma), \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (0)^2 + (\sigma_{III} - \sigma_I)^2 = f(\tau) \\
 \text{uni-axial } \sigma_{ax}, \text{ multi-axial } (\sigma_{ax} + \sigma_{hyd}, \sigma_{hyd}, \sigma_{hyd}) &\text{ in the rod's minimum neck section } \Rightarrow \sigma_{eq}^{Mises}.
 \end{aligned}$$

Again: The use of notched rods is principally also possible but considering that the original notch radius ρ increases. Thereby the critical rod surface stress concentration reduces a little and the originally surface-located critical material location moves to the center. *Fig.15-9(left)* shall display

different stress states and the associated points on the respective TrF -beams. In the subpicture down left the indicated 2D stress-states and up left further the 3D stress states all collected in the table right down.

Of interest for the designing engineer is that the spatially formulated SFC $F^{NF} = 1$ dents the failure body at the pressure vessel situation $\{\sigma\} = (2,1,0) \rightarrow TrF = \sqrt{2}/3$, Fig.15-2 and 15-9. Remember: In the 2D principal stress plane F^{NF} is a straight line and in the 3D failure body a hyperbolic curve!

Fig.15-9 (right) shall make the non-linear development of TrF more clear and further make familiar with the design failure surfaces in the very ductile regime. The figure schematically shows that the strain-controlled failure surface is outside and thereby larger than the load-controlled one.

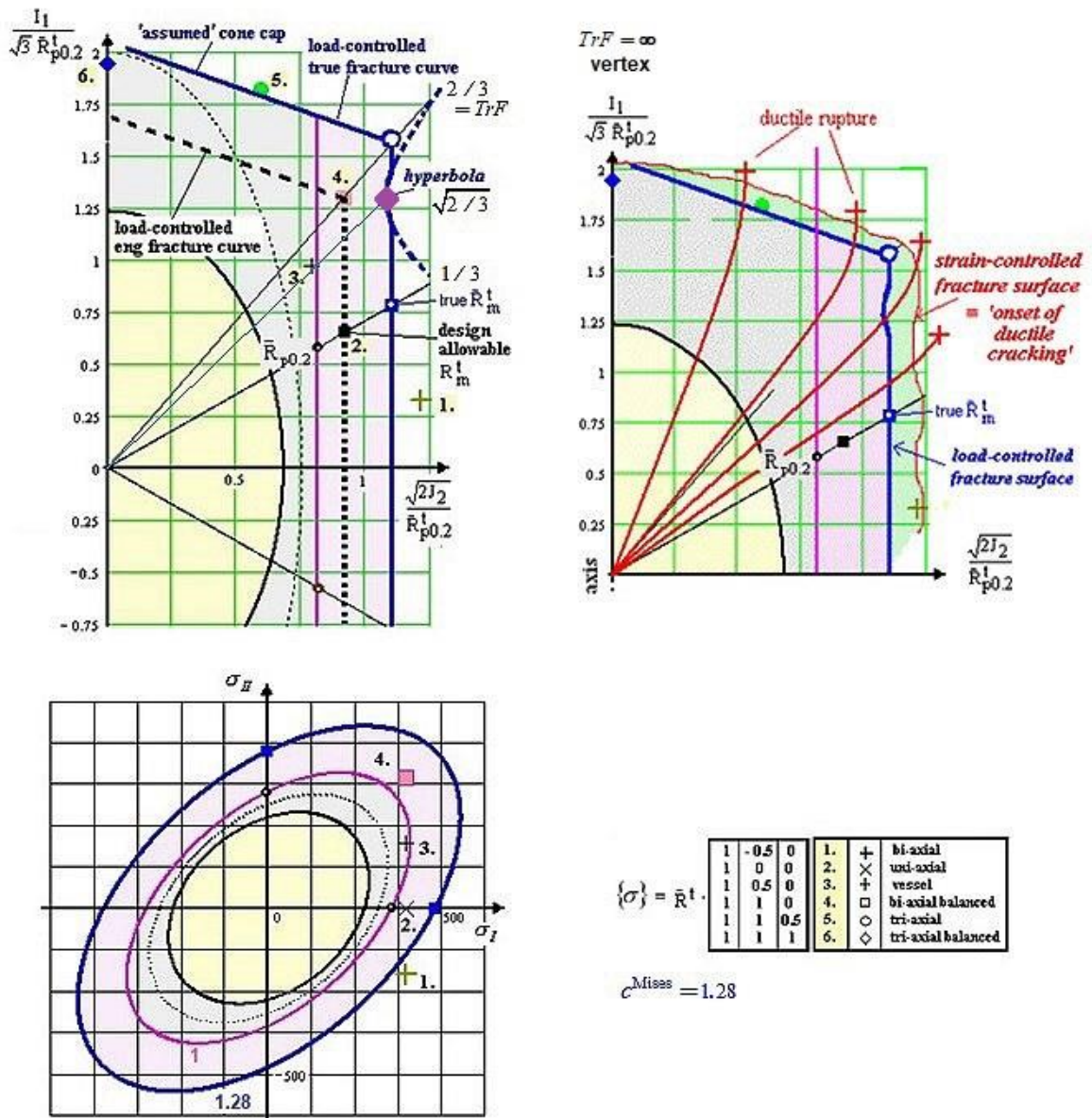


Fig. 15-9: Visualization of the effect of the TrF -beams and the related strengths, illustration of some stress state points and failure zones. 2D-potential surfaces on the inclined cross-section of the rotationally-symmetric failure body

15.4 Proposal of the Two Parameter 'Extended Mises' Yield function in the porosity domain

Extended Mises yield potential function

Originally, Gurson proposed for a metal, containing well distributed voids, a yield condition-based solution for a single spherical void. The model was modified later by Tvergaard and Needleman, including the porosity f and the increasing Flow stress σ_F of the 'matrix' material: The porous body, called bulk material (*smearred material*), consists of the matrix material and the voids or pores. The voids are nucleated in tension, only. The dense matrix phase follows the HMH ('Mises') model, and f represents the mean void volume fraction or porosity (*average value of a porous matrix*) as the so-called internal damage variable. For $f = 0$, fully dense material, the model reduces to that of von Mises, whereas a ultimate value f_{ult} implies that the material is ultimately voided that it has lost its stress carrying capacity due to local ductile rupture. Here, f_{ult} shall be replaced by the smaller $f_{odc} = f_{crit}$. Values for the increasing porosity f are strain-controlled detectable and therefore, the ratio is fixed. Table 15-2 describes the procedure how a relationship

Table 15-2: 'Comparison of Coefficients' of the models 'Gurson' \leftrightarrow 'Extended Mises' with σ_F as increasing true Flow stress as running stress variable

$$F^{GTN} = \frac{3J_2}{\sigma_F^2} + 2 \cdot f \cdot q_1 \cdot \cosh\left(\frac{I_1 \cdot q_2}{2 \cdot \sigma_F}\right) + q_3 \cdot f^2 = 1 \text{ ductile micro-damage failure function}$$

$$\text{simplified to } \frac{3J_2}{\sigma_F^2} + 2 \cdot f \cdot \cosh\left(\frac{I_1 \cdot q_2}{2 \cdot \sigma_F}\right) + f^2 = 1 \text{ appropriate for idea demonstration, } q_3 = q_3 = 1$$

If the cosh-function is replaced by the first two terms of the associated Taylor row [Cun98, Cun01]

$$\cosh x = \pm (1 + x^2 / 2 + \dots) \rightarrow \cosh(I_1 \cdot q_2 / 2 \cdot \sigma_F) = \pm (1 + (I_1^2 \cdot q_2^2 / 8 \cdot \sigma_F^2) + \dots)$$

The negative sign is to chose because porosity reduces strength capacity

$$\frac{3J_2}{\sigma_F^2} - 2 \cdot f \cdot \left(1 + \frac{I_1^2 \cdot q_2^2}{8 \cdot \sigma_F^2}\right) + f^2 = 1 \Rightarrow \frac{3J_2}{\sigma_F^2} - 1 \cdot f \cdot \left(\frac{I_1^2 \cdot q_2^2}{4 \cdot \sigma_F^2}\right) + 2f - f^2 = 1.$$

With $f^2 \ll f$ can be derived

$$\frac{3J_2}{\sigma_F^2} - f \cdot \frac{q_2^2}{4} \cdot \frac{I_1^2}{\sigma_F^2} - 2f = 1 \text{ with } q_2 = 1.5 \text{ as guess for the plastic damage flow function}$$

$$\frac{3J_2}{\sigma_F^2} - f^* \cdot \frac{I_1^2}{\sigma_F^2} - 2f = 1 \text{ with the elliptic shape parameter } f^*$$

$$\kappa = \frac{1 - 2\nu}{2 + 2\nu} \equiv f \cdot \frac{q_2^2}{4} = f^* \rightarrow \nu = (4 - f^*) / (8 + 2f^*).$$

\Rightarrow Failure state, normalized again with the shear strength, to insert is $\sigma = \bar{R}_{odc}$

$$F = \frac{3J_2}{\bar{R}_{0.2}^2} + f_{ult}^* \cdot \frac{I_1^2}{\bar{R}_{0.2}^2} + 2f_{ult} = 1 = Eff = 100\% \text{ material stressing effort,}$$

$F = 100\%$ = potential surface, which may be a fracture surface or a yield surface.

From 'Comparison of Coefficients' finally is obtained

$$\text{ExtendedMises } F = \frac{3J_2}{\bar{R}_{0.2}^2} + c_{12} \cdot \frac{I_1^2}{\bar{R}_{0.2}^2} = c^{\text{ExtMis}}, \text{ generally}$$

$$\text{Analogous to 'Mises' } Eff^{\text{Mises}} = \sigma_{eq}^{\text{Mises}} / \bar{R}_{0.2} = \sqrt{3J_2} / \bar{R}_{0.2} \Rightarrow Eff^{\text{ExtMises}} = \sigma_{eq}^{\text{ExtMises}} / \bar{R}_{0.2}$$

between the subsequent ‘Gurson’ type yield model and the ‘Extended Mises’ model was developed. A further equation is needed to determine the size parameter, such as with c^{Mises} of the ‘Mises cylinder’.

Void Porosity-linked reduction of Poisson’s ratio $0.5 > \nu$

Porosity means volume change due to void coalescence and volume change may be transferred to a decaying Poisson’s ratio, remind Beltrami. From the ExtM-model can be geometrically deduced $f^* = f \cdot q_2^2$ and $\nu = (4 - f^*) / (8 + 2 \cdot f^*)$.

Fig. 15-10 points out how the Poisson ratio is linked to the true strains (left), schematically to the true equivalent stress (center), and to the porosity f^* .

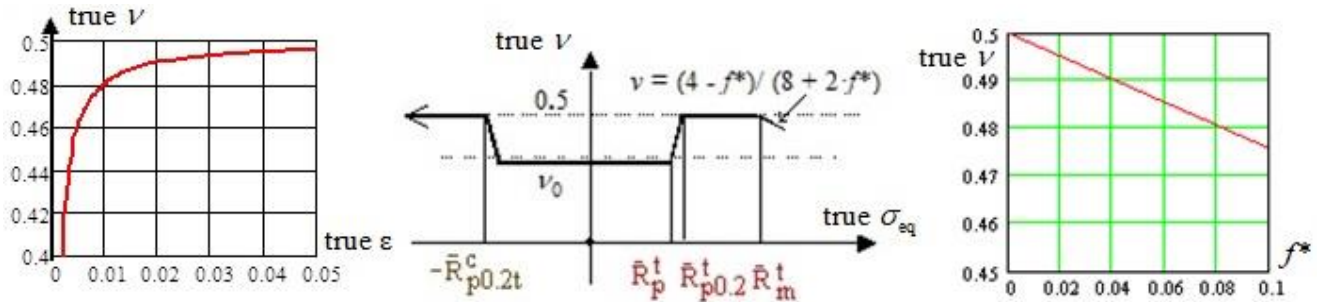


Fig. 15-10: Dependence of ν on the different parameters, the various regimes

Here, $f_{\text{ult}} (\bar{R}_{\text{odc}}) < f_{\text{rup}}$ is employed as that critical porosity which was dedicated by the author to ‘Onset of ductile cracking’, in order to ‘remain on the safe side’. The evolution function of f is assumed to follow an exponential course with practically $f = 0$ at the tensile strength point up to the defined ultimate value f_{ult} located at \bar{R}_{odc} .

15.3 Visualization of ‘Gurson’-model versus ‘Extended Mises’-model

Failure conditions enable the designer to assess multi-axial states of stress $\{\sigma\}$ by an equivalent stress σ_{eq} and to map multi-axial stress-strain behavior $\sigma_{\text{eq}}(\epsilon_{\text{eq}})$ via a measured, smeared stress F/A . For $f = 0$, fully dense material, the model reduces to that of HMH, whereas a maximum value f_{ult} implies that the material is ultimately voided that it loses its stress carrying capacity due to local ductile rupture.

The conventional visualization – as a parameter investigation - of the Gurson model is presented in Fig.15-11 (left) with f being the porosity parameter of the curves and q_2 a Gurson parameter from the comparison. A growing f means higher true stress but less cross-section or load-carrying material in the strain-controlled ‘hot spot’. This is displayed in the figure by the change of the cylinder shape versus an egg shape.

Another visualization, usually practiced in structural mechanics, is given by using the Lode-Haigh-Westergaard parameters. This leads to a change in the shape, Fig.15-11 (center). For $f = 0$ the Mises cylinder is obtained.

Fig.15-11 (right) depicts the various strength values such as $\text{true} \bar{R}^t, \bar{R}_{\text{odc}}$ as increasing true strength points to be inserted into the Extended Mises function size parameter, finally visualized as flow potential surfaces for four strength-linked porosity levels.

The parameter comparison with ‘Gurson’ let to take a reduced value $q_2 = 1.13$, however, due to missing test data the author sticks to 1.5. In this context, the respective ExtendedMises parameter c_{12}

can be determined, decoupled from the ‘Gurson’ Comparison of Coefficients, if having a reliable test data set available

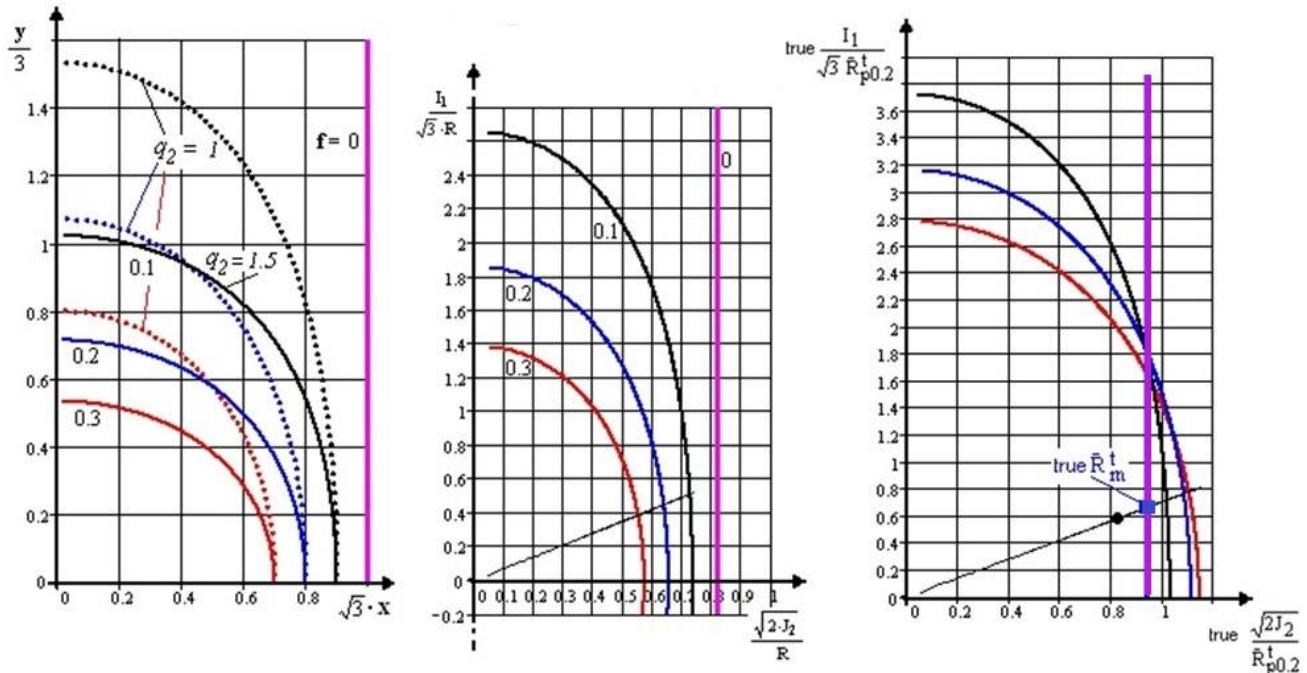


Fig. 15-11: Schematic comparison of the Gurson model (dots) and Extended Mises model Potential surfaces. (left) Display of curve parameter porosity f influence, using the ‘Gurson’ coordinates $x = (\sqrt{J_2} / \sigma_F)$, $y = (I_1 / \sigma_F)$, $\sigma_F = \bar{R}_{p0.2}^t$; (center) Display of the Gurson yield model in Lode-Haigh-Westergaard parameters $\bar{R}_{p0.2}^t =$ normalisation strength); (right) Potential surfaces of the ExtMises-model with four increasing true (graphs made about 2001)_AA2219, ($q_2=1.5$, $q_{2ExtM}=1.13$) $\text{true}\bar{R}^t, \bar{R}_{odc}$.

Table 15-3: Replacement of the Mises-based Bridgman curve $\sigma_{eq}(\epsilon_{eq})$ by an ExtMises one

Table 25-1: $\sigma_{ax}^t(r) = \sigma_1 + \sigma_{hyd}(r)$ and from Bridgman $\sigma_{hyd} \approx \sigma_1 \cdot \ln\left(1 + \frac{a^2 - r^2}{2 \cdot a\rho}\right)$

$\sigma_{eq}^{MisBri} = \sqrt{3 \cdot J_2} = \sigma_1 = \frac{F}{A} / (1 + 2 \cdot \rho / a) \cdot \ln(1 + 0.5 \cdot a / \rho)$, valid $\bar{R}^t < ?$,

$I_1 = \sigma_1 + 3\sigma_{hyd} = \sigma_1 \cdot (1 + 3 \cdot \ln(1 + 0.5 \cdot a / \rho))$ for the critical central ‘fiber’ at $r = 0$

considering Bridgman (above) and the notch-curvature change by Lorrek-Hill’s approach,

giving a maximum value for the unknown $\rightarrow \max \frac{a}{\rho} = \sqrt{\ln\left(\frac{1}{1-Z}\right) - \ln\left(\frac{A_0}{A_{oon}}\right)}$,

inserting $\sqrt{3 \cdot J_2} = \sigma_1$ and $\frac{F}{A} = \text{true}\sigma_{ax}$ the equivalent stress reads:

$\sigma_{eq}^{ExtMises} = \sqrt{\frac{3J_2 - f^* \cdot I_1^2}{1 + 2f}} = \sigma_1 \sqrt{\frac{1 - f^* \cdot (1 + 3 \cdot \ln(1 + 0.5 \cdot a / \rho))^2}{1 + 2f}}$

valid $\bar{R}^t < \bar{R}_{odc}$, (shape parameter) $f \cdot \frac{q_2^2}{4} \equiv f^*$, set $q_2 \cong 1.5 \rightarrow 1.13$.

Porosity parameter f and curvature parameter a/ρ increase from about 0 to the maximum at \bar{R}_{odc} .

The author’s full idea consisted of the two parts: Above ExtMises model plus porosity-improved

Bridgman evaluation, which was depicted in *Table 15-3*. The table displays all relations in order to establish the 'searched' equivalent stress $\sigma_{eq}^{ExtMises}$.

Reminder: to capture '120°-rotational symmetry' would require to replace J_2 by $J_2 \cdot \Theta$.

15.3 Visualization of the Bridgman-corrected true curve with consideration of porosity

In order to obtain a realistic equivalent stress curve it is physically mandatory to consider the increase of porosity f and the increase of the notch curvature by applying a / ρ . The mapping of the changing notch curvature and the changing porosity is shown below:

Mapping of the changing notch curvature: Data and determination procedure by Mathcad

$$\frac{a}{\rho} = \sqrt{\ln\left(\frac{A_0}{A_{rupt}}\right) - \ln\left(\frac{A_0}{A_{oon}}\right)} = 1.096 \quad \text{from} \quad \frac{A_{rupt}}{A_0} = 1 - Z = 1 - 0.20 = 0.80,$$

$$\frac{A_0}{A_{rupt}} = \frac{1}{0.80} = 1.25, \quad \frac{A_0}{A_{oon}} = \left(\frac{4.0}{3.89}\right)^2 = 1.057 \rightarrow \max \frac{a}{\rho} = 0.409 \quad \text{at} \quad R_{rup}.$$

Applying Lorrek-Hill's value Bridgman's approach delivers $\max\left(1 + \frac{2\rho}{a}\right) \cdot \ln\left(1 + \frac{a}{2\rho}\right) = 1.096$.

Then, for the previously proposed formulation the curve parameters can be computed:

$$\text{stressing} \quad \frac{a}{\rho} = a\rho = c_1 + c_2 + \text{true}\varepsilon_{ax} R_{odc}^2 + c_3 \cdot \text{true}\varepsilon_{ax} R_{odc}^3.$$

$$ap = 1.096 \quad \text{true}\varepsilon_{ax} R_{02} = 0.0071 \quad \text{true}\varepsilon_{ax} R_t = 0.055 \quad \text{true}\varepsilon_{ax} R_{odc} = 0.231$$

$$\text{Vorgabe} \quad c_1 := 1 \quad c_2 := 100 \quad c_3 := 1$$

$$1 = c_1 + c_2 \cdot \text{true}\varepsilon_{ax} R_{02}^2 + c_3 \cdot \text{true}\varepsilon_{ax} R_{02}^3$$

$$1.00005 = c_1 + c_2 \cdot \text{true}\varepsilon_{ax} R_t^2 + c_3 \cdot \text{true}\varepsilon_{ax} R_t^3$$

$$ap = c_1 + c_2 \cdot \text{true}\varepsilon_{ax} R_{odc}^2 + c_3 \cdot \text{true}\varepsilon_{ax} R_{odc}^3$$

$$Ap = \begin{pmatrix} 1 \\ -0.548 \\ 10.148 \end{pmatrix}$$

$$Ap := \text{Suchen}(c_1, c_2, c_3)$$

$$c_1 := Ap_0 \quad c_1 = 1$$

$$c_2 := Ap_1 \quad c_2 = -0.5476$$

$$c_3 := Ap_2$$

$$c_3 = 10.1478$$

$$Ap_j := c_1 + c_2 \cdot (\text{true}\varepsilon_{ax_j})^2 + c_3 \cdot (\text{true}\varepsilon_{ax_j})^3$$

Mapping of the changing porosity f : Data set used and determination by Mathcad

The set points of the curve are the porosity values at the tensile strength point R^t and at R_{odc} .

$$\text{Vorgabe} \quad e_1 := 0 \quad e_2 := 1$$

$$0.0002 = \left(e_1 \cdot \exp\left(\frac{\text{true}\varepsilon_{ax} R_t}{\text{true}\varepsilon_{ax} R_{odc}} - 1\right) - 1 \right)^{e_2} \quad f_{odc} = (e_1 \cdot \exp(1 - 1))^{e_2}$$

$$Af := \text{Suchen}(e_1, e_2)$$

$$Af = \begin{pmatrix} 0.6292 \\ 6.9469 \end{pmatrix}$$

$$e_1 := Af_0$$

$$e_1 = 0.6292$$

$$e_2 := Af_1$$

$$e_2 = 6.947$$

$$f_{exp_j} := \left(e_1 \cdot \exp\left(\frac{\text{true}\varepsilon_{ax_j}}{\text{true}\varepsilon_{ax} R_{odc}} - 1\right) - 1 \right)^{e_2}$$

Fig.15-12 displays the author's design verification idea, about 2000. The influence of the

practically starts at R_{odc} .

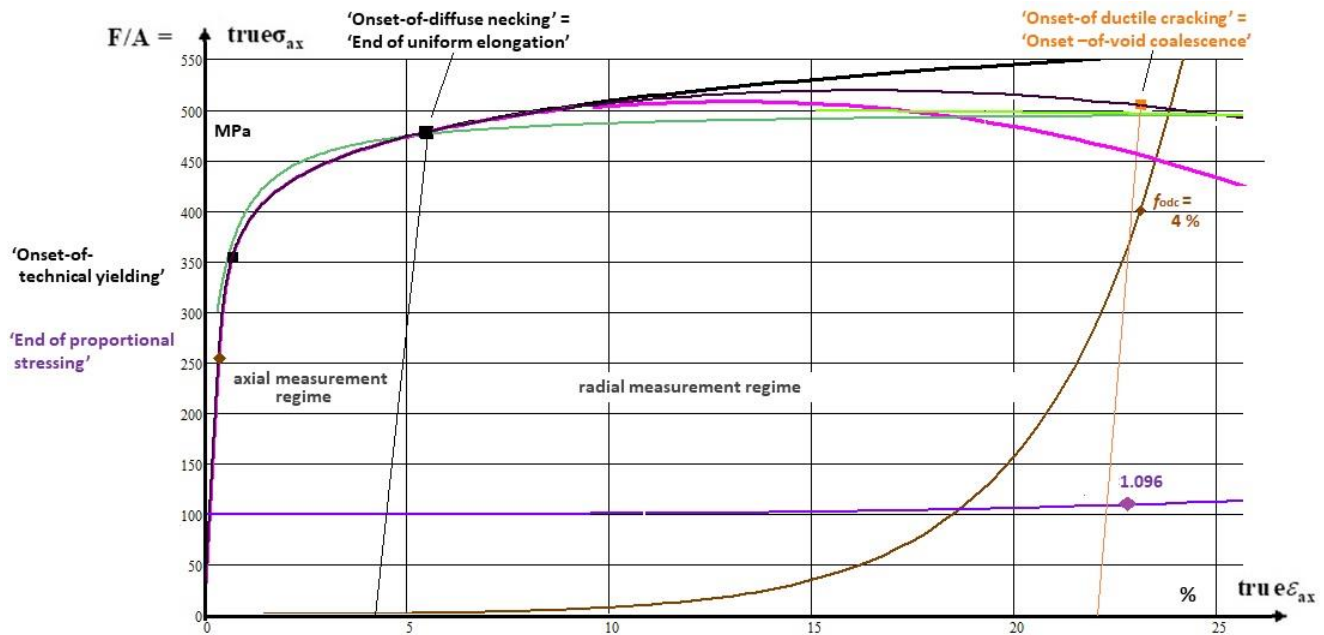


Fig. 15-12, AA2219, base material T2, 6 mm thick: Visualization of the *equivalent stress curve* $\sigma_{eq}^{ExtMises}$; *Ramberg-Osgood-mapped measured cross-section smeared axial stress F/A; Increase of plastic porosity f with $f_{odc} = 4\%$ at R_{odc} ; Increase of the notch curvature a / ρ with $a / \rho = 0.409$ at R_{ult} (replacing the higher R_{odc}); Increase of ν in the elastic-plastic transition domain approaching 0.5 and barely visible the decrease in the porous domain*

15.4 Specific Potential Surfaces being Strength Failure Criteria

Brittle ‘porous’ materials may still fracture in the elastic-plastic transition domain. For this fact, Ismar and Mahrenholtz [Is82] developed a Beltrami-based SFC model describing the failure behavior of a material between the proportional limit and the ‘onset of yielding’. In *Table 15-4* the SFC-formulations in all regimes shall be comparatively displayed. This includes potential surface descriptions and associate strength failure criteria SFCs.

LL:

- ✓ Whereas with the elasticity formulation of Beltrami the Poisson ratio ν is growing this is opposite with the formulation of a porosity-linked plastic model due to the increasing porosity
- ✓ The hypotheses of Beltrami, Mises, Gurson describe an increase or decrease of surfaces of constant potential. The shape of the surface theoretically begins with $\nu = 0$ (sphere, found with foams) growing up from $0 < \nu$ to $\nu = 0.5$ via the growing Mises cylinder keeping $\nu = 0.5$ and ending with an ellipsoid, which shrinks into a spherical direction represented by $0.5 > \nu$.
- ✓ For two domain limits a clear value for the varying Poisson ratio is given:

$$\text{proportional limit } \sigma \leq \bar{R}_{prop}^t \Rightarrow \nu = \nu_0 \quad \text{and} \quad \text{yield limit } \sigma = \bar{R}_{p0.2}^t \Rightarrow \nu = 0.5$$

- ✓ Designing requires to use limit state formulations, termed failure criteria (SFCs). These are fracture failure criteria for brittle materials namely for ‘Onset-of-fracture’ and yield failure criteria for ductile materials. In practice, for ductile materials these failures are ‘Onset-of-yielding’ and - for the author - ‘Onset-of-void coagulation = Onset of ductile Cracking’ in the case where strain-softening applies
- ✓ A Strength Failure Criterion represents a defined Design Limit State and is therefore a special
- ✓ critical Potential Surface F.

- ** The novel Extended Mises model just requires the determination of one more parameter, the porosity value f . All model parameters are measurable quantities.
- ** With the novel porosity-capturing σ - ε curve, being a ductile porosity-improved Bridgman correction, a simplified plastic analysis procedure could be achieved.
- ** For engineering reasons $true\bar{R}^t \Rightarrow \bar{R}_{odc}^t$ will represent the load carrying capacity to be considered.

Table 15-4, Isotropic materials: Determination of model parameters, single mode view.

| elastic-plastic | plastic | plastic porous |
|--|--|---|
| Beltrami | Hencky-'Mises'-Huber | 'Gurson' type |
| $\bar{R}_{prop}^t < \sigma_{Bel} < \bar{R}_{0.2}^t$ | $\bar{R}_{0.2}^t < \sigma_{Mis} < \bar{R}_m^t$ | $\bar{R}_m^t < \sigma_{eq} < \bar{R}_{odc}^t$ |
| \leftrightarrow | | \leftrightarrow |
| <i>stress - controlled</i> | | <i>strain - controlled</i> |
| σ := running variable of the subsequent potential surfaces | | |
| $\bar{R}_{prop}^t / \bar{R}_{0.2}^t < c^{Bel} < 1$ | $1 < c^{Mis} < \bar{R}_m^t / \bar{R}_{0.2}^t$ | |
| \leftrightarrow | | \rightarrow |
| ellipsoid | cylinder | ellipsoid |
| $I_1 = (\sigma_I + \sigma_{II} + \sigma_{III}) = f(\sigma)$, $6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2 = f(\tau)$ | | |
| $\frac{3J_2}{\bar{R}_{0.2}^t} + \kappa \cdot \frac{I_1^2}{\bar{R}_{0.2}^t} = c^{Bel} \leftrightarrow F^{Mis} = \frac{3J_2}{\bar{R}_{0.2}^t} = c^{Mis}, \kappa = \frac{1-2\nu}{2+2\nu}$ | | |
| Insertion of a (measurable) normalizing strength, yield strength point with $\nu = 0.5 \rightarrow \kappa = 0$ | | |
| $F^{Bel} = \frac{3\bar{R}_{0.2}^t / 3}{\bar{R}_{0.2}^t} + \kappa \cdot \frac{\bar{R}_{0.2}^t}{\bar{R}_{0.2}^t} = c^{Bel} \rightarrow c^{Bel} = 1 + \kappa = 1$, and κ an elliptic shape parameter | | |
| * Strength FailureCriteria (SFC), $\bar{R} \rightarrow R$, (with $\Theta = 1$ for full rotational symmetry) | | |
| R := strength design allowable, marking a special potential surface \equiv design limit state | | |
| elastic, very brittle | ductile | very ductile |
| $Eff = \frac{\sigma_I}{\bar{R}_m^t} = 1$ * | \leftrightarrow | $Eff = \frac{3J_2}{\bar{R}_m^t} = 1 \leftrightarrow Eff = \frac{3J_2}{\bar{R}_{odc}^t} + c_{12} \cdot \frac{I_1^2}{\bar{R}_{odc}^t} - 2f_{odc} = 100\%$. |
| For similarity reasons: for the 2 modes Normal Fracture NF , Shear Fracture SF (brittle) | | |
| and after inserting $\sigma = R \cdot Eff$ and dissolving for Eff follows | | |
| $I_1 > 0 : F^{NF} = 0.5 \cdot \frac{\sqrt{4J_2 - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t} = 1$; $Eff^{NF} = 0.5 \cdot \frac{\sqrt{4J_2 - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t}$ | | |
| $I_1 < 0 : F^{SF} = c_1^{SF} \cdot \frac{3J_2}{\bar{R}^{c2}} + c_2^{SF} \cdot \frac{I_1}{\bar{R}^c} = 1$; $Eff^{SF} = \frac{c_2^{SF} \cdot I_1 + \sqrt{(c_2^{SF} \cdot I_1)^2 + 12 \cdot c_1^{SF} \cdot 3J_2}}{2 \cdot \bar{R}^c} = \frac{\sigma_{eq}^{SF}}{\bar{R}^c}$ | | |
| $c_1^{SF} = 1 + c_2^{SF}$ with direct consideration of the Poisson ratio $c_2^{SF} = (1 + 3 \cdot \mu) / (1 - 3 \cdot \mu)$ | | |
| Last unknown to be searched is the elliptic shape linked parameters such as c_1^{SF} by insertion of a bi-axially compressive failure stress or a fracture angle $\mu = \cos(2 \cdot \theta_{fp}^c \cdot \pi / 180)$. | | |

Non-linear stress–strain analysis Note:

Usually Co-axiality, Prandtl-Reuss equations and an Associated Flow Rule is employed in order to predict strain rate $\dot{\varepsilon}_{ij}$ and the Lagrange multiplication (*proportionality*) factor λ .

16 Note on Continuum (micro-)Damage Mechanics (CDM)

Aim: Primarily checking CDM application whether it is mature for a reliable Static Design Verification.

CDM is applied for ductile and brittle materials. The loading may be static and cyclic, with the latter requiring fatigue investigation. Regarding stress-strain curves, CDM principally captures the load-controlled hardening part and the deformation-controlled softening part. Softening part examples are the still mentioned embedded UD layer (*Fig.16-1*) and the ductile metal tensile rod described in the last Chapter by a porosity-capturing ‘Gurson’ model. Results of isotropic analyses, employing the softening curve branch, can be used to better design notches, openings in pressure vessels (fuel tank task in Ariane 5 upper stage) etc.

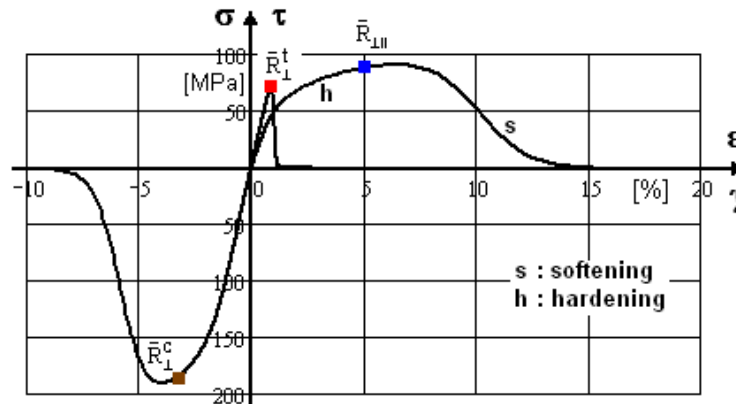


Fig.16-1, example UD ply: Full stress-strain curve with load-controlled hardening and deformation-controlled softening of the layer (ply) embedded in a laminate

CDM is pretty linked to multi-scale modelling, which will be looked at in the next Chapter. All materials are generally composites. Applying CDM one goes down to the constituents of a composite to metallic grains or to fiber and matrix for instance. Moving down on the scales it is helpful to use the physical formulations gained on the macro-scale such as Mises yielding with ductile metals in the tension and compression loading domain and Mohr-coulomb friction behavior of brittle materials in the compression domain. Shear stress loading is composed of a tensile stress with a compressive stress. This activates two failure modes, which leads to normal fracture in the case of brittle materials. These physical effects stay valid at the lower scale and are to consider *adjusted*.

LL:

It is always to check, whether a Mises yield criterion can be applied to quantify micro-damage portions or a fracture criterion in the case of very brittle behavior, i.e. Fiber Reinforced Plastics (FRP) experiencing matrix yielding:

16.1 Static Behavior

Micro-damage formulations:

CDM is basically used to capture the evolution of the micro-damage state from micro-damage $D = 0$ up to ‘Onset-of-Failure’ at $maxD$, which is for brittle materials at the end of hardening or at achieving the strength R .

In CDM, the formulation of the describing constitutive equation is based on one of the following two approaches (*Here the stress-strain curve is meant*):

- (1) The strain equivalence principle approach or on
- (2) The stress equivalence principle approach.

From engineering side, the latter is preferred because 3D stress states and residual stresses have to be considered in design dimensioning.

The constitutive relationships are formulated in the effective undamaged configuration $\sigma_{eff} = \sigma / (1 - D)$ with a stress-strain relation σ linked by the stiffness elasticity matrix $[C]$, which reduces due to growing micro-damage. *Fig.16-2* exemplarily depicts the relationship for a 2D-loaded transversely-isotropic UD material. By inversion of the effective compliance matrix S_{eff} the decaying stiffness matrix C_{eff} is obtained.

$$\{\sigma\} = [C] \cdot \{\varepsilon\} \rightarrow \{\varepsilon\} = [S] \cdot \{\sigma\} \quad \text{as practical test-data evaluation formulation}$$

$$S_{eff} = \begin{bmatrix} \frac{1}{E_1 \cdot (1 - D_{11})} & \frac{-\nu_{21}}{E_1} & 0 \\ \frac{-\nu_{12}}{E_2} & \frac{1}{E_2 \cdot (1 - D_{22})} & 0 \\ \text{(symm)} & 0 & \frac{1}{G_{12} \cdot (1 - D_{66})} \end{bmatrix} \quad \text{with } D = \begin{bmatrix} D_{11} & D_{21} & 0 \\ D_{21} & D_{22} & 0 \\ \text{(symm)} & 0 & D_{66} \end{bmatrix}$$

usually not considering the off-diagonal D_{21} .

Fig.16-2, 2D-example UD material: Compliance matrix [S] and micro-damage matrix [D].

The D_{ij} represent the accumulation of the micro-damage process portions and are theoretically terminated by $\max D$ at the tensile strength point in the case of brittle materials and at the rupture point for very ductile isotropic materials. These portions may occur during a monotonically increasing static loading. For brittle materials micro-damage starts at the ‘elastic end’ being a level where Eff has still reached a value, see *Fig.16-3*. Unfortunately, $\max D$ in static CDM cannot become 100% due to its usual modelling basis! The center figure outlines how a stress-man views the ‘onset

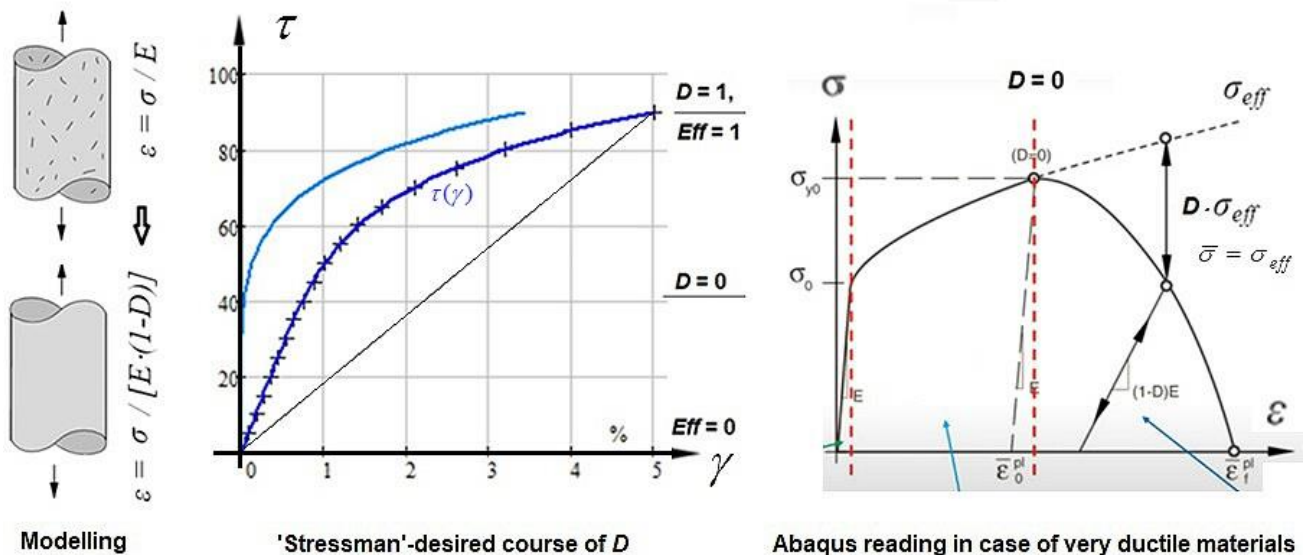


Fig.16-3: The various 'Onset- of- Failure' envelopes: (left) Smearing of the micro-damaged material, (center) shear of a slightly brittle material, (right) Ductile material (Ansys FEA code)

of micro-damage' of a slightly brittle material. In the elastic domain $< R_{prop} \equiv R_{e(lastic)}$ there is no D-contribution. The blue 'flow curve' then will contribute.

The right figure (from Abaqus) surprisingly outlines that there micro-damage first begins with void nucleation and coagulation which rises the Question:

Does really not any micro-damage happens below R^t ??

Micro-Damage-free (in German *schädigungsfrei, nicht schadensfrei*) and crack (= macro-damage, in German *Schaden*)-free does not mean free of flaws.

LL:

* CDM is generally always good for understanding static & cyclic material behavior

* Confusing is faced regarding 'onset of counting micro-damage' portions in static case: once $< R^t$ but also $> R^t$

Material behavior-determined slip and failure angles:

The number of slip systems in ductile metals is usually high, and those that are active possess an orientation near to the planes with maximum shear stress. Under uniaxial loading the planes of micro-cracks are always inclined approximately 45° to the direction of the applied tensile stress, see (Fig.16-4). In single crystals, the lattice structure is spatially oriented in such a way that a sliding plane is obtained at an angle of 45° . In poly-crystalline metals with randomly distributed lattice sub-structures this will change a little.

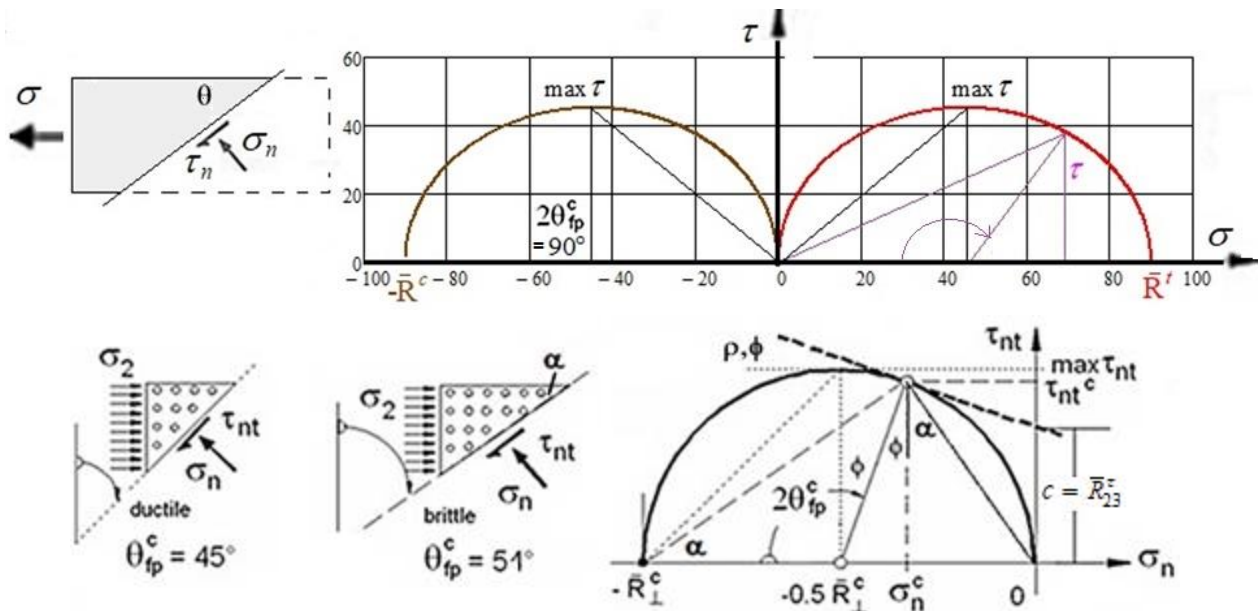


Fig. 16-4, very ductile metal material: (up) Mohr stresses and failure angles. (below) Mohr stress circle for a compressive and a tensile uniaxial external stress of a semi-brittle material

$$\tau_n = \sigma \cdot \cos(\alpha) \cdot \sin(\alpha) \text{ with } \alpha \text{ the angle to } \sigma \text{ direction, } 2 \cdot \max \tau = \sigma \text{ for } \alpha = 45^\circ$$

Known from brittle material behavior under compression is: *The failure angle depends on the friction value μ .* After the formula, derived in [Cun23c], the computation of the failure angle with the

Mohr-Coulomb model delivers exemplarily for a material friction value $\mu = 0$ (= fully ductile) the expected value of 45° and for a friction value $\mu = 0.2$ the angle 51° , see Fig.16-5. The author presents in this figure that the angle changes from the 51° at the compression strength point \bar{R}_\perp^c up to 90° at the tensile strength point \bar{R}_\perp^t .

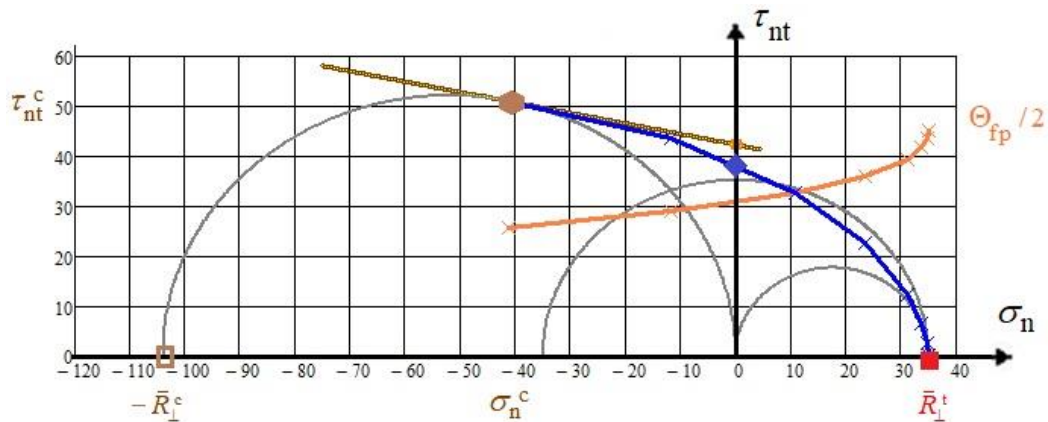


Fig.16-5, brittle UD-material: Joint display of the UD failure curve in Mohr stresses, indicating an fracture angle increase Θ_{fp}° when approaching \bar{R}_\perp^t . Shear fracture plane angle in the touch point 51° and linear Mohr-as well as a more realistic curved Mohr-Coulomb friction curve. Touch point is defined by $(\sigma_n^c, \tau_{nt}^c)$, linked to \bar{R}_\perp^c .

16.2 Cyclic Behavior of Ductile Metals applying Micro-scale Material Modelling

Once micro-cracks have nucleated due to strain accumulation from cyclic slipping, they grow in the early stage typically in the order of the material's grain size (text from M. Mlikota - S. Schmauder: Thanks to Siegfried). In the course of further cyclic loading, micro-cracks, formed along these slip bands, will grow and link together. In metals and alloys they grow predominantly along the crystallographic planes because they are highly affected by microstructural barriers such as grain boundaries or other micro-structural features. The coalescence of trans-granular micro-cracks, namely, if two micro-cracks meet each other at the same grain boundary, is performed in the numerical simulation of the crack initiation after Tanaka-Mura. It occurs if the average stress in between their tips surpasses the elastic limit Re of the material's new micro-crack, created on this grain boundary line, uniting the two trans-granular micro-cracks into a single one (example pure iron $Re = 260 \text{ MPa}$).

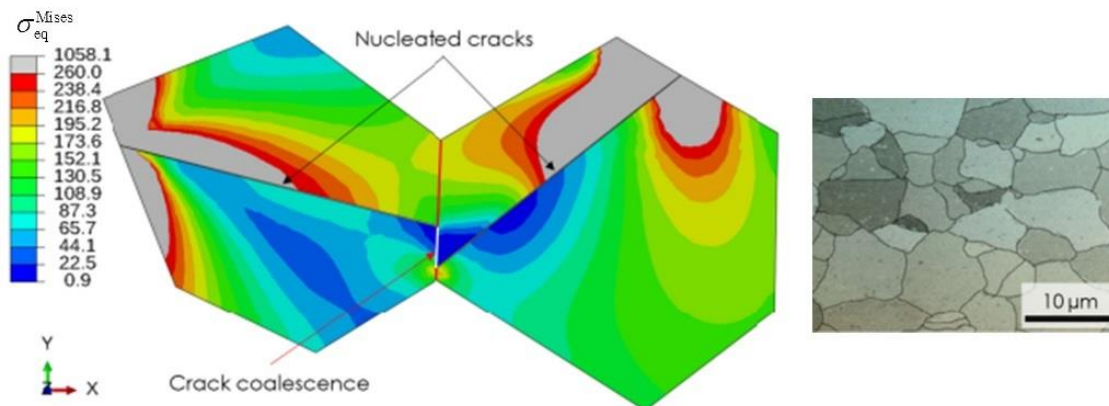


Fig.16-6: Simulation of AA micro-crack coalescence (Lorenzino, P., Navarro, A. & Krupp, U. (2013), 'Naked eye observations of microstructurally short fatigue cracks', Int. J. of Fatigue 56(0), 8-16.

Already nucleated crack segments tend to extend along the whole grain, causing local stress relaxation as well as concentrations at their tips and by that amplifying the likelihood for new crack formation in the vicinity. In the course, micro-cracks form along the slip bands, grow and join.

The change of the crack plane from the crystallographic plane to a non-crystallographic plane perpendicular to the external stress axis is called the transition from Stage I (*crystallographic growth*) to Stage II (*non-crystallographic growth*) or transition from the micro-crack initiation to a micro-crack growth stage resulting in a short crack, as depicted in *Fig.16-6*.

However, the dominant short crack does not always continue propagating. Namely, in the case of a lower stress level, the short crack may stop growing. Such a situation is typically known as run-out, which indicates that at very low stress levels an infinite life may be obtained. Run-out below the endurance limit means crack-retardation, *Fig.16-7*. In the long-crack regime the fatigue crack growth rate da/dn can be characterized by the stress intensity factor range ΔK as a dominant driving parameter.

The CDM-driven Region I in the figure below is here of interest, but should be illustrated as part of the full crack failure picture: A typical fatigue crack growth rate curve da/dn (ΔK) for the long crack is illustrated in *Fig.16-7*, too. If in a double logarithmic scale the long crack propagation rate follows a straight line in Region II, in sufficient distance from the threshold ΔK_{th} , then the long crack growth rate domain can be well described for most engineering alloys by the so-called Paris law:

$$\text{Paris: } da/dn = C_{Paris} \cdot \Delta K^{n_{Paris}} \quad , \quad \text{Forman: } da/dn = \frac{C_{Forman} \cdot \Delta K^{n_{Forman}}}{(1-R) \cdot K_{Forman} - \Delta K} \quad [\text{HSB 63205 - 01}]$$

In the figure and in the formulas above da/dn is the crack growth increment per cycle, $\Delta K = \max K - \min K$ is the range of stress intensity factor, and C (intercept with the y-axis) and n_{Forman} (slope) are material curve parameters that are deduced by fitting the course of experimental data. K_{Ic} is the so-called fracture toughness.

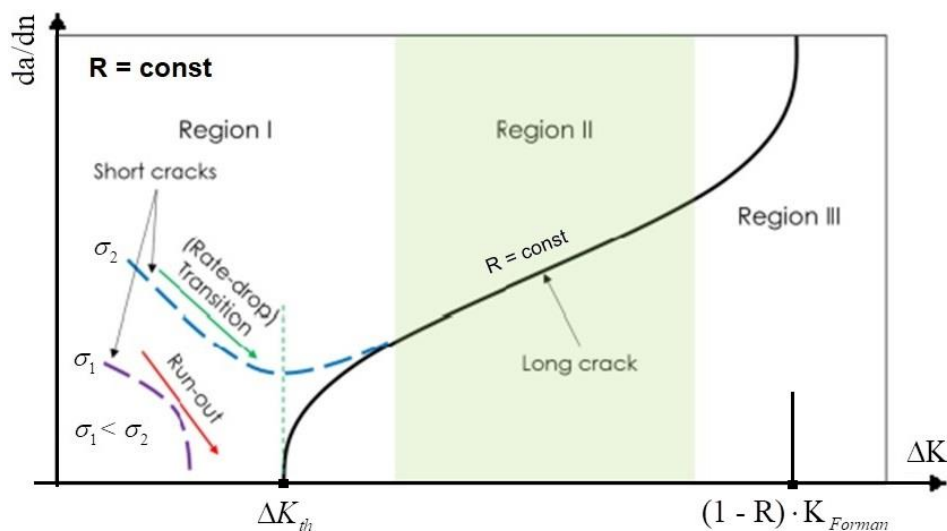


Fig.16-7: Fatigue growth rates of micro-cracks (short) and long cracks in dependence of Δ stress intensity factor. Schematic representation of the loading level- dependent transition from region I into region II.

n = number of cycles, a is crack size-

LL:

- * There is a hope, that in future for metals a basis will be provided, that the estimation of an endurance limit will be possible.
- * A grain is usually polycrystalline with crystal planes in various spatial orientations. Hence, a metallic 'composite' material can be only termed homogeneous and isotropic if these orientations are randomly distributed in order to become quasi-homogeneous. By the way, this is the same for an isotropic short fiber-reinforced polymeric material, otherwise the so-called orientation tensor has to take care of the non-isotropy.

16.3 Note on Application of Continuum (micro)-Damage Mechanics (CDM) in Static Strength

Note on Stress effort Eff versus micro-damage development D :

For the designer of interest is how the material's stiffness decreases with increasing stress effort or load, respectively. Design allowable R and average strength \bar{R} lead to different stress efforts in design verification and in modelling of material damaging (50% value = highest expectance probability), see Fig.16-8. The enlarging effect of the design FoS j on the value of Eff , when reaching failure, is considered in the design verification curve (dashed line) depicted below. The more reserve is, indicated by a positive Margin of Safety MoS , the lower Eff is. This has an effect on the actual strain in the non-linear analysis case. It becomes smaller and the strain is less plastic, which is of interest for the validity limit of an elastic analysis.

In the case of 3D modal SFCs (for comparison) the common micro-damage-caused degradation is considered by an interaction equation that reflects the micro-damage influence of all acting stress states and associated modes. The single mode efforts are interact via the experience-based interaction exponent m being about $m = 2.6$.

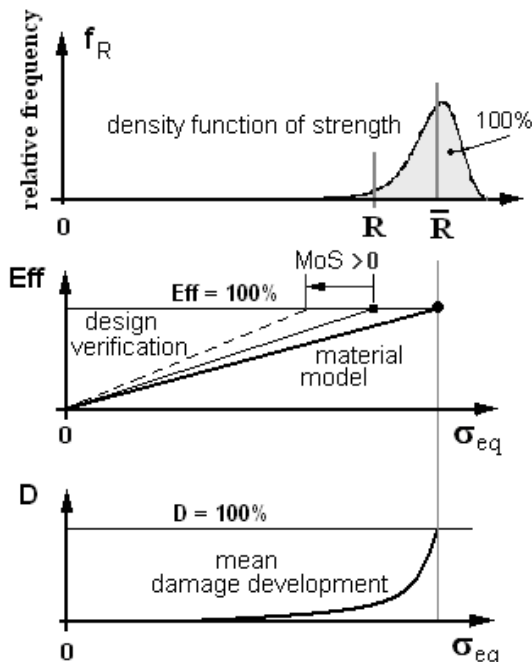


Fig.16-8:

Visualization of the development of stress effort, strength value, equivalent strength, and

Micro-damage understanding of a 'stressman'.

$$MoS = RF - 1$$

‘Stressman’s’ Assessment of CDM applications:

During his engineering life CDM was often propagated to make in future a Design Verification possible. In literature, i.e. [Jai20], Continuum (micro-)Damage Mechanics (CDM) models are also used to determine a RF . However, this intention faces some obstacles.

Analogous to the standard procedure then statistically-based micro-damage model parameters would be required and a total maximum value D is to define according to $D < D_{admissible} < 100\%$ at failure and this must be statistically based. Defining such a D -value is a challenge for the application of (micro-)Damage Models in the Design Verification (DV) for serial production certification. This challenge is novel and higher than providing the classical strength design allowables R , necessary for computing Eff .

Further, in known standard procedures Eff runs $0 < Eff < 100\%$, whereas D begins at a distinct Eff -value but should principally also end at 100%, see [CUN22, §15.3]. Here, a very essential question comes up: “*How does the designer assess a stress level that is below the ‘onset-of-micro-damage’?*” In this context another question arises: “*How are to consider low stresses in Low Cycle Fatigue?*” The provision of a CDM-failure body would be mandatory for obtaining DV. Hence, up to now CDM seems not to meet the authority-demanded DV-requirements regarding the statistically reduced design strength R and regarding the relationship $\sigma \sim R \cdot Eff$, which is valid in the linear elastic and in the non-linear regime.

LL:

- * *Stiffness decay CDM model parameters are difficult to apply*
- * *The ‘stress-man’ will not understand that at maximum load, which is at the strength point, the sum of micro-damage does not approach 100%.*
- * *The author could not sort out a consistent procedure that might be used in design verification. A clear derivation of the maximum micro-damage values seems to be missing.*
- * *How is the interaction of the damage portions in 3D-CDM solved?*
- * *Stiffness decay CDM model parameters are difficult to apply*
- * *Looking at ‘well analyzing’, which requires well-mapping of the stress-strain behavior in the hardening domain, one should always remember the scatter of the measured curves.*

Engineer’s question, regarding above body text:

Is it possible to provide the engineer with similar design verification information when using micro-damage quantities D_i ?

Fig.16.-9 left shows the scatter and distributions of some strain curves depicting strength and strain quantities.

Fig.16.-9 right up demonstrates that a compression test can, due to barreling, can just give a value for the yield strength $R_{0.2}^c$. This requires the determination of the increased hoop diameter, when aiming at realistic $R_{0.2}$ - and E -values for tensile and compression. The figure also informs that for a static test specimen of a product the directions are marked by the subscripts L, LT and ST and that these are used for the description of sheet-type test specimens. These specimens are machined in the rolling direction (letter L), transversal direction (T) and thickness direction (S). In the case of thick structural

parts smooth tension bar test specimens are cut out, in the case of thin plates flat test specimens are investigated, which better represent 2D-structural shapes.

This is similarly performed for the radial and axial direction of a cylindrical test specimen.

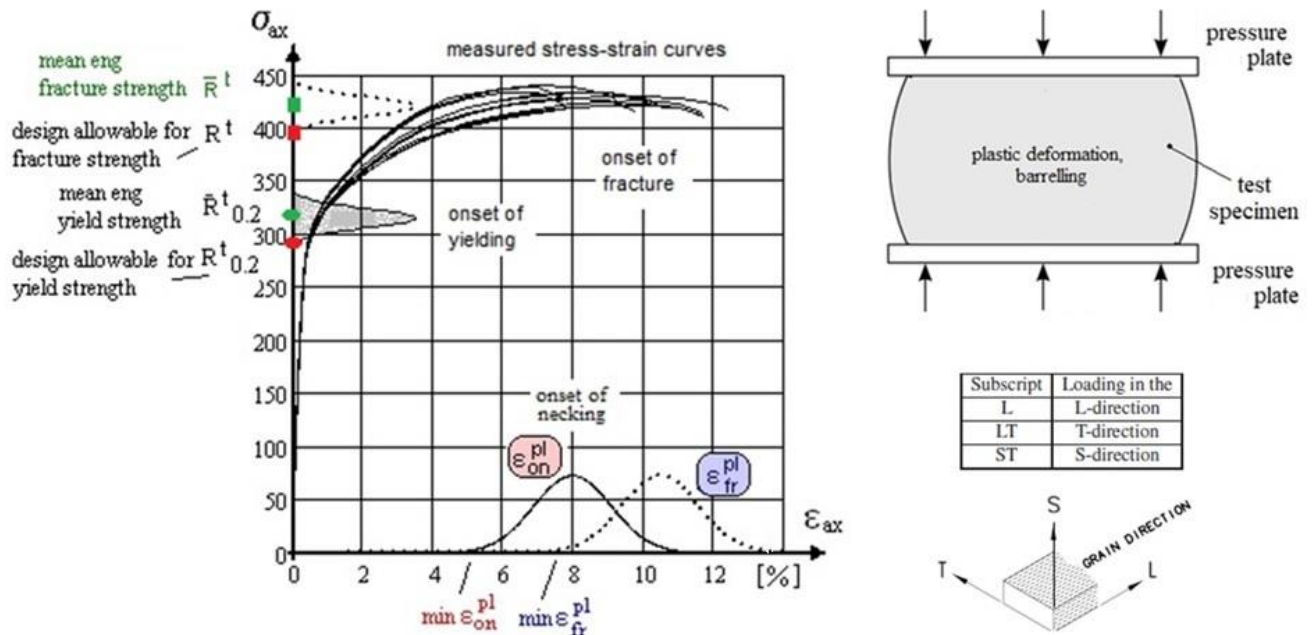


Fig.16-9: AA2219 engineering quantities and curves, deformation of a compressed ductile test specimen. (right down) Marking of sheet-type test specimens

Eventually Fig.16.10 shall show the shape of the tensile rod test specimen and a picture of the porous fracture surface of the ductile material used

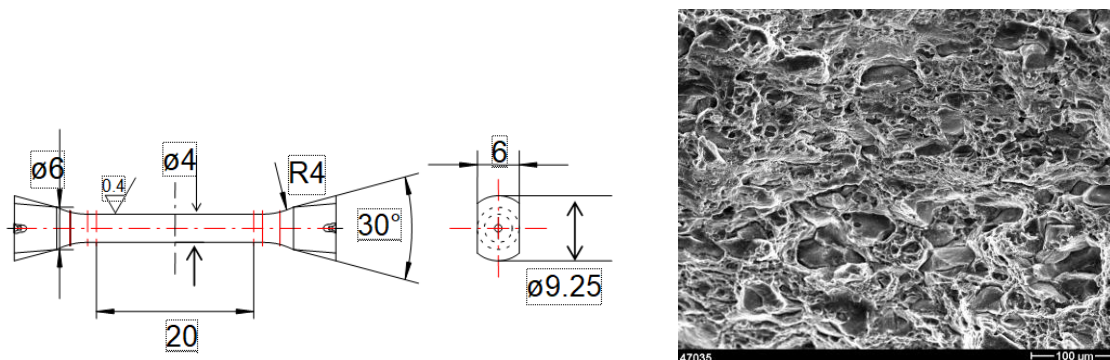


Fig.16-10: (left) Geometry of the tensile rod; (right) Voids on the fracture surface [IWM]

LL:

- * Before executing any analysis with a distinct code the designer has to check whether the actual stress-strain curve fits to the shape of the implemented curve
- * For the best possible estimation of the component behavior, the average stress-strain curve $\bar{\sigma}\bar{\epsilon}$ must be taken
- * The average stress-strain curve $\bar{\sigma}\bar{\epsilon}$ does not inescapably run through the means of yield $(\bar{\sigma} - \bar{\epsilon})_{yield}$ and of fracture $(\bar{\sigma} - \bar{\epsilon})_{fr}$.

17 Multi-scale Structural modelling with Material Modelling and some Analysis

Aim: Making aware of limits when applying validated macro-scale formulations at lower scales.

17.1 Structural Analyses over the Scales

Structural modelling with associate analyses is performed at many scales, see Fig.17-1, from the macro-scale up to the Burj Khalifa building size.

Thereby, the challenging task is the input of the right material properties: Which values are to insert when analyzing at the lower scale? What about the stress-strain curve, and which for instance for the anisotropic UD material remains always bound to the macro-scale?

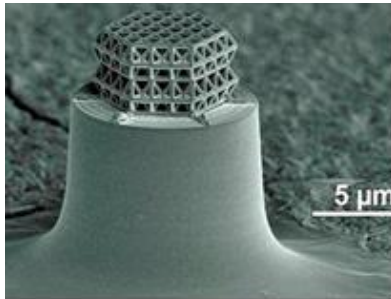


Fig.17-1: Size variety of structures.

(left) Truss structure, created by J. Bauer and O. Kraft with laser lithography. Glass-like carbon nano-framework $R^c = 3000$ MPa. Advanced Materials, Progress Report, 'Nanolattices: An Emerging Class of Mechanical Metamaterials'. Jens Bauer, Lucas R. Meza, Tobias A. Schaedler, Ruth Schwaiger, Xiaoyu Zheng, Lorenzo Valdevit. 2017, Wiley Online Library



Burj Khalifa, 828 m

All this requires investigating the applicability of the usual macro-scale formulations especially concerning static strength, fatigue and fracture mechanics. For the assessment of a stress state, when viewing Design Verification (DV), it is to know the 'Onset-of-micro-damage' and the later following 'Onset-of-micro-cracking'.

Multi-scale modelling is executed for static and cyclic problems. In the cyclic case, there are three key 'points' that separate the regions in Fig.17-2:

- Ultimate strength R_m^t : Stress level required to fail with one cycle, $n = 1$
- Onset of Yield, R_e : Stress value at onset of plastic behavior with being $R_e < R_{0.2}$
- Endurance limit $S_{e(\text{endurance})}$: Stress corresponding to the horizontal asymptote of the SN-curve.

The course of the cyclic failure test data, termed SN-curve, is again mapped by the 4-parameter Weibull formula $R = \text{constant} : \sigma_{\max}(R, N) = c_1 + (c_2 - c_1) / \exp(\log N / c_3)^{c_4}$.

As the average SN-curve cannot be applied in fatigue life DV, a statistically reduced curve is to determine as design curve. This design curve defines a full $D_{\text{design}} = 100\%$ -SN-curve from the tensile strength as original point and ends in the running-out defining an endurance limit stress.

17-2 Macroscopic SN-curve with Relation Material Stressing Effort $Eff \leftrightarrow$ Micro-damage D

There are practically two possibilities to present SN curves:

- (1) Ductile: Applying the stress amplitude $\sigma_a(R, N)$, also termed alternating stress
- (2) Brittle: Applying the upper stress $\sigma_{\max}(R, N)$

The maximum stress is physically simpler to understand by the 'stress-man' than the amplitude, according to smooth transfer from the static to the cyclic behavior, Fig.17-2.

Namely, a decaying SN curve is interpretable like a decaying ‘static’ strength after a micro-damage process with n cycles.

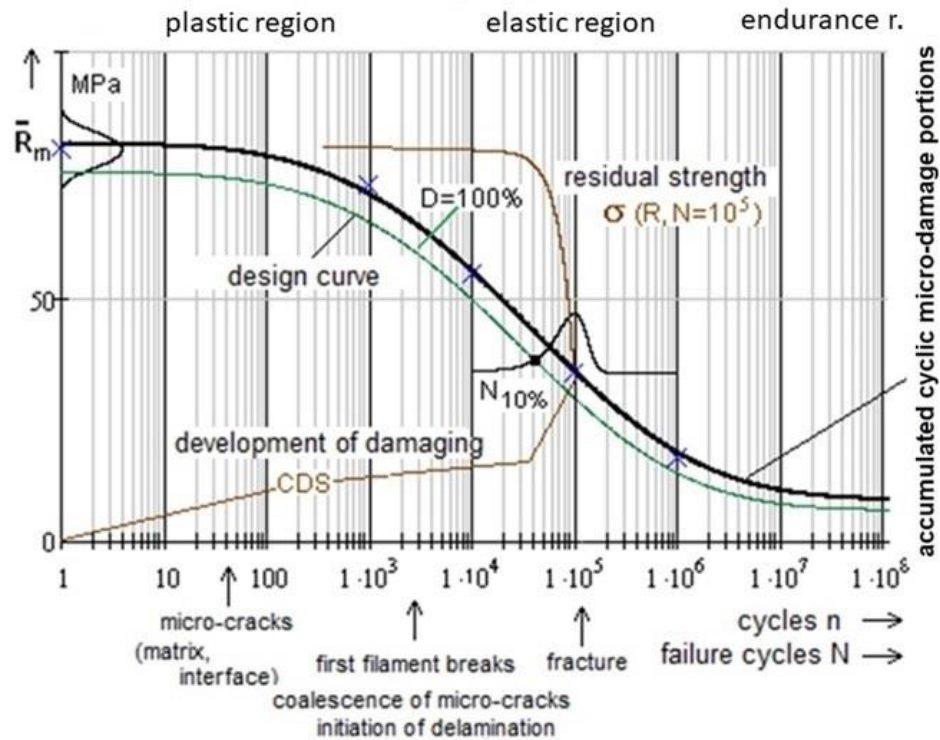


Fig.17-2, Design Verification: Fatigue average curve and **design curve $R = 0.1$** . $D = D_{design}$ for a survival probability P with a confidence level C . CDS is ‘characteristic damage state’ of a lamina

[Hiatt, J. (2016), 'What is a SN-Curve?', Technical report, Siemens PLM Community). $N_f = N_{initial} + N_{crackgrowth}$. Run-out below the endurance limit means crack-retardation]

Thereby, the static material stressing effort Eff (Werkstoffanstrengung, $N_f = 1$) is replaced by the accumulated cyclic micro-damage sum $D(N)$. Applied here is the classical 4-parameter Weibull curve with one parameter still fixed as strength point origin, because for brittle materials the strength value $\bar{R}^t = \sigma_{max}$ ($n = N = 1$) is preferably used as origin in the tension domain and anchor point of the SN curve and in the compression domain $-\bar{R}^c = \sigma_{min}$ ($n = N = 1$). In detail, [Fig.17-3](#) visualizes the transfer from the static load-driven increase of the material stressing effort ($n = N = 1$) $Eff = 100\%$ (expectance value 50%) at the strength point to the cycle-driven micro-damage sum $D_{mapping} = 100\%$ (expectance value 50%) of the SN curve. The evolution of Eff is not linked to the accumulation of the micro-damage. At onset-of-micro-cracking Eff is still > 0 .

If static failure $\rightarrow \max \sigma = \bar{R}_{static}$ at $Eff = 1$ and if cyclic failure $\max \sigma = \bar{R}_{cyclic}$, at $D = 1$.

LL:

- * It is always necessary to check whether the material at the lower level behaves in such a way that physically-based macro-mechanical formulations can be used
- * The material data input should satisfy physical model demands, which includes measurable parameters
- * DV demands for a statistically reduced SN-curve.

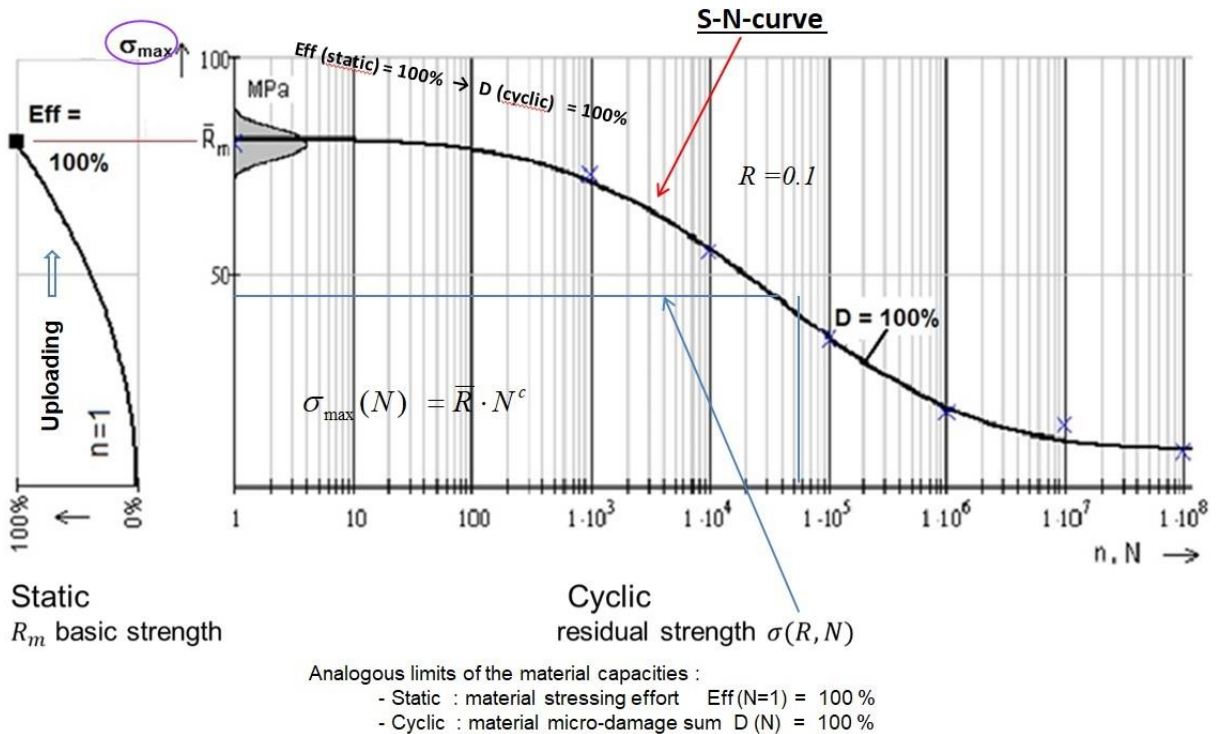


Fig.17-3, Mapping: Eff versus D. Mapping deals with averages \equiv 50% expectance value

17.3 Multi-scale Material Modelling regarding Infinite Life (endurance limit) of Metals

Infinite life or, in other words, the endurance limit is an ever-lasting topic of highest interest in structural design and concerns all materials.

Nowadays, valuable investigations on the micro-mechanics level seem to bring a significant progress for isotropic metals by using Continuum Damage Mechanics (CDM).

Mlikota and Schmauder found that the so-called Critical Resolved Shear Stress CRSS is the relevant fatigue-responsible quantity, (Fig.17-4), regarding the behavior of ductile metals in the micro-scale regime. Multi-scale Material modelling (MMM), based on enough computer power will probably allow in future 'Computational material mechanics' from < micro-scale models (*Molecular Dynamics-treated and test results-supported from statically and cyclically loaded 10 μ m thick pillars for instance*) via micro-scale to bridge with the necessary properties (*hopefully statistically based*) to the classical macro-scale models in structural design.

Multiscale materials modelling could grow and become a significant tool for understanding complex material micro-damage processes for many homogeneous isotropic materials, a benefit for macroscale investigations.

The conclusions of Mlikota are:

- The CRSS is the resistance for the dislocations to move through the crystal. It is governed by the present strengthening mechanisms in the crystal. The CRSS is - according to critical stress strength - a micro-shear strength.
- The fatigue crack growth modeling procedure in the High Cycle Fatigue regime should include the following steps: Micro-crack nucleation within a grain \rightarrow Coalescence of already existing flaws and/or arrest at grain boundaries \rightarrow Short crack or Stage I growth \rightarrow Transition from Stage I to Long crack or Stage II growth

- The discovered relation between endurance limit and the CRSS allows the virtual selection of those types of materials, which are more fatigue resistant! The physically-based MMM approach represents a breakthrough in the field of fatigue research
- The higher the CRSS magnitude of the metal of interest, the higher the loading stress level σ will be necessary to accomplish the transition from infinite to finite life
- The multiscale fatigue simulation approach is capable of properly taking into account the mean stress $\sigma_m = \max \sigma \cdot (1+R) / 2$ with the stress ratio $R = \min \sigma / \max \sigma$ and capturing the stress concentration factor K_t , which are influencing factors when designing structural components.
- Experimental tests demonstrate, that there is a drop in resistance to fatigue fracture with the increase of the grain size.

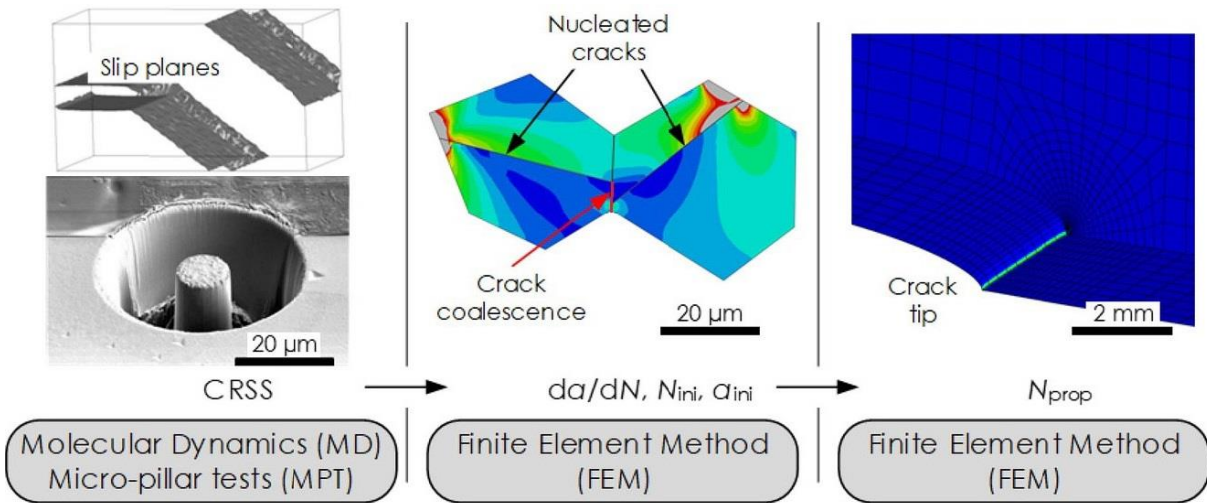


Fig.17-4: Full modelling approach. CRSS critical resolved shear stress, da/dN crack growth rate, N_{in} number of stress cycles until *short-crack* initiation, a_{ini} initiation short-crack length, N_{pro} number of stress cycles until short-crack propagation.

[Mlikota M. & Schmauder S. (2018), 'On the critical resolved shear stress and its importance in the fatigue performance of steels and other metals with different crystallographic structures', Metals 8(11), 883]

LL:

* *There is a hope for some ductile materials in future to estimate the endurance limits of various metallic materials in the Ultra HCF regime just by knowing their CRSS values !*

Available CDM models seem to be neither to be clear-defined nor classified to be used for Design Verification (DV). A DV-procedure is searched

* *A grain is usually polycrystalline with crystal planes in various spatial orientations. Hence, a metallic material can be only termed homogeneous and isotropic if these orientations are randomly distributed in order to become quasi-homogeneous. (By the way, this is the same for an isotropic short fiber-reinforced polymeric material. Otherwise, the so-called orientation tensor has to take care of the non-isotropy).*

* *For the analysis the Mises SFC was employed in order to localize the peaks of shear banding (yielding) of the investigated steel material*

$$\sigma_{eq}^{Mises} = \sqrt{3 \cdot J_2} \quad \text{with} \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2 = f(\tau), \quad \tau_{oct} = \sqrt{J_2 / 3}$$

* *Clearly to be defined is the quantification of the D-portions for ductile and brittle behavior with a maximum value of total D = 100%:*

- *static case: the achieved micro-damage value at a distinct (equivalent) stress level*

- *cyclic case: the cycle-associated micro-damage portions with its derivation formula.*

17.4 'Meso'–Modelling of the Example UD material

Fig.17-5 gives a look at the present multi-scale modelling performed with Fiber-Reinforced-Polymers (FRP). Two scales are linked together, the micro-scale with the macro-scale by a meso-model. What is meso? Meso is no scale, per definitionem!

- * Micro-scale $> \mu m$, macro-scale $> mm$.
- * The author experienced (1999) in a BMFT R&D discussion round on three MaTech Competence centers of institutes working from the polymer-scale to the structural macro-scale - after one day - that the term meso-scale is used in polymer mechanics by the research colleagues at the nano-level. This level is one thousand times smaller than the solid mechanics people apply meso.
- * A further classification is available for porous materials, according to pore size: 'microporous' pores < 2 nm, 'mesoporous' pores between 2 nm and 50 nm, 'macro-porous' pores > 50 nm. [*International Union of Pure and Applied Chemistry*].

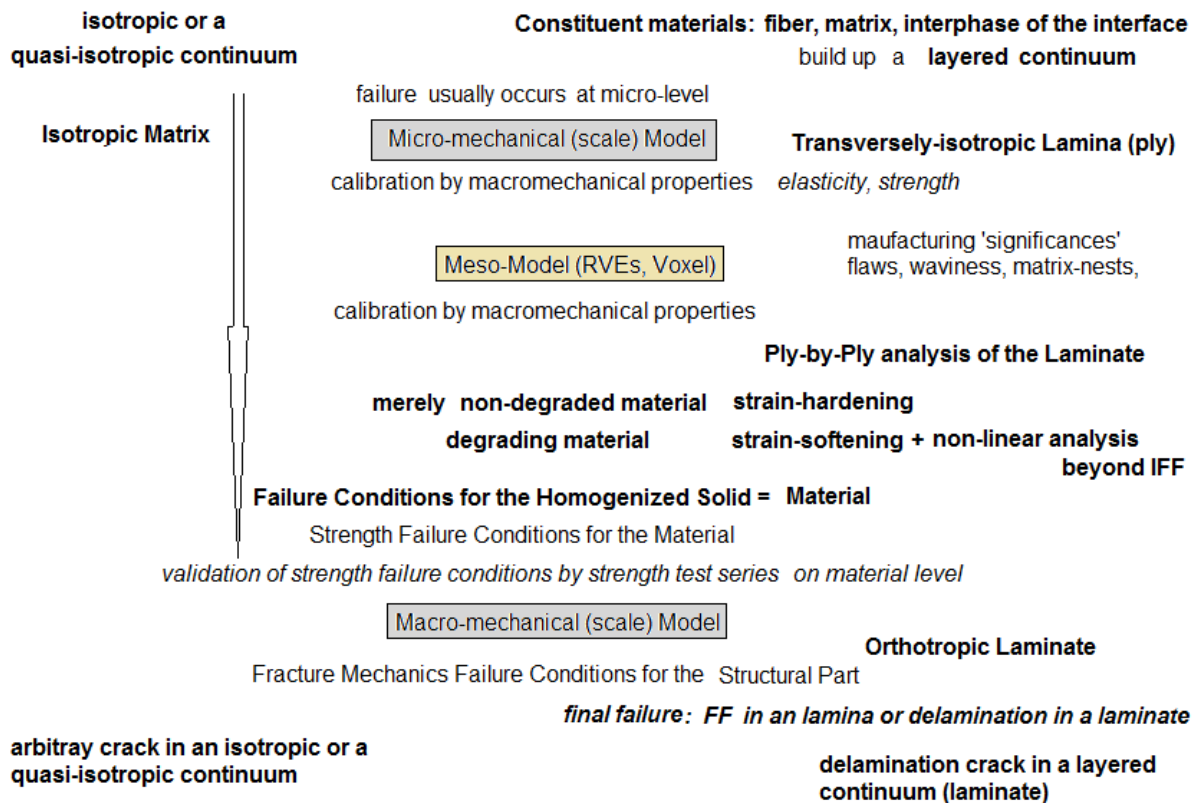


Fig.17-5: Multi-scale modelling, example FRP, brittle. 2 scales. RVE: Representative Volume Element, Voxel: volumetric pixel

LL: The term meso is a task-linked chosen size level. Apply the term meso-model, not meso-scale, and define it. In structural engineering meso is used at about 0.1 mm.

17.5 Note on Micro-mechanical Formulas (mixture rules) for Example UD lamina (ply)

Aim: Guideline how to use micro-mechanical models and properties with giving some warning.

Mixture rules are employed in many technical disciplines (polymer and mineral composites like concrete). Exemplarily, here at the so-called micro-mechanical formulas of UD-materials will be looked at, only.

Creep investigations and pressure-related effects on the matrix and in consequence on the UD material of composite materials i.e. usually require a micro-mechanical input.

Examples of the author, a centrifuge and a WWFE Test Case: The non-creeping constituent fiber is to separate from the creeping/relaxing constituent matrix. In order to capture these features the use of 'micro-mechanical mixture rules' in structural engineering is common practice. It requires properties of the constituents and the so-called mixture rule, how these constituent properties are linked, to be able to predict properties of the envisaged ('smeared') material on the macro-scale. Not all micro-mechanical properties applied can be measured. A solution will be obtained by setting up mixture rules and calibrate them via macro-mechanical test results on the lamina macro-level. This makes an inverse parameter-identification necessary.

Hence, the application of a micro-mechanical formula underlies the constraint that the given micro-mechanical properties can be only used together with the formulas they are based on. Otherwise the results might be pretty wrong. For example within the WWFE, Test Case 1, the organizer QinetiQ just provided micro-mechanical material properties but not the associated micro-mechanical formula. Therefore, the author had to apply micro-mechanical UD formulas from [VDI 2014, sheet 3] and found a discrepancy of a factor 2 for the data to be predicted! This is not acceptable for the WWFE-task model validation.

LL: Micro-mechanical properties can be used only together with the formulas they have been determined with! ← Warning!!

18 Some Lessons Learned from Testing and from Evaluation of Test Results

Aim: Forwarding lessons learned.

In structural design one basically faces 3 types of testing:

- Structural Testing (destructive, non-destructive)
- Materials Testing (destructive, non-destructive) and
- Non-Destructive Testing of structure and material (NDT, NDI, NDE).

Other tasks here are: Failure detection, localization, size + shape, Failure assessment (risk-based).

All structural tests to be performed aim to uncover a deficiency: Workmanship, design mistake, oversight of a failure mode, tightness, shock resistance etc.

Fig.18-1 presents the test strategy of the MIL handbook 17, a forerunner guideline for the development of composite structures which are more challenging than developing isotropic structures.

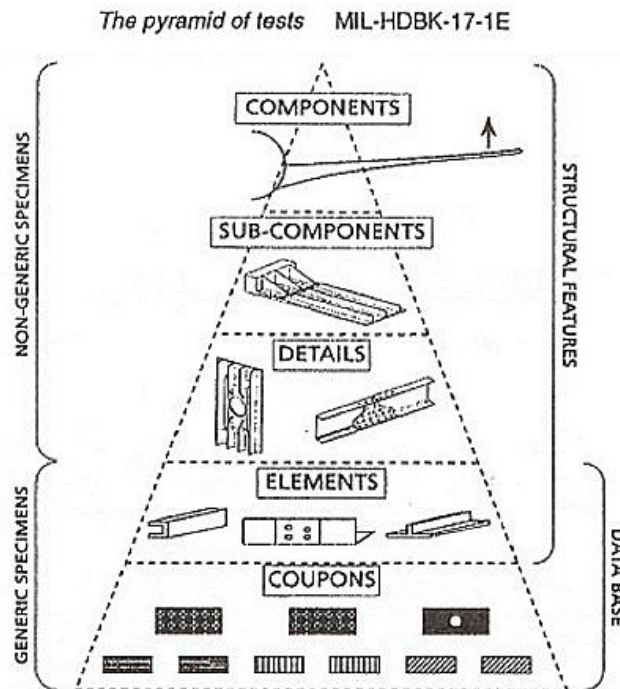


Fig18-1: Test strategy of MIL-HDBK 17 (original edition about 1970). MIL-HDBK-17/1F (VOL. 1 OF 5), DEPARTMENT OF DEFENSE HANDBOOK: COMPOSITE MATERIALS HANDBOOK - POLYMER MATRIX COMPOSITES GUIDELINES FOR CHARACTERIZATION

In this Chapter some personal experience is presented, beginning with structural testing.

18.1 Structural Testing primarily based on the Ariane launcher development

At first, a Test Agreement is to provide. It consists of test rig, test specification, test specimen and test data evaluation method and the Test Procedure. Therefore, one can only speak about 'exact' test results in the frame of the obtained test quality.

Fig.18-2 presents the so-called sub-structuring (*affecting shares between the participating Ariane partners*) an example for violating mechanics: MAN was not permitted to include the neighboring structural part despite of the fact that it was also a MAN contract part. We could not

implement the FE model of this neighboring part in order to optimally represent the real boundary stiffness conditions in the model of the ‘studied structural part’ but had to implement the given boundary conditions of the contract. This caused a wrong behavior of the ‘studied structure’ and was a real mess regarding the evaluation of the test results and comparison with analysis results. The first test article has been allegedly strengthened, which was senseless.

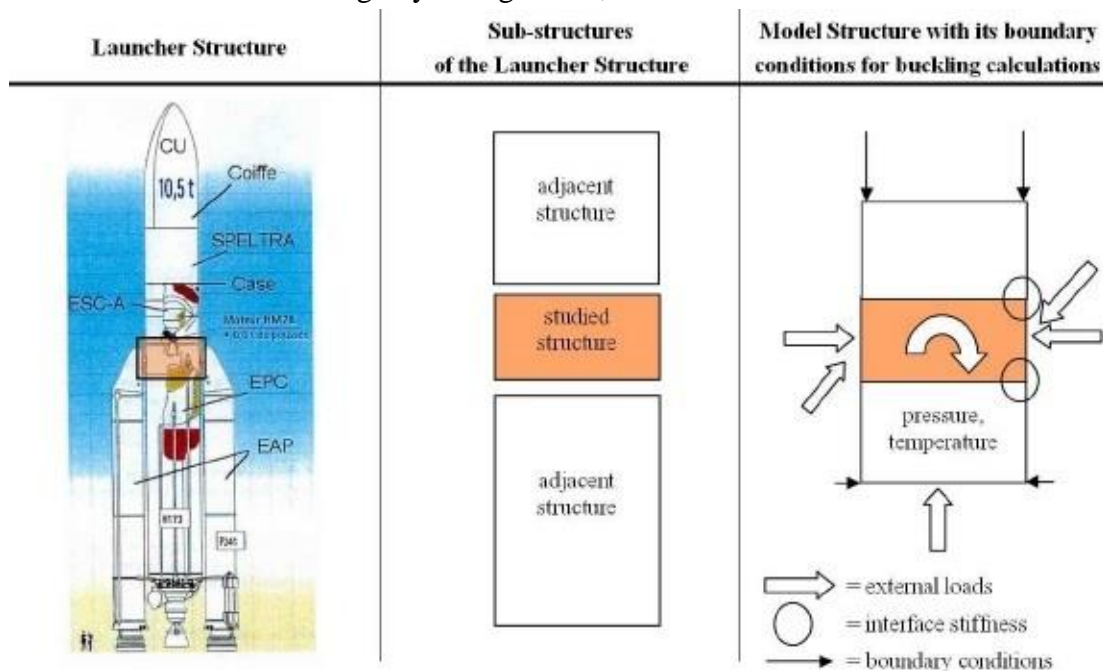


Fig.18-2: Sub-structuring of the Ariane 5 launcher, Front Skirt test

LL

- * Test article analysis is mandatory to interpret the test results and simulation-based improve the design. Only well-understood experiments can verify the design assumptions made!
- * Splitting of a large structure (Ariane experience) is dangerous: The first buckling mode can appear on an adjacent structure and not on the studied one
- * Mandatory for a realistic qualification of a sub-structure is a realistic set of cross-section loadings and pressure loading with an accurate structural designing of the interface stiffness of the adjacent structural parts. If the interface is too stiff in the test assembly this will attract loading and lead to a non- realistic failure site (experience from Ariane 5 tests)
- * Not all critical locations of a structural component can be tested, because an ‘over-testing’ of some parts may happen to be. ‘Verification By-Analysis-Only’ is to be considered if the structure is too big or if the test model shall e.g. be applied later as flight model
- * Put strain gauges there where a clear stress situation is in order to avoid useless discussions about the interpretation. Check locally by strain measurements and then rely globally on FEA-test result comparison
- * Specific design requirements drive testing
- * Requiring different so-called system margins MoS_{sys} (suffered nonsense in a Ariane Technical Specification) for the various structural parts, then not all critical locations can be tested without overloading other integrated parts. Components of such a structural assembly cannot be verified by a qualification test, because system margins cannot be used locally like a ‘fitting factor’. They should have been considered directly in the Ariane 5 as a usual design FoS, applying $j_{sys} = (MoS_{sys} + 1) \cdot j$. Otherwise, the design process is obscured and is prevented from applying the most economic measure in order to take risk out of the structure
- * Requirement to put a design FoS j on a design temperature violates physics and structure behavior

** So-called test correction factors are applied to adjust the design verifications by accurately evaluated structural test results linked to the test article analysis results.*

18.2 Material Testing primarily based on the *World-Wide-Failure-Exercises-I and -II*

The author succeeded with test-validation of 3D-strength criteria models for isotropic concrete, transversely-isotropic UD-material, orthotropic ceramic (fabrics) with visualization of the derived 3D failure surfaces if reliable test data sets were given.

This was only partly given in the *the World-Wide-Failure-Exercises-I, concerning 2D-mapping, and -II, concerning 3D-mapping of UD materials*. The author's WWFE-I and -II contributions had to be based on an intensive assessment of provided test results. In this sub-chapter the Lessons Learned during the examination of several WWFE-Test Cases (TC) will be collected.

Validation of the lamina-material SFCs models can be only achieved by 2D- together with 3D-lamina test results. Since SFC-model validation is focused just lamina-TCs are now investigated in detail. The normal user is just interested to well map his course of failure test data by a UD-SFC and not on the laminate analysis tools.

The laminates test cases serve for the verification of the laminate design. There the full WWFE failure theory is required. This makes a comparison between the contributions very challenging because different FE codes were applied by the contributing competing institutes. These better tools further had to be equally compared to the retired author's tools. He could just use his handmade non-linear CLT-code upgraded by experience and using his sensibleness for the problem and the delivered input.

LL, more general ones

- * Measurement data is the result of a Test Agreement (norm or standard), that serves the desire to make a comparability of different test procedure results possible. Hence, there are no exact property values. Material properties are the result of the material model applied inclusively mapping process.*
- * Stresses, strength, strains, elasticity properties cannot be directly measured*
- * Check of assumptions is necessary before designing (example: WWFE on UD-material). Pore-free material, specimen surfaces polished, well-sealed, fiber volume is constant, tube specimens show no warping and do not bulge, perfect bonding, no layer waviness, edge effects do not exist*
- * Sometimes one must live with a substitute test situation in order to get some approximate properties (Example: UD-Tension/Compression-Torsion test device → Arcan test device)*
- * Before thinking about test data evaluation the associated underlying micro-damage processes must be sorted out in order to get a better understanding of failure*
- * Test specimens shall be manufactured like the structure ('as-built')*
- * Comparisons between theoretical predictions and test data help to identify the major discrepancies, limitations, and areas which require further theoretical and experimental work. There is always a lot to be done and following Moslik Saadi "All is difficult prior to becoming simple"! This begins with the provision of appropriate test specimens for the various material families being extreme ductile or brittle and ends with appropriate test procedures and an appropriate test data evaluation*
- * Considering FE-results: We must more and more 3D-design! However the situation of properties, especially for composites is: „3D-property data test sets are seldom sufficiently available“.*

Of high interest for future scientists and engineers might be the following assessment results of the provided properties during the author's many WWFE-designated years. They are results which stem

from a very careful and effortful test data evaluation of about one man year. Otherwise, a successful WWFE-contribution could not have been made possible.

Thereby, some essential WWFE TestCase-examples for lamina-input shortcomings were found:

* WWFE-I, TC1: the provided strengths have been changed from Part A to B and two test points are doubtful regarding own test results (*Reason is known: non-accurate raw test data evaluation of the test engineer at DLR Stuttgart. Organizers did not question the test data but required mapping of the false ones!*).

* WWFE-I TC2: the author informed the organizers that apples and oranges have been put here together in a diagram. One cannot fill into the same diagram 90°-wound tube test specimen data together with 0°-wound tube data. The 0°-stresses have to be transformed in the 2D-plane due to the fact that shearing under torsion loading turns the fiber direction (see *Fig.17-3*) and the lamina coordinate system CoS is not anymore identical with the structure coordinate system of the tube. In order to also use these test data the author exemplarily transformed magenta-colored **two fracture test points** by the occurring twisting angle γ using a non-linear CLT-analysis. Then he could achieve a good mapping showing, that the two transformed fracture points accurately lie in the lamina CoS on the 90°-curve.

* WWFE-II, TC3: the same mistake happened again! However, here the much more complicated 3D-stress situation was to face, so that the 3D-transformation of the 0°-data set could be simply performed.

* WWFE-II, TC2 an average stress-strain curve should have been provided because otherwise no realistic treatment is possible. Therefore the Part A results could be only inaccurate. From the Part B information the author could derive an average curve and then all 3 TC test data courses could be mapped and the mutual check points in the fully connected TC2-TC3-TC4 matched. Incomprehensively, there was no response of the organizers to the author's idea, which made 3 TCs to successful test cases.

* Viewing the final papers of the WWFE-organizers "A comparison of the predictive capabilities of current failure theories for composite (UD-composed) laminates, judged against experimental evidence" and "Maturity of 3D failure criteria for fiber-reinforced composites, comparison between theories and experiments", there is not any doubt to find concerning the quality of the only available, provided test data sets. **One third of the provided TC test data was at least questionable till not applicable for model validation.**

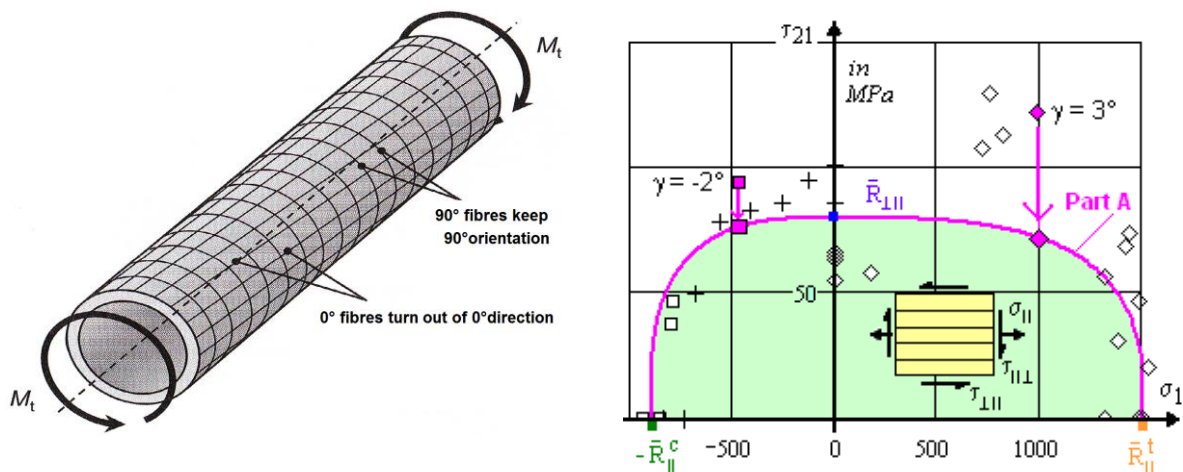


Fig.18-3, $\tau_{21}^{fr}(\sigma_2)$ basic cross-section of the fracture failure body: (right) WWFE-I, TC2, UD lamina, CFRP, T300/BSL914C Ep ; (left) Tube test specimen picture: [Courtesy IKV Aachen] The normal user is just interested to well map his course of failure test data by a SFC

* Test results can be far away from the reality like an inaccurate theoretical model.

* Theory creates a model of the reality, one experiment shows one realization of the reality.

19 2D-Laminate Design: Direct Determination of Tsai's 'Omni principal FPF strain failure envelopes

Aim: Replacing the ply-by-ply proof of multiple-ply laminates by a much simpler method

Steve Tsai's idea was to by-pass the effortful ply-by-ply analysis of multiple-ply laminates by using a so-called 'Omni-(principal FPF strain) failure envelope'. This envelope surrounds an intact Non-FPF area whereby FirstPlyFailure (FPF) includes Fiber Failure FF and Inter-Fiber-Failure (IFF).

Such an 'Omni failure envelope' is to determine for each composite material, applying a FPF-Strength Failure Criterion (SFC), and will capture all possible laminate stacks. Naturally, the used SFC significantly determines the shape of the envelope, see *Fig.19-1*.

Dimensioning is performed by showing that the design loading-caused principal strains are lying within the Non-FPF area. The idea can serve as a very practical Pre-design tool.

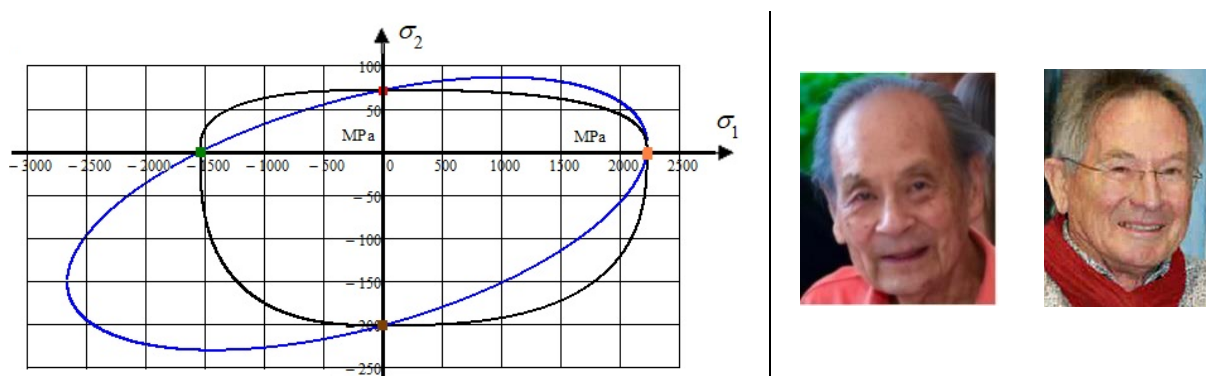


Fig.19-1: Cross-section $\sigma_2(\sigma_1)$ of the failure body, Tsai-Wu versus Cuntze

19.1 Tsai's indirect Determination of the 2D 'Omni envelope'

Fig.19-2 displays different 'butterflies' (name, how the author Cuntze termed the bundle of i FPF-curves), derived using the SFCs of Tsai-Wu and Cuntze). These numerical results of the FPF-linked

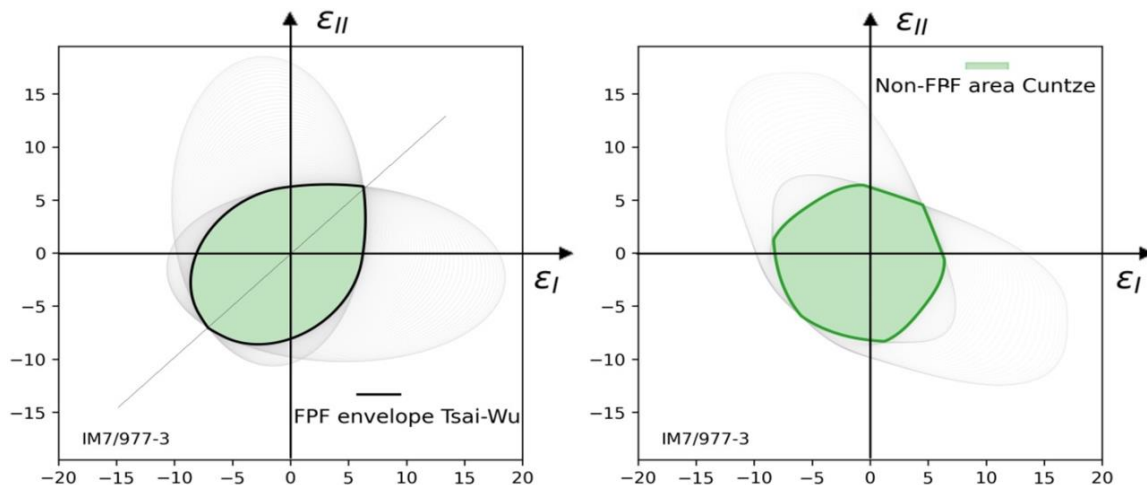


Fig.19-2, bundle of all FPF envelopes = 'butterflies': All ply FPF-envelopes enclosing a non-FPF failure area; $0^\circ < \alpha < 90^\circ$ (91 ply angles). Principal strain in %, suffix FPF is skipped. CFRP IM7/977-3. In all pictures: (left) Tsai-Wu with $\mu_{\perp\parallel} = 0$, $F_{12} = -0.5$ and (right) Cuntze with $\mu_{\perp\parallel} = 0.2$, $m = 2.7$

principal strain curves clearly depict the significant effect of the chosen SFC, see above figure. The different lateral properties determine the shape (wing edge) of the obtained symmetric ‘butterfly’ with its single, grey-marked principal strain curves provided by E. Kappel.

19.2 Cuntze’s Determination of the 2D ‘Omni Envelope’

The derivation of such an ‘Omni failure envelope’ is pretty effortful and no direct formulation could be found in the past. Recently, this bottleneck could be by-passed by an idea of the author, who examined various horizontal cross-sections $\tau_{21} = \text{constant}$ of the UD-FPF fracture body in *Fig.19-3* below. He found that $\tau_{21}=0$ delivers the smallest Non-FPF area. ▶ Pre-Dimensioning can now be performed by showing that the design loading-caused principal strains are located within the Non-FPF area, a simpler pre-design of arbitrary laminates is possible.

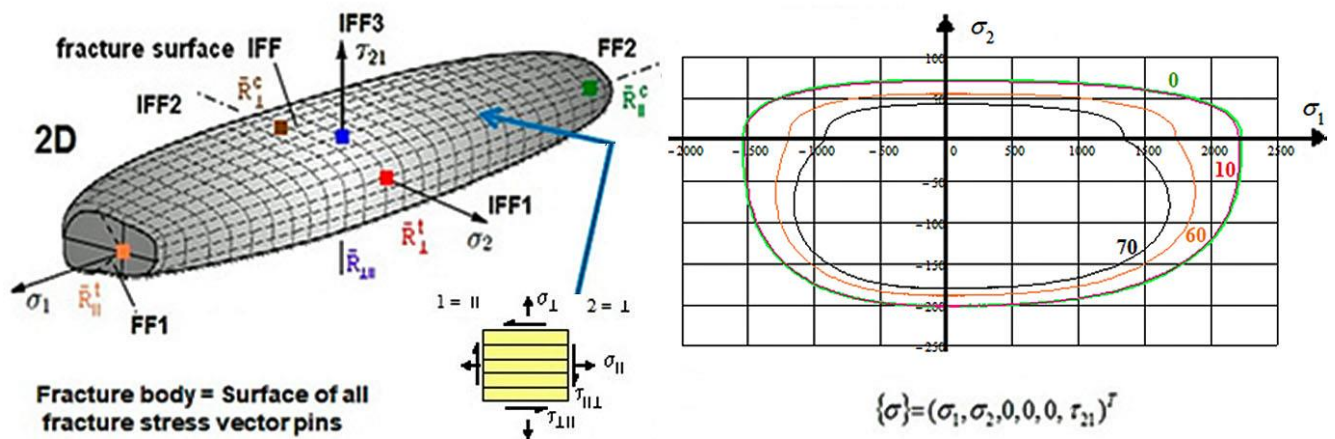


Fig.19-3: (left) 3D UD Failure body. (right) FPF-envelopes for 3 planes $\tau_{21} = \text{const}$. CFRP IM7/977-3

Fig.19-4 (left) presents the resulting Omni principal strain FPF curves $\varepsilon_{II}(\varepsilon_I)$ with a not unambiguously solution $\varepsilon_{II}(\varepsilon_I)$ for each parameter level $\tau_{21} = \text{const}$. → The failure curve $\sigma_2(\sigma_1, \tau_{21} = 0)$ describes the ‘Omni envelope’.

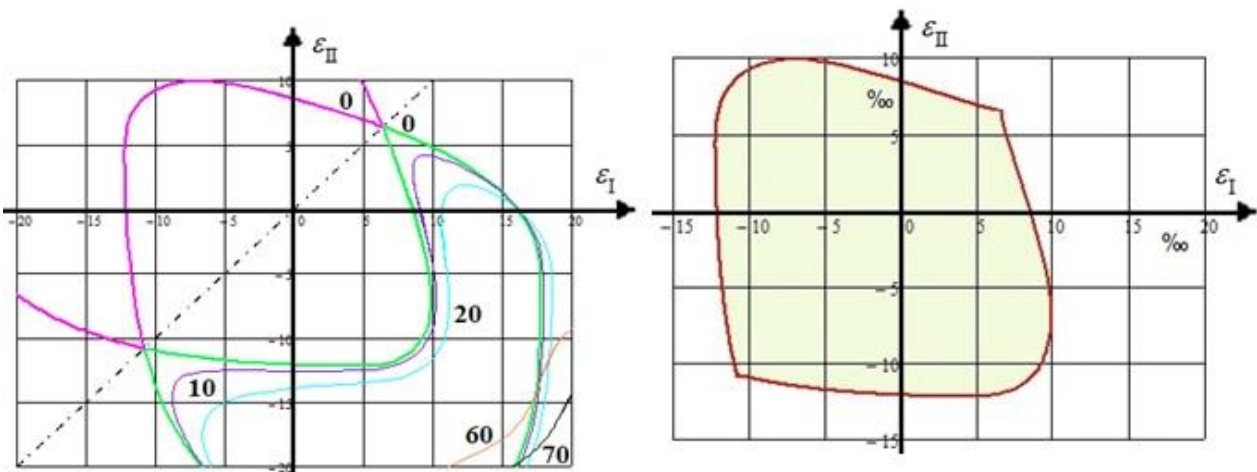


Fig.19-4: Mirrored envelope of the Non-FPF area (Cuntze procedure), CFRP IM7/977-3

Originally, the ‘second’ solution-linked additional outer curve parts were excluded in the graph and the right figure eventually shows the ‘cleaned-up’ envelope, representing the limit $Eff = 100\%$, enveloping the Non-FPF area. The cleaned-up graph is identical to the Non-FPF area obtained by the standard Tsai ‘butterfly’-determination procedure.

Domains of the envelope could be dedicated to the locally faced failure mode types FF and IFF.

In a novel investigation, detailed in *Table 19-1*, Cuntze could give a complete look of the different envelopes in *Fig.19-4* (left). Depicted are the ‘butterfly’ wings (outside) and internally the green shadowed Non-FPF area. For optical comparison reasons E. Kappel ‘traditionally’ provided the ‘butterfly’ procedure plots for *Fig.19-4* (right) and *Fig.19-5*.

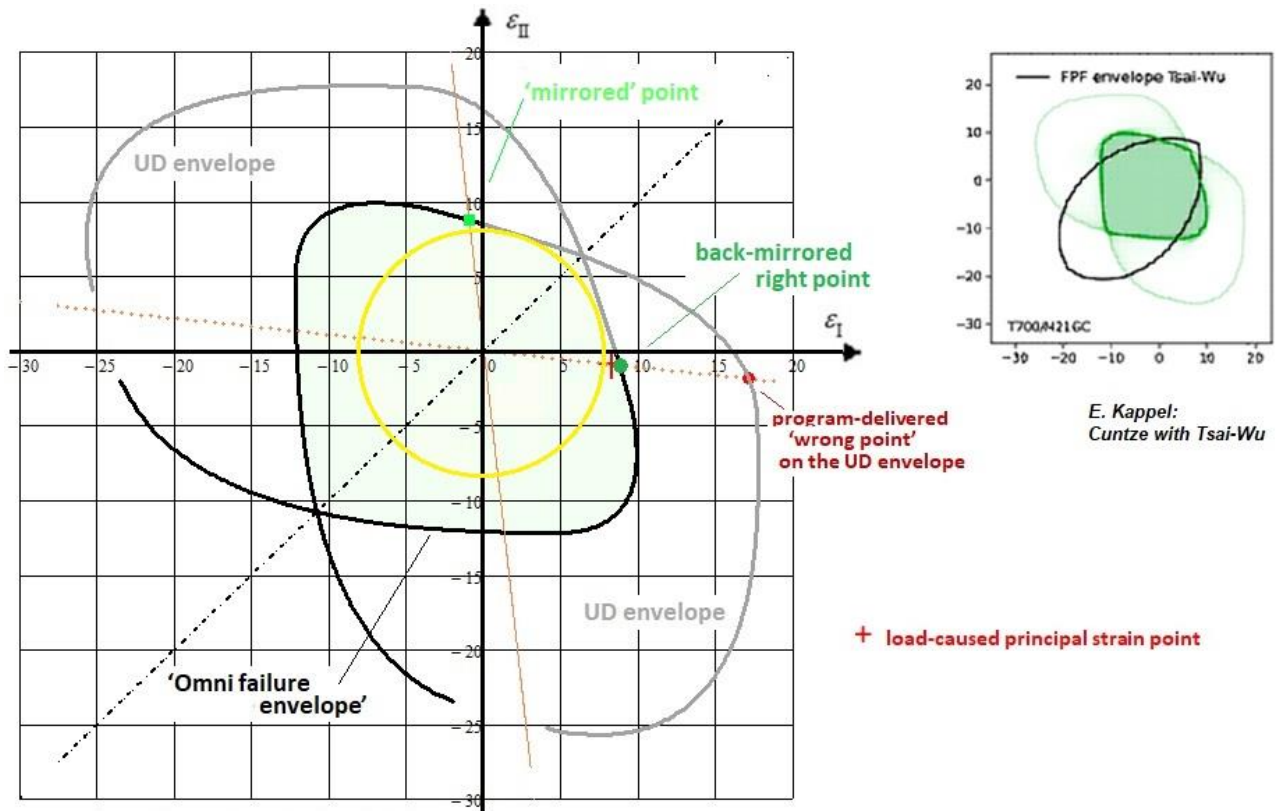


Fig.19-5: (left) Various envelopes of the Non-FPF area (Cuntze procedure following Principal Strain Procedure Cuntze in Table 19-1)..(right) ‘Butterfly’ and Non-FPF area applying the SFCs of Tsai-Wu and Cuntze

19.3 Pre-design Example using the ‘Omni Non-FPF area’ and Determination of Reserve Factor

Of highest interest is the reserve factor which must be smaller for a simplified design method than obtained by the classical ‘Ply-by-ply procedure’, thus remaining on the Safe Side. Laminate Design Verification is traditionally performed by above ‘ply-by-ply’ analysis, assessing the obtained ply (lamina) stresses $\{\sigma\}$ in the critical location of the most critical plies. Now, a simpler more global assessment is possible (*Table 19-2*) by using the in-plane principal strains of the laminate, strains

Table19-2: Procedure of checking a probably critical design stress state

A Non-FPF area within an ‘Omni failure envelope’ is given for the chosen laminate material
 ➤ FEA delivers the maximum state of the 3 strains of the laminate stack

- Transformation into the 2 principal strains as coordinates of the Non-FPF area
- Check, whether the strain point $(\varepsilon_I, \varepsilon_{II})$ lies within the envelope or Non-FPF area
- Determine material reserve factor $f_{RF} = \text{vector length ratio of failure strain/design strain}$.

which represent the loading. Such principal strains are a standard output of modern FE software. They are mathematical and not material symmetry-linked quantities.

Remember, please: The execution of the Design Check runs under the Presumption
“Linear Analysis, proportional stressing $\sigma \sim \varepsilon$ is permitted”.

Table 19-1: Procedures, how to obtain the material reserve factor f_{RF}

SFC Cuntze: Failure Function $F(\{\sigma\}, \{\bar{R}\}, \mu \text{ directly}) = 1$

$$Eff_{FPF} = [(Eff^{//\sigma})^m + (Eff^{//\tau})^m + (Eff^{\perp\sigma})^m + (Eff^{\perp\tau})^m + (0)^m]^{m^{-1}} = 1, \quad m = 2.7$$

Input

$$\{\sigma\} = (\sigma_1, \sigma_2, \sigma_3, \tau_{23}, \tau_{31}, \tau_{21})^T \rightarrow (\sigma_1 = 900, \sigma_2 = 20, 0, 0, 0, \tau_{21} = 25)^T \text{ MPa,}$$

$$\{\bar{R}\} = (\bar{R}'_{//}, \bar{R}^c_{//}, \bar{R}'_{\perp}, \bar{R}^c_{\perp}, \bar{R}_{\perp//})^T \rightarrow (2230, 537, 71, 202, 78)^T \text{ MPa, } \mu_{\perp//} = 0.2.$$

$\{\sigma\}$ -loading-caused ply strains and loading-representing principal strains

$$\varepsilon_1 = s_{11} \cdot \sigma_1 + s_{21} \cdot \sigma_2, \quad \varepsilon_2 = s_{21} \cdot \sigma_1 + s_{22} \cdot \sigma_2, \quad \gamma_{21} = s_{66} \cdot \tau_{21}$$

$$\varepsilon_I = 0.5 \cdot \left[(\varepsilon_1 + \varepsilon_2) + \sqrt{(\varepsilon_1 - \varepsilon_2)^2 + \gamma_{21}^2} \right], \quad \varepsilon_{II} = 0.5 \cdot \left[(\varepsilon_1 + \varepsilon_2) - \sqrt{(\varepsilon_1 - \varepsilon_2)^2 + \gamma_{21}^2} \right]$$

$$\varepsilon_I = 0.0083 \equiv \text{UD} \varepsilon \text{I}, \quad \varepsilon_{II} = f_{\varepsilon} \cdot \varepsilon_I = 0.0009, \quad f_{\varepsilon} = \varepsilon_{II} / \varepsilon_I = -0.109.$$

*Stress Procedure Cuntze: Lamina task, solved by ply-by-ply failure analysis

$Eff^m = [(Eff^{//\sigma})^m + (Eff^{//\tau})^m + (Eff^{\perp\sigma})^m + (Eff^{\perp\tau})^m + (Eff^{\perp//})^m]$ with the mode portions inserted,

$$Eff = \left[\left(\frac{(\sigma_1 + |\sigma_1|)}{2 \cdot \bar{R}'_{//}} \right)^m + \left(\frac{(-\sigma_1 + |\sigma_1|)}{2 \cdot \bar{R}^c_{//}} \right)^m + \left(\frac{(\sigma_2 + |\sigma_2|)}{2 \cdot \bar{R}'_{\perp}} \right)^m + \left(\frac{(-\sigma_2 + |\sigma_2|)}{2 \cdot \bar{R}^c_{\perp}} \right)^m + \left(\frac{|\tau_{21}|}{\bar{R}_{\perp//} + 0.5 \cdot \mu_{\perp//} \cdot (-\sigma_2 + |\sigma_2|)} \right)^m \right]^{1/m}$$

$$\rightarrow Eff = 0.513 \Rightarrow \text{material reserve factor } f_{RF} = 1 / Eff = 1.95.$$

*Principal Strain Procedure Cuntze: classical laminate task, solved by a laminate failure analysis; $\tau_{12} = 0$

Due to $\varepsilon_I = \varepsilon_I, \varepsilon_{II} = \varepsilon_{II}$ for the 2 failure determining stresses follows, $\gamma_{21} = 0$

$$\sigma_1 = (s_{21} \cdot \varepsilon_2 - s_{22} \cdot \varepsilon_1) / (s_{21}^2 - s_{11} \cdot s_{22}) \quad \text{and} \quad \sigma_2 = (\varepsilon_2 - s_{21} \cdot \sigma_1) / s_{22} \quad \text{which is to insert into}$$

the FPF-criterion-based 'Omni principal strain failure envelope' formula

$$\left(\frac{(\sigma_1 + |\sigma_1|)}{2\bar{R}'_{//}} \right)^m + \left(\frac{(-\sigma_1 + |\sigma_1|)}{2\bar{R}^c_{//}} \right)^m + \left(\frac{(\sigma_2 + |\sigma_2|)}{2\bar{R}'_{\perp}} \right)^m + \left(\frac{(-\sigma_2 + |\sigma_2|)}{2\bar{R}^c_{\perp}} \right)^m + (0)^m = 1 = 100\% .$$

With the chosen SFC the \rightarrow Non-FPF area $\varepsilon(\sigma_{FPF})$ is to derive .

On the strain beam $f_{\varepsilon} = \frac{\varepsilon_{II}}{\varepsilon_I} = \frac{\varepsilon_{II,FPF}}{\varepsilon_{I,FPF}}$ it will be finally obtained

$$f_{RF} = \sqrt{(\varepsilon_{I,FPF}^2 + \varepsilon_{II,FPF}^2)} / \sqrt{(\varepsilon_I^2 + \varepsilon_{II}^2)} = \varepsilon_{I,FPF} / \varepsilon_I = 1.06 .$$

Cuntze's direct determination of the 'Omni failure envelope' enables to determine the reserve factor straightforward instead of using the *Non-FPF smaller* internal circle in Fig.19-5, how it was usually performed up to now, see [Cun 24].

However, there was a computational problem: Mathcad unfortunately delivers a principal failure strain value ε_{FPF} outside of the Non-FPF area as result of its solution process. The other solution seems to be received, if a shear strength is involved. This **wrong** point value can be localized on the UD 'butterfly wing' edge in Fig 19-4 and this enabled to successfully use the symmetry of the envelope as it is executed in Fig-19-5.

Now, Design Verification can be performed as described below:

$$f_{\varepsilon} = \frac{\varepsilon_{II}}{\varepsilon_I} \quad f_{\varepsilon} = -0.109 \quad UD\varepsilon_I = 0.0083$$

Vorgabe $\varepsilon_{IFPF} := 0.01 \quad \sigma_1 := 100 \quad \sigma_2 := 10$

$$\varepsilon_{IFPF} = s_{11} \cdot \sigma_1 + s_{21} \cdot \sigma_2 \quad f_{\varepsilon} \cdot \varepsilon_{IFPF} = s_{21} \cdot \sigma_1 + s_{22} \cdot \sigma_2$$

$$\left(\frac{\sigma_1 + |\sigma_1|}{2R_{1t}} \right)^{mint} + \left(\frac{\sigma_2 + |\sigma_2|}{2R_{2t}} \right)^{mint} + \left(\frac{-\sigma_1 + |\sigma_1|}{2R_{1c}} \right)^{mint} + \left(\frac{-\sigma_2 + |\sigma_2|}{2R_{2c}} \right)^{mint} = 1$$

$$A\varepsilon := \text{Suchen}(\varepsilon_{IFPF}, \sigma_1, \sigma_2) \quad \varepsilon_{IFPF} := A\varepsilon_0 \quad \varepsilon_{IFPF} = 0.01712$$

$$f_{\varepsilon} = -0.109 \quad f_{RF\varepsilon} := \frac{\varepsilon_{IFPF}}{UD\varepsilon_I} \quad f_{RF\varepsilon} = 2.07$$

This result of the Mathcad program leads to a value which belongs to another solution brunch (see the figure). Using the plot's symmetry the real value can be found after the replacement of f_{ε} by $f_{\varepsilon r} = 1/f_{\varepsilon}$

$$f_{\varepsilon r} := \frac{1}{f_{\varepsilon}} \quad f_{\varepsilon r} = -9.214 \quad f_{RF\varepsilon r} := \frac{\varepsilon_{IFPFr}}{UD\varepsilon_I} \quad f_{RF\varepsilon r} = 1.06$$

$$< f_{RF\sigma} = 1.95$$

Fig.19-6 Successful computation of f_{RF} after utilizing the plot's symmetry (code Mathcad 15). $\varepsilon_I \equiv UD\varepsilon_I$

LL:

- * The method is more or less a linear method.
- * The investigation of various cross-sections $\tau_{21}=\text{constant}$ proved, that $\tau_{21}=0$ delivers the smallest Non-FPF area, thus making a simpler pre-design of arbitrary laminates possible
- * Basic result:
The principal strain approach delivers the required smaller reserve factor compared to the conventional ply-by-ply stress-based procedure. → The approach is 'on the safe side' !

Note, once again please:

Tsai's 'Omni principal strain envelope' principally surrounds a Non-FPF or even a Non-LPF area.

- *FPF is required if the design requirement asks to fulfill a First-Ply-Failure in the critical locations of the plies of the laminate.
- *LPF, if to apply, is required to fulfill a Last-Ply-Failure limit. However, this usually involves a non-linear analysis up to the ultimate failure load of the structural part.

In order to cope with the reserve factor definition these shall be sketched again below:

About 'linear' FPF: stress-defined $f_{RF} = \frac{\text{Strength Design Allowable } R}{\text{Stress at } j \cdot \text{Design Limit Load}} > 1$

20 Note on Criticality of Fiber Micro-Fragments and Dusts of CFR-Plastic/CFR-Concrete

Matter of my heart:

Supporting the application of sustainable carbon concrete with low-risk PAN-CFs in Production and my concern regarding Recycling.

Carbon Fibers (CFs) usually are produced using the precursors Polyacrylonitrile (PAN) and Pitch. Problem and question: Machined Pitch CFs generated many toxic split-up fiber fragments. What about the PAN-based CFs? They can be classified into the types: intermediate-modulus (IM), high-modulus (HM) and ultrahigh-modulus (UHM), whereby UHM-CFs seem to show some and the lower modulus Standard PAN no hazard. These facts ask for an investigation of the UHM-CF with the objective to finally sort out that the use of the less ‘risky’ Standard PAN CF causes no threat.

Inhaled particles with its size, geometric shape and contaminants adhering to the surface are relevant for a health effect. Of course, targeted workplace prescriptions always have to counteract the occurrence of excessive stress on the lungs from inhaling too large amounts. Respirable bio-persistent particles accumulate in the alveoli of the lungs. These so-called ‘WHO fibres’ pierce the macrophages in the lungs and can migrate into the abdomen and pleural tissues and cause cancer.

CF application in Construction

As structural engineer, who has founded and led two working groups in the carbon concrete sector for 10 years: “*It is my deep wish to use more fatigue-resistant [VDI2014] PAN-CF in the construction industry in order to increase the life of bridges and to save concrete, a composite material, which has a negative CO₂ footprint due to the necessary clinker (cement constituent) production.*”

The next figure displays a CFRP application by a fiber grid (*mat*) as a slack reinforcement (*no pretension*) of a bridge.

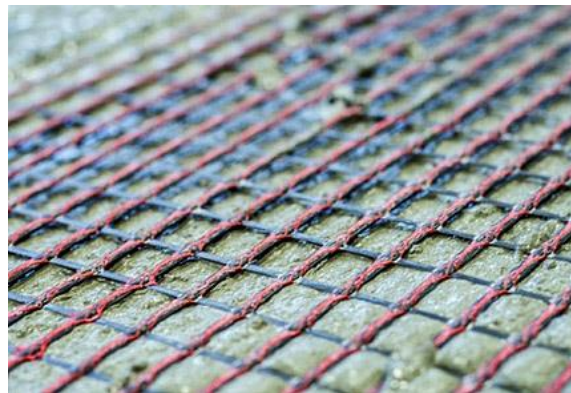


Fig.20-1: Bridge Wurschen, 2022: (left) Superstructure made exclusively of carbon concrete, shell construction. (right) Textile FRP mats in the super-structure) (Foto: Stefan Gröschel, IMB, TU Dresden)

Note: Full exploitation of the Carbon Fiber (CF) is only to achieve by pre-tensioning, which will advantageously compress the usual low tensile strength of the matrices concrete and plastic. Just pre-tensioning of plates is still series production.

Carbon Fiber Production

CF-properties strongly depend on the production process and above precursors which need different conditions but the essential processes are similar. A CF requires a heating and stretching treatment to get the high strength products. A thermoset treatment is first applied in the temperature range from 200 to 400 °C in air under stretching to get the stabilized fiber, followed by a carbonization process in

the temperature range from 800 to 1500 °C in oxygen-free condition to remove impurities and to improve the crystallinity of carbon. To further improve the performance of CFs, a graphitization process is required to graphitize carbonized fibers with temperature up to 3000 °C. During these processes, stretching is required to get preferred orientated carbon crystals, because the crystal alignment makes the fiber incredibly strong and stiff. The graphitization process leads to differences between PAN and Pitch and within the PAN-CFs. This will be later of interest.

The very expensive Pitch CF is mainly used in spacecraft and antennas. The market is dominated by the PAN-CF. With regard to possible toxic fragments, PAN-CF (Ø 7 µm, usually) is therefore of interest, especially the 'highly' graphitized UHM-PANCF such as Torayca's M60J, which comes next to the Pitch-CF considering the tensile modulus (stiffness). CF tensile modulus and fracture toughness naturally depend on the fabrication regarding precursor, on carbonization and graphitization. Furthermore, Pitch-CFs are more layer-like in their crystal structure in contrast to the more granular PAN-CF. This probably further explains the higher tensile modulus compared to the PAN-CF. Knowing the different crystal structure is therefore important for explaining the splintering process, originator of possible toxic fragments.

'WHO-Fiber' criticality

WHO criterion for respirable fibers: 'WHO-Fiber' ≡ tiny fragment of a filament with a diameter Ø of less than 3 µm, a length L of greater than 5 µm and a length-to-diameter ratio of $L/\text{Ø} > 3:1$.

Naming Fiber: (1) Does not address a long CF, which of course never meets the WHO criterion. (2) Asbestos fiber, for example, is just a fiber-like looking particle, which may break into above tiny WHO-size fragments).

Too many dust-related particles, smaller than the WHO 'fiber' size, can also cause a hazard. A so-called Particulate Matter of the µm-size PM2.5 can penetrate into the alveoli and ultrafine particles with a diameter of less than 0.1 µm (*Corona virus size level*) can even penetrate into the lung tissue. Aerosol particles from the environment have diameters ranging from about 1 nanometer (nm) to several 100 micrometers (µm). Larger particles quickly sink to the ground, particles smaller than 10 µm can remain in the air for days.

The figure below summarizes the topics faced when considering the criticality. The macrophage lifespan of a few weeks is one of the decisive factors for the success of disposal or 'cleaning'. 'WHO-fiber'-pierced macrophages usually die.

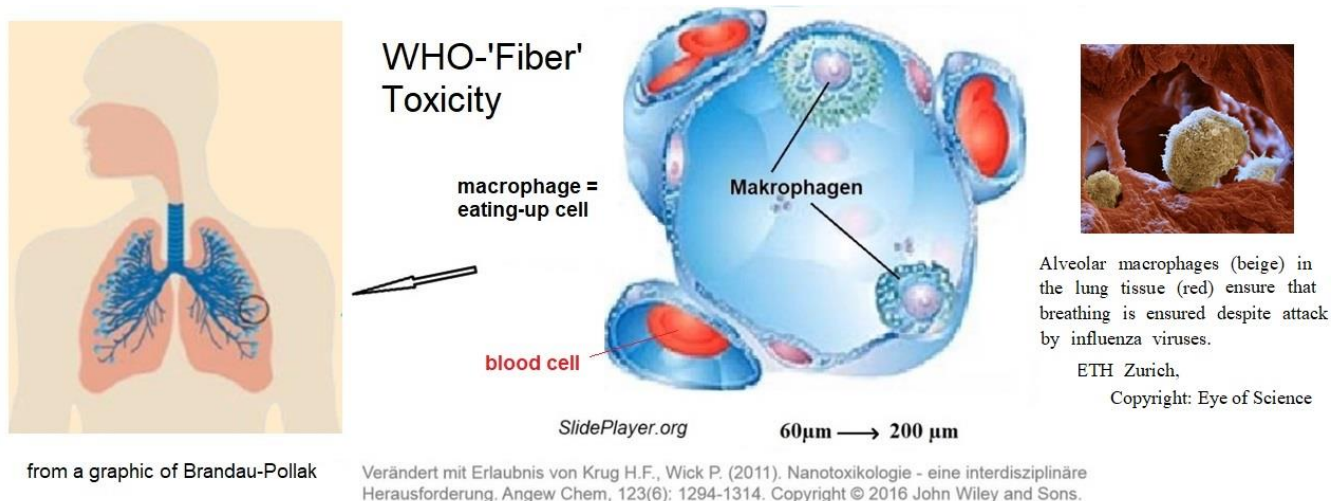


Fig.20-2: Effect of WHO- 'Fibers'

A distinction must be made between long fibers, micro-fragments of fibers such as the 'WHO-fiber' size, as well as the micro-fragments of composite constituents, i.e. fiber-reinforced polymers FRP or fiber-reinforced concrete FRC. In addition to the fiber, the matrix with the interphase material in the fiber-matrix interface must be considered, too.

Criticality-relevant variables are geometry and bio-resistance:

Geometry: Critical are the already defined 'WHO-fiber', as well as dusts and fiber fragments with $\varnothing < 3\mu\text{m}$, which penetrate directly into the alveoli and the lung tissue. Since the 'WHO-fiber' size is smaller than the diameter of common CFs, the fiber fragment must experience a reduction of the diameter. This can happen by splintering or by burning. CF is not toxic per se!

Bio-persistence: High bio-persistence causes high toxicity, a low bio-solubility in living organisms already speaks as an indication of possible carcinogenicity. Fragments with short residence times that are quickly dissolved or removed are less risky.

Only if a sufficiently high amount of CF-'WHO-fibers' is produced and inhaled there is a potential for danger, whereby the following applies:

$$\text{Risk} = \text{hazard potential (severity)} \cdot \text{probability of occurrence.}$$

Hazard potential = exposure to CF-WHO (size) particles combined with toxicity.

The duration of the exposure in terms of quantity and the possible frequency of occurrence of the event per unit of time are therefore decisive.

Generation and Counting of WHO 'fibers'

A quantity for the risk assessment delivers the counting of the fragments which are generated in machining processes. Question: *Which machining processes seems to be the worst for the generation of 'WHO-fiber' shaped CF particles, faced in production and recycling?*

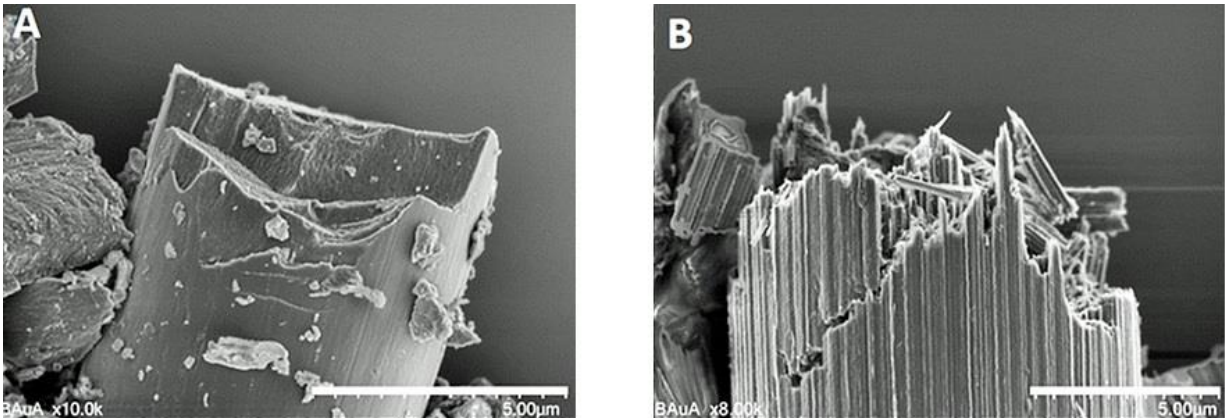


Fig. 20-3:(left) PAN-based, (right) Pitch-based. (Courtesy BAuA, Berlin)

Some answer is given in the BMBF research project *CarboBreak* (headed by BAuA: the Federal Institute for Occupational Safety and Health conducts research for a safe, healthy and humane working environment): Investigation of the release behaviour of respirable fragments made of pure fibres and fibre composites (consisting of CF, sizing, matrix etc.) under mechanical stress. Basically here, rovings were subjected to an extreme mechanical stress in a so-called ball vibrating mill (an assumed 'worst case' machining process), the resulting CF fragments were evaluated with regard to their morphology and then the WHO 'fibers' counted, namely the 'WHO-Fiber' quantity / unit

volume. The CF portion is considered to be the critical part of the full composite. One significant finding was the different splintering process between PAN (left) and Pitch CF (right).

Fact & idea:

- (1) Pitch fibers are obviously more dangerous because they do extremely splinter. Since the UHM-CF comes closest to the pitch fiber in terms of stiffness of all PAN-CFs, the PAN-UHM represents the more critical PAN CF in terms of risk of splintering.
- (2) A CF-parameter is being sought that could be a parameter for explaining the fiber splintering hazard and finding a characteristic.

The sought-after, splinter hazard-descriptive parameter could be the fracture toughness. This property is likely to show some difference in relatively similarly stiff (Young's modulus) brittle materials. *The author lectured fracture mechanics, which he also had to apply at MAN.* His test proposal was a micro-fracture mechanics investigation of a laser-notched single fiber to determine the different brittleness based on the fracture toughness values of K_{Ic} to be measured. In fracture mechanics, fracture toughness describes the resistance of a material to unstable crack progression. An ultra-high graphitized UHM PAN CF such as Torayca's M60J is to be basically investigated, because it is to place narrowest to the behavior of the critical Pitch-Fiber.

Assumption: *Different fracture toughness values indicate different risk of splintering.*

- *The proposed test specimens, together with the difficult notching of a single CF by a laser beam, have already been realized in Kaiserslautern by the institutes IVW with PZKL!
- *The search for a fracture mechanics model that allows us to estimate the fracture toughness of a CF is essential for the qualitative differentiation of the envisaged fibers. A formula will provide a not realistic 'exact', but a quantified relationship which is fully sufficient.

The searched characteristic for the tensioned notched test specimen is the so-called critical stress intensity factor (SIF) K_{Icr} (= fracture toughness), at which unstable crack progression begins. Its formula reads $K_{Icr} = \sigma_{fracture} \cdot \sqrt{\pi \cdot a_{cr}} \cdot Y$, with the so-called geometry factor Y taking the fact into account that the SIF value is theoretically independent of the dimensions of the test specimen only for infinitely large plates. Therefore, the corresponding function Y must be sought for the intended test specimen 'Notched Single Fiber'. This was made possible by the author-available Manual "*NASGRO Reference Manual Version 9.01 Final; December 2018. Fracture Mechanics and Fatigue Crack Growth Analysis Software*".

The application of the full model requires several assumptions:

- CF is a very brittle material
- The crack instability, expressed by the formula, can be applied at the μm -level (micromechanics) for these brittle materials!
- The cross-section, cut by the laser beam, is just a circle section but can be transferred to the elliptical shape of a typical crack
- The 'model for a full cylinder' given as SC07 in the NASGRO document is applicable. Experience has shown that the impact is small, the model can be used also in the μm range
- The crack depth a is given by the laser notch depth.
- Diameter $D = \varnothing = 0.007 \text{ mm}$, UHM 60J.
- The applied stress $\sigma_{fracture}$ at the fiber ends = breaking tensile force F / area A
- The cross-section cut by the laser beam can be transferred to the elliptical shape of a typical SC07 crack. The difference in surface area is neglected because it is the same for all tested

fibers. In the SC07 associated Table C15: For $R/t = 0$, i.e. a solid cylinder with $R = 0$ ($t = \text{wall thickness} = R$), approximately to be expected $a/t = 0.3$, gives $c/t = 0.35$ and thus $Y = 1.6$.

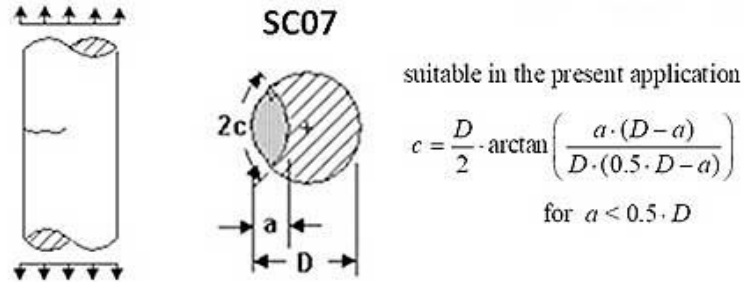


Table C15: CC07 (one crack) - SIF Correction Factors by BEM Analysis (FRANC3D)

Fig. 20-4: Thumbnail crack in a solid cylinder. Surface crack case SC07
Manual NASGRO Reference Manual Version 9.01 Final; December 2018.
Fracture Mechanics and Fatigue Crack Growth Analysis Software

The author's great wish, driven as a GROWIAN wind turbine co-responsible (*about 1980*), in view of future fear-spreading media about a wind turbine fractures with blades made of standard CFs, i.e. not UHM-CFs:

Submission of an 'official recommendation' by the BAuA, together with Composites United (CU), including adapted recycling safety requirements. on working with CFRP in general and specially on PAN-CF carbon reinforced concrete.

LL:

*** The test idea could be fully realized, which is a seldom experienced luck when testing. Unfortunately there is no deeper research ongoing, which would give the basis for the realization of the author's wish.*



Read in Sikkim, about 2011 !

Personal Note on Oil consumption in CF-production and Carbon Concrete Recycling

Fig.20-5 shall give a survey about the portions of the structural materials in the market, dated 2016. It shows how insignificant the carbon fiber content presently is in relation to its origin oil and to the material competitor steel. A yearly CF output of 50000 t equals 4 min steel production (2018). The yearly concrete production equals oil production. This is of basic interest and helpful for many discussions. CF is not yet a real market in construction, basically due to the present regulations of the authorities which does not permit a faster gain of knowledge which is always the result of widespread application, only. Of course, if the concrete mass saving Carbon Concrete market will become significant (*presently about 100.000 t / year*), then CF-production has to be multiplied.

In the context of this chapter's focus and considering recycling: (1) *Why is this marginal crude oil consumption very often considered to be very harmful to the environment.* (2) *Why must Carbon Concrete be recycled by separating the CF and thereby downgrading it to rCF!* The author does not consider it reasonable for ecological and economic reasons to extract CF – as required by the current regulations – from shredded carbon concrete parts instead of bringing the recycled CF material parts together with the multifold concrete content into the superstructure of a bridge or street. For safety reasons one can provide measurements of the traffic-generated abraded dust if no further cover is foreseen and the official recommendation above is not yet available.

If basalt fibers BsF will reach a general approval from sustainability reasons they would be much better ecologically and economically due to the fact that enough base material is available. Added ZrO₂ is foreseen to provide alkali resistance. Unfortunately, the available reliable property knowledge is not made public. Of course, the production of carbon fibers still requires energy. However, this will also be the case if carbon fibers are produced from natural fibers in the future.

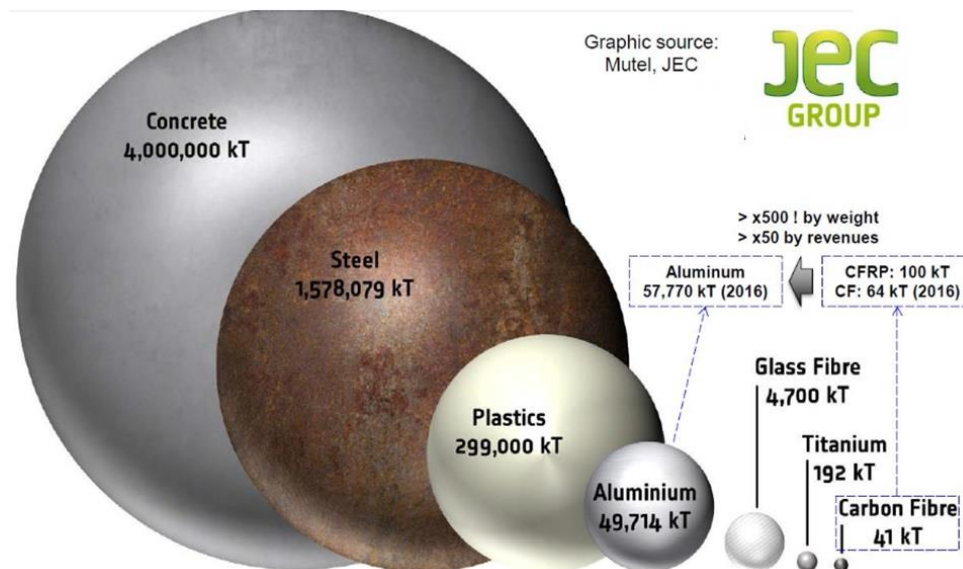


Fig.20-5: Weight ratios of structural materials, year 2016

Please keep in mind:

40000 tons carbon fibers would require just about 40 / 4,000,000 = 0.001 % crude oil.

CF total / Steel = 1/10000.

In Germany it is CF total / concrete reinforcing steel ≈ 0.1%.

Concrete / crude oil = 1, GF / CF = 100.

Single car consumes about 1 t oil / year.

21 A novel Determination of the Residual Strength R_{res} , non-cracked, Fatigue Phase 2

Aim: Derivation of a procedure to determine and rendering the residual strength value R_{res}

21.1 General for a Proof of Structural Integrity in Projects

Residual strength R_{res} is the fracture stress after pre-damage and re-loading. Not only in mechanical engineering design but also in civil engineering residual strength values are required such as in soil mechanics or for UD-hangers of a railway bridge at Stuttgart, below a hanger or for tension rods of cranes.



Fig.21-2 Stuttgart Stadtbahn bridge. World's first network arch railway bridge (127 m) that hangs entirely on tension elements made of carbon fiber-reinforced plastic (CFRP). The 72 hangers are produced by Carbo-Link AG

The value is of basic interest, because – due to authority demands - Design Ultimate Load is to sustain even after a distinct fatigue life. The residual strength task is one task to demonstrate structural integrity.

This subject is linked to cyclically micro-damaged structural components (*Phase 2 of fatigue life, strength tools applied*) and macro-damaged ones (*Phase 3 of fatigue life, fracture mechanics problem, damage tolerance mechanics tools applied*), as displayed in *Fig.21-2*. The cyclic loading may range from constant amplitude-loading up to spectrum-loading and has to capture proportional and non-proportional loading scenarios.

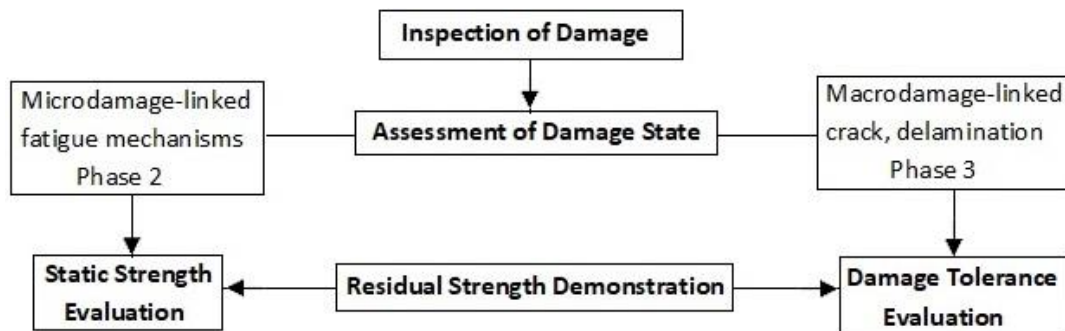


Fig.21-2: Ways of residual strength determination

This task especially comes up in cases such as: A multiple site damage phenomenon is faced with aerospace components such as fabrication-induced flaw clouds (fatigue strength problem, Ariane 5 Booster wall) or real short-crack ‘clouds’ from e.g. multiple rivet holes in stringer-stiffened panels of

aging aircraft components (fracture mechanics problem). Here, the focus is on the Phase 2 residual strength R_{res} . Mind: R_{res} should not be confused with *residual stress* σ_{res}).

In some projects a number for the residual strength at a certain operation cycle value is required. This is well known from impact cases of laminated panels. There, a Compression-After-Impact (CAI) test is to execute after the impact event because the impact may result in a barely visible external damage and it may generate a dramatic reduction of compressive strength due to separation of layers resulting in a large bending stiffness loss. Regarding crack-linked fatigue life Phase 3 residual strength problems the reader is referred to fracture mechanics.

Residual strength tests are long-lasting and expensive. Therefore, procedures are searched that help to reduce the test effort if enough physical knowledge is available.

First step is to map the relevant SN-curve (Wöhlerkurve) by taking the widely used 4-parameter Weibull function

$$\text{stress ratio } R = \sigma_{\min} / \sigma_{\max} = \text{constant} : \sigma_{\max}(R, N) = c1 + (c2 - c1) / \exp(\log N / c3)^{c4}.$$

(stress ratio \rightarrow straight letter R, strength \rightarrow bias letter R).

An SN-curve describes the relation between the cyclic loading and the number of cycles to failure N . On the horizontal axis in Fig.21-3 the number of cycles to failure is given on logarithmic scale. On the vertical axis (*either linear or logarithmic*) the stress amplitude $\sigma_{\text{amplitude}}$ of the cycle is often given. In the case of brittle materials *sometimes the maximum stress* σ_{\max} . The provided mean SN-curves, $R = \text{constant}$, base on the fatigue test measurement types ‘pearl-chain testing’ or ‘horizontal load level testing’. Fatigue curves are given for un-notched test specimens ($K_t = 1$) and for notched ones, the loading can be uniaxial or multi-axial. Considering residual strengths, measurements on the vertical axis at $n = \text{constant}$ are required.

In design verification very often as fractile (quantile) numbers, representing the failure probability p_f , 5% or 10% are taken in order to capture some uncertainty compared to the average of 50%. For the loading side the design FoS j , in construction γ , capture the uncertainty of the loading. The residual strength design verification has to meet Design Ultimate Load. Following HSB 62200-01 the determination of the static residual strength for single load paths must be made with statistically significant A-values; for possible multiple load path structural parts B-values may be used.

Moving to the required statistical properties some notions are to depict. Capturing the uncertainty of the resistance quantities, the following is performed: Denoting P the survival probability and C the confidence level applied, when estimating a basic population value from test samples, partly enriched by some knowledge of the basic population. Regarding C a one-sided tolerance level it reads:

Static \rightarrow Statistical reduction of average strength from (P= 50%, C= 50%) to e.g. (B-value: P = 90%, C = **95%**).

Cyclic \rightarrow Statistical reduction of average SN curve from (P=50%, C= 50%) to e.g. (P= 90%, C= **50%**).

All this is executed to keep a generally accepted survival reliability of about $\mathfrak{R} = 1 - p_f > 1 - 10^{-7}$.

21.2 Classical way to determine R_{res}

Determination via the interpretation “The course of the residual strength is the difference of the static strength and the maximum strength $\sigma_{max}(N)$ of an SN curve R”, see Fig.21-3. This leads to the

formulation $R_{res} = \sigma_{max}(N) + [R^t - \sigma_{max}(N)] \cdot \rho(n)$ with $\rho(n) = 1 - (n/N)^p \equiv 1 - D^p$,

where the exponent p describes the decay of the residual strength capacity and D the micro-damage quantity, (see Hahne C: Zur Festigkeitsbewertung von Strukturbauteilen aus Kohlenstofffaser-Kunststoff-Verbunden unter PKW-Betriebslasten. Shaker Verlag, Dissertation 2015, TU-Darmstadt).

Fig.21-3 depicts for $R = 0.1$ the mean (average) 50% SN-curve and the 90% SN-curve. The residual strength curve R_{res} is given for the point (10^5 cycles, $\sigma = 34$ MPa). The stress σ belongs to a so-called ‘one stage test’ or constant amplitude test. Regarding the residual strength value at the 90% SN-curve the question arises: “Where does the necessary statistical basis for a reduced SN-curve come from, if not sufficient test series on vertical and horizontal levels were run”?

Due to missing test data a test data-based work case cannot be presented. Therefore, the author tried to figure out a procedure which gives an understanding of the subject.

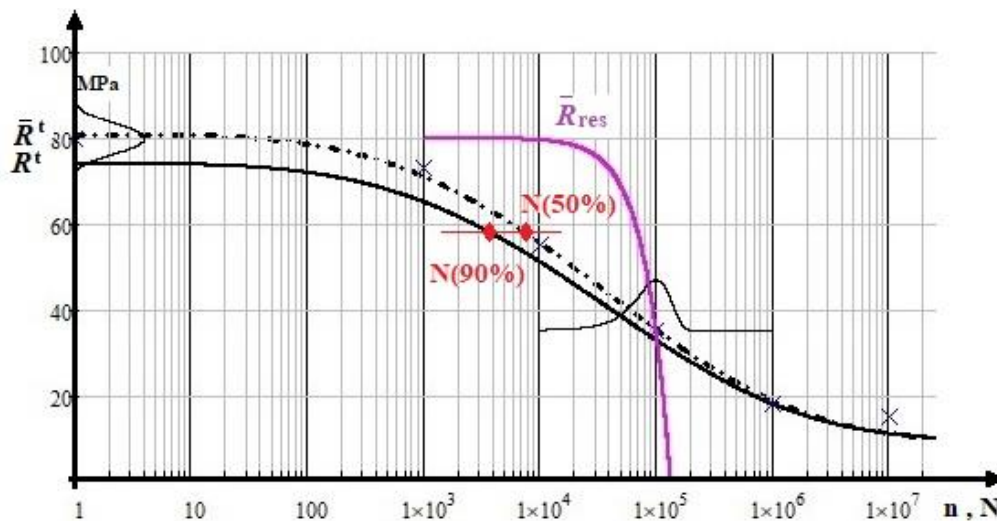


Fig.21-3, Schematic example, uniaxial loading: $R = 0.1$. \bar{R}_{res} is mean tensile residual strength

21.3 Idea Cuntze, probabilistic way to determine a 90% value by the convolution integral

A possibility to determine a 90%-value is given by the application of the so-called convolution integral, using density distributions of R_{res} and of N with just a little hope to find the distribution measured, Fig.21-3. The output of the mathematical expression convolution integral represents the probability of failure p_f . The numerical analysis is based here on the assumption: ‘The density distributions on x- (f_N) and y-axis (f_{Res}) are approximately basic populations and of Normal Distribution-type’ f_{ND} (for the density distributions also a logarithmic, a Weibull density function or a truncated function could be employed). The convolution integral, solved by Mathcad 15, reads

$$(1 - p_f) = \mathfrak{R} = p_{ii} = \int_{-\infty}^{\infty} \left(\int_{R_{res}} f_{Res}(R) \cdot dR \cdot f_N(N) \right) dN = 90\% \text{ fractile for ND density distributions}$$

$$\text{with } f_{ND}(x) = \frac{1}{\sigma \cdot \sqrt{2 \cdot \pi}} \cdot \exp \left[-\frac{1}{2} \left(\frac{x - \mu}{\sigma} \right)^2 \right] \text{ for abscissa } N \text{ and ordinate } R^t .$$

Data base of the numerical probabilistic example (statistical: μ = mean, σ = standard deviation) is:

- * Static strength distribution $\mu = 80$ MPa, $\sigma = 3.2$ MPa
- * R_{res} distribution in computation point, y-axis, $\mu = 43.5$ MPa, $\sigma = 2.9$ MPa
- * Cycle distribution in computation point, x-axis, $\mu = 3431$ cycles, $\sigma = 446$ cycles and the Coordinates of the chosen computation point * (38 MPa, $n = 2000$ cycles in Fig.21-4).

(Note, please: The presented application outlines a limit of the Mathcad 15 code application. Mathcad has

(Note, please: The presented application outlines a limit of the Mathcad 15 code application. Mathcad has no computation problem with the computation of the required so-called convolution integral. However, when visualizing the probability hill in Fig.21-5, it was only partly able to manage the 'big data' problem and runs into endless loops. Therefore the author had to sort out a work case with reduced stress and cycle regimes. The original SN data set was for fiber fracture (FF) of CFRP considering the hanger. This reduction to a relatively simple numerical example does not matter because the procedure is of interest and will explain the posed task.)

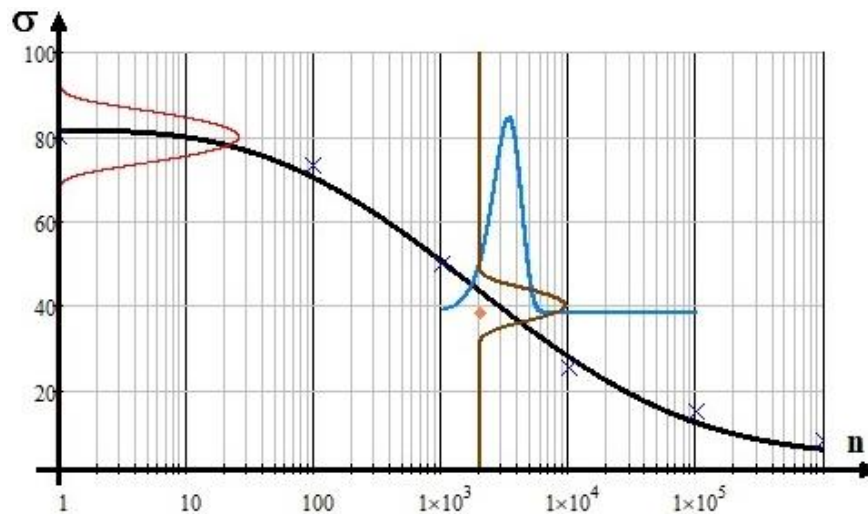


Fig.21-4, Simplified Mathcad calculable example: Assumed distributions of residual strength and cycles linked to R_{res} (38 MPa, 2000 cycles). SN-curve, $R = 0.1$: $c1 = 20$ MPa, $c2 = 80$ MPa, $c3 = 3.77$, $c4 = 2.92$

Fig.21-4 depicts the SN-curve, the chosen computation point, static strength distribution with an assumed residual strength distribution and cycle distribution, all through the computation point *. It is a semi-logarithmic graph. As it is a brittle example material, the use of σ_{max} (involves R^t as origin!) as ordinate is of advantage for the 'strength-oriented' design engineer compared to using a stress amplitude σ_a .

The probabilistic treatment delivers the 'joint' probability hill of both the distribution functions in Fig.21-5, (right). The hill's average center coordinates are 43.5 MPa, 3430 cycles. The figure further depicts the density distributions of the residual strength R_{res} (σ) and of the fracture cycle N .

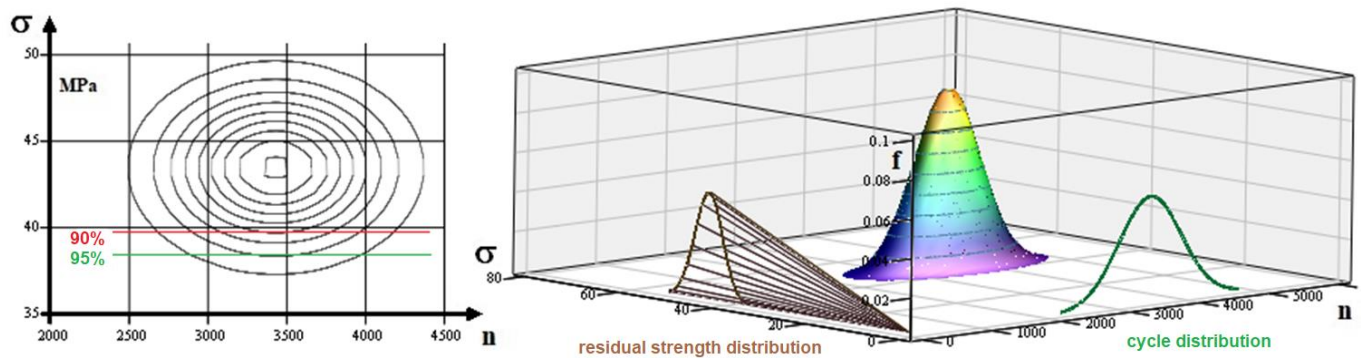


Fig.21-5: (right) Cyclic distributions and assumed residual strength distribution with survival probability hill applying the convolution integral. (left) Projection of lines of equal probability with two chosen residual strength cut-offs, M is the hill designation

In the right part figure, the residual strength distribution is not clearly visible due to additional Mathcad-drawn beams running out from the origin, which are to neglect. The task seems to be an overloading of the Mathcad code which could not anymore handle the numerically effortful task for too large cycle numbers. The left figure shows the projection of the probability hill with lines of equal probability belonging to the chosen computation point *. Below, the computation parameter input set is depicted:.

Design Safety considering the scatter of the design parameters is tackled as follows:

The scatter of loading is considered in the residual strength design verification because DUL with its design safety factor j_{ult} has to be verified. The scatter of the residual strength R_{res} and of the fracture cycle N is captured by a joint probability calculation indicated below. This procedure is effortful, however of high fidelity if test data is available.

Under above assumptions an estimation of a required 90%-linked residual (tensile) strength value can be determined according to the formula below representing the probability hill volume truncated by

R_{res}

$$\begin{aligned}
 \mu_{NR} &= 3431 & \sigma_{NR} &= 446 & \mu_{\sigma R} &= 43.5 & \sigma_{\sigma R} &= 2.9 & n &:= 2000..6000 & \sigma &:= 0..80 \\
 x_n &:= \text{dnorm}(n, \mu_{NR}, \sigma_{NR}) & y_{\sigma} &:= \text{dnorm}(\sigma, \mu_{\sigma R}, \sigma_{\sigma R}) & F(x, y) &:= x \cdot y & M_{n, \sigma} &:= F(x_n, y_{\sigma}) \cdot 750 \\
 p_{\bar{u}} &:= \int_{\sigma_{res}}^{R_t + \sigma R_t} \int_{2500}^{5500} \frac{1}{\sigma_{\sigma R} \cdot \sqrt{2\pi}} \cdot e^{-\frac{1}{2} \left(\frac{y - \mu_{\sigma R}}{\sigma_{\sigma R}} \right)^2} \left[\frac{1}{\sigma_{NR} \cdot \sqrt{2\pi}} \cdot e^{-\frac{1}{2} \left(\frac{x - \mu_{NR}}{\sigma_{NR}} \right)^2} \right] dx dy
 \end{aligned}$$

The computation delivers for the point ($\sigma_{res} \equiv R_{res} = 38.0$ MPa, 2000 cycles) the value $p_{\bar{u}} = P = 95\% = \mathfrak{R}$.

Setting the value 39.5 MPa, the demanded survival probability $p_{\bar{u}} = 90\% = (1 - p_f)$ is obtained for R_{res} .

LL:

➤ The proposed procedure clearly shows how to statistically understand a residual strength value

- It could be proven that the proposed model leads to an acceptable value for the residual strength of fatigued, non-cracked structural parts.

21.4 Residual Strength R_{res} , pre-cracked, Fatigue phase 3, Fracture Mechanics (for completion)

To estimate the residual strength of a pre-cracked structural part or the critical length of an initial macro-crack is essential regarding the questions:

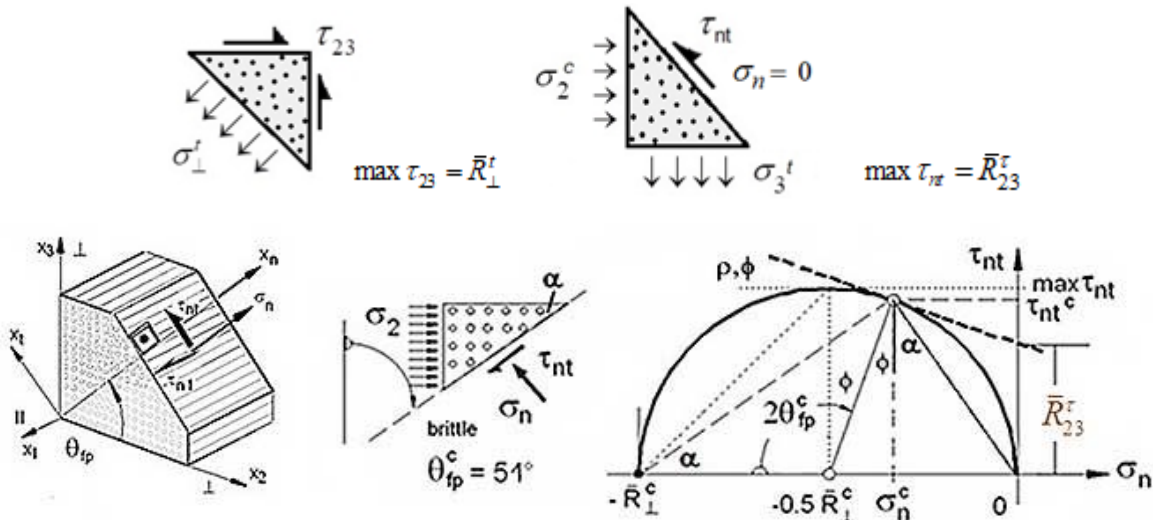
- (1) Is the crack-length at the end of static loading critical?
- (2) Is the crack-length at the end of cyclic loading critical for further static loading, considering a SN-curve? Here, the certification of cracked components in aircraft structures requires a damage tolerance assessment.

22 Full Mohr UD Envelope $\tau_{nt}(\sigma_n)$, Derivation of $\Theta_{fp}(\sigma_n)$ and of Cohesive shear Strength

Aim: Unlocking the ‘mystery’ behind the shear quantities R_{23}, R_{23}^r and R_{23}^A faced in UD analysis.

22.1 Shear Strength Quantities in Analysis, Survey

Fig.22-1 collects all figures which are necessary to understand the difference of applied shear quantities (*upper part figure*): Shear fracture stress (Tsai-Wu, Hashin) and so-called cohesive strength R_{23}^r (construction, rock mechanics) and the Action plane shear strength R_{23}^A (Puck).



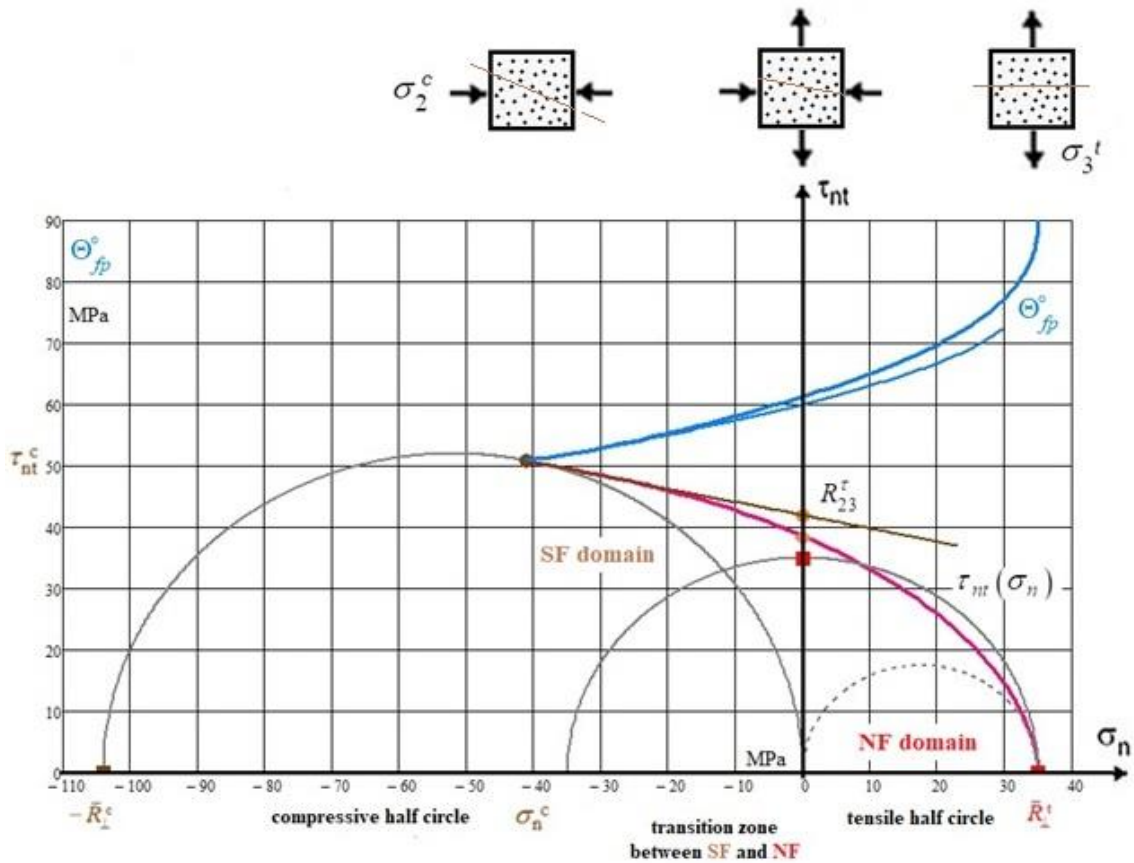


Fig. 22-1: (up) Difference of transversal shear fracture stress and cohesive strength. (below) Mohr-Coulomb curve characteristics, Mohr shear curves $\tau_{nt}(\sigma_n)$ with its special points and three Mohr half-circles

The brown curve in Fig.22-1 is the **Linear Mohr-Coulomb (M-C)** curve. This approach is a simple IFF2-extrapolation from the compressive strength point, keeping the fracture angle measure $C = C_{fp}^c$ constant, when estimating the so-called cohesive strength by $R_{23}^r = \tau_{nt}^c + \mu \cdot \sigma_n^c$ at $\sigma_n = 0$. The letters $\rho = \phi$ address the so-called friction angle. The 3 sketches above the bottom figure demonstrate that the cohesive strength point \bar{R}_{23}^r is located in the mode's transition zone and cannot be reliably estimated just by an IFF2-Extrapolation, employing just SF! The parallel acting normal fracture part NF, namely IFF1, was neglected. But IFF1 usually causes much more failure danger than the compression-linked shear mode IFF2 from the transition zone beginning on.

The analytical determination of the M-C failure curve and of a value for the cohesive strength depends on the quality of the used IFF2-model and the interaction of both in the transition zone. Therefore, in order to accurately determine $\tau_{nt}(\sigma_n)$ both the modes are to include in the derivation process of a **realistic M-C curve**, the determination of the **fracture angle** Θ_{fp}° and of the **cohesive strength** \bar{R}_{23}^r at $(\tau_{nt}^{\text{fracture}}, \sigma_n = 0)$. An

improved treatment by a correction f_{corr} of the M-C curve has been effortful executed by the author in [Cun23b]. This became necessary because any SFC has to be as simple as possible. Of course, this means that all presently applied SFCs have a deficiency in the mode transition zones. The author has compensated for this with a correction, for details of this elaboration see [Cun]. The bottom figure in Fig.22-1 displays, how the fracture angle increases, when approaching \bar{R}_1^r . Thereby the **bold** curve represents the optimum corrected mapping of the M-C curve in the transition zone around $\sigma_n = 0$.

Now the steps of the tedious way obtain Fig.22-1 shall be presented.

22.2 Relations for a Transformation from a Test Fracture Curve $\sigma_3(\sigma_2)$ to Mohr's $\tau_{nt}(\sigma_n)$

The general stress state $\{\sigma\}$ in the material point of the lamina has to be transformed around the 1-axis to the arbitrary Mohr stress state $\{\sigma^\theta\} = [T_\sigma(\theta)] \cdot \{\sigma\}$, a fibre-parallel plane, by applying Fig.22-1, wherein $c := \cos \theta$, $s := \sin \theta$ and n is normal to the 'action plane' [Cun22]. Values of the parameters depend on the approach, whether it is a linear or a parabolic one.

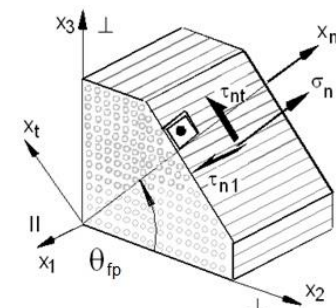
$$\begin{matrix} \rightarrow \begin{bmatrix} \sigma_1 \\ \sigma_n \\ \sigma_t \\ \tau_{nt} \\ \tau_{t1} \\ \tau_{n1} \end{bmatrix} \\ \rightarrow \{\sigma^\theta\} = [T_\sigma(\theta)] \{\sigma\} = \\ \rightarrow \end{matrix} \begin{bmatrix} 1 & 0 & 0 & 0 & 0 & 0 \\ 0 & c^2 & s^2 & 2sc & 0 & 0 \\ 0 & s^2 & c^2 & -2sc & 0 & 0 \\ 0 & -sc & +sc & c^2 - s^2 & 0 & 0 \\ 0 & 0 & 0 & 0 & c & -s \\ 0 & 0 & 0 & 0 & +s & c \end{bmatrix} \begin{bmatrix} \sigma_1 \\ \sigma_2 \\ \sigma_3 \\ \tau_{23} \\ \tau_{31} \\ \tau_{21} \end{bmatrix}$$


Fig. 22-2: Visualization of the transformation of lamina stresses into associated Mohr stresses. $\theta = \theta_{fp}$ denotes the angle of the anti-clockwise transformation from the (1, 2, 3)-CoS to the (1, n, t)-CoS

According to

$$\sigma_n^A(\theta) = c^2 \cdot \sigma_2 + s^2 \cdot \sigma_3 + 2 \cdot s \cdot c \cdot \tau_{23}, \quad \tau_{n1}^A(\theta) = c \cdot \tau_{21} + s \cdot \tau_{31}, \quad \tau_{nt}^A(\theta) = -s \cdot c \cdot (\sigma_2 - \sigma_3) + (c^2 - s^2) \cdot \tau_{23}$$

the transformed stresses $\sigma_n(\theta)$, $\sigma_t(\theta)$, $\tau_{nt}(\theta)$, which Puck termed 'Action Plane' Stresses, Fig.22-1, right, in the turned CoS depend on $(\sigma_2, \sigma_3, \tau_{23})$ only, whereas τ_{t1} , τ_{n1} is linked to (τ_{31}, τ_{21}) . They are acting in the potentially physical (fracture) failure 'plane' and are decisive for fracture. In case of normal stress- induced fracture (NF) σ_n will be responsible for fracture and in case of shear stress- induced fracture τ_{nt} will be the fracture dominating Mohr stress. The Mohr stress τ_{t1} has no impact but has to be considered in the derivations of the *Eff*-functions until it vanishes during the later transformation process.

Fracture plane will become that 'action plane' where the material stressing effort $Eff(\sigma(\theta))$ will reach the value 1 = 100% at (maximum) failure loading and by that, where the theoretical material reserve factor f_{RF} will become a minimum.

22.2 Accuracy Problem of the IFF2-model in the transition zone IFF2 (SF) - IFF1 (NF)

In this subchapter the cohesion strength R_{23}^τ , activated by τ_{nt} in the quasi-isotropic plane of the UD material is envisaged. This quantity is located in the transition zone of the two modes IFF1 and IFF2. With isotropic materials the author learned that a transformation from UD lamina stresses into the desired Mohr stresses τ_{nt} , σ_n must be also possible. Thereby a closer look at R_{23}^τ and at the Mohr envelope $\tau_{nt}(\sigma_n)$ or M-C curve will be possible.

Here addressed is the quasi-isotropic UD plane (*works similar to isotropic concrete materials, using available multi-axially compression test-based data [Cun22]*). The compromise is on the 'safe Reserve Factor side'. This means: The engineering approach of above $Eff^{d,\tau}$ (SF) is not problematic for Design Verification, because $Eff = 1$ delivers conservative *RF*-values in the transition zone, since

the curve runs more internally due to the generally *minimum value choice* of the interaction exponent m .

Focus here is the derivation of $\tau_{nt}(\sigma_n)$, $\Theta_{fp}(\sigma_n)$ and R_{23}^τ from a **well mapped** measured fracture curve $\sigma_3(\sigma_2)$ and its course in the 2nd quadrant of $\sigma_3(\sigma_2)$. In Table 22-1 all relations necessary for the transformation are compiled and formulas for the searched entities τ_{nt} , σ_n , Θ_{fp}° are presented. After transformation of the UD lamina (layer) stresses σ_2 , σ_3 , τ_{23} in the quasi-isotropic plane into the principal stresses σ^{pr} (*index^{pr} means principal*), the shear stress τ_{23} vanishes. Therefore, with no loss of generality σ^{pr} can be simpler written in the further text, back again as plain letter σ , but thinking they might be principal stresses acting in the quasi-isotropic plane. In the addressed quasi-isotropic plane this transformation of the lamina stresses into Mohr stresses practically works via addition theorems and using $C(\Theta_{fp}^\circ) = \cos\Theta^2 - \sin\Theta^2$, which might be termed ‘fracture angle measure’.

As the author still found with isotropic materials, the interaction considering magenta curve (**thinly**-marked) in Fig.22-3 cannot accurately map the course of test data. The improved **bold**-marked curve is physically more accurate and this local mapping shortcoming is to model more detailed as follows. Fig 22-1 shows that with the IFF2-function the shear effort $Eff^{d\tau}$ cannot become zero in the M-C domain at $\sigma_2 = 0$. This numerical behavior is a shortcoming in the transition zone of the ‘simple’ engineering FMC-based IFF2 approach. An accurate alteration of the fracture angle Θ_{fp}° and of the associated Mohr stresses τ_{nt} , σ_n is not to achieve with the mathematical course of the given ‘engineering’ IFF2 function. The mapping quality of the given IFF2 is not fully sufficient if the alteration of the fracture angle Θ_{fp} in the transition zone is to determine. This bi-axially stressed transition zone between the normal fracture mode domain NF and the shear fracture mode domain SF is ruled by interaction and therefore requires both the Eff-modes to be inserted into the interaction equation $Eff = 1$. Specific points of the investigated M-C domain are: $(\sigma_2 = -\bar{R}_\perp^c, 0) \rightarrow (\sigma_2, \sigma_3 = -\sigma_2) \rightarrow (0, \sigma_3 = \bar{R}_\perp^t)$. In order to sort out a better mapping description it is essential to know how the pure mode efforts of the activated modes IFF1 and IFF2 change its influence along the σ_2 -axis, which is depicted in Fig.22-2. $Eff^{d\tau}$ firstly becomes zero at the equi-biaxial tensile ‘strength’ point $(\bar{R}_\perp^{tt}, \bar{R}_\perp^{tt}) < \bar{R}_\perp^t$. This zero point lies physically ‘too late’ for a more accurate revised local mode description. An improvement is to achieve.

22.3 Improvement of the IFF2 Criterion in the Transition Zone

The required entities τ_{nt} , σ_n , Θ_{fp}° and \bar{R}_{23}^τ only become accurate if a physically necessary correction of $Eff^{d\tau}$ is considered by using a correctively acting decay function f_{corr} . In order to implement f_{corr} one just has to replace $a_{\perp\perp}$ by $f_{corr} \cdot a_{\perp\perp}$ and $b_{\perp\perp}$ by $f_{corr} \cdot b_{\perp\perp}$. For a realistic transformation of the test curve, formulated in lamina stresses into a Mohr stress formulation, it is considered that $Eff^{d\tau}$ (SF) physically must become zero when reaching the pure NF domain at the point $(\sigma_3 = \bar{R}_\perp^t, \sigma_3 = 0)$, (see the course in Fig.22-3):

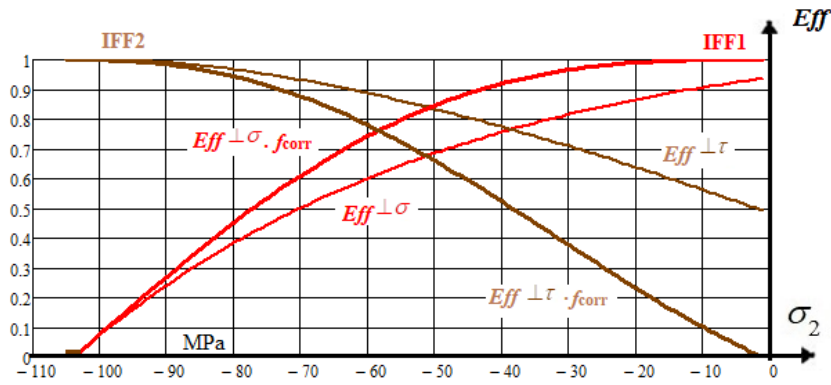


Fig.22.3: Course of the two efforts $Eff^{\perp\sigma}$, $Eff^{\perp\tau}$ composing the fracture stress curve $Eff=1=100\%$.

$$\bar{R}_{\perp}^c = 104 \text{ MPa}, \bar{R}_{\perp}^t = 35 \text{ MPa}, \Theta_{fp}^{\circ} = 51^{\circ}, C^c = -0.21 \leftrightarrow a_{\perp\perp} = 0.26, \mu_{\perp\perp} = 0.21.$$

$$f_{\text{corr}} = 1 + c_0 \cdot (\bar{R}_{\perp}^c + \sigma_2)^2 \quad \text{with } c_0 \text{ from inserting } (\sigma_2 = 0, \sigma_3 = \bar{R}_{\perp}^t), c_0 = 8.9 \cdot 10^{-5}.$$

$$\text{IFF1: } Eff^{\perp\sigma} = [(\sigma_2 + \sigma_3) + \sqrt{\sigma_2^2 - 2\sigma_2 \cdot \sigma_3 + \sigma_3^2}] / 2\bar{R}_{\perp}^t,$$

$$\text{IFF2: } Eff^{\perp\tau} = [a_{\perp\perp} \cdot f_{\text{corr}} \cdot (\sigma_2 + \sigma_3) + b_{\perp\perp} \cdot f_{\text{corr}} \sqrt{\sigma_2^2 - 2\sigma_2 \sigma_3 + \sigma_3^2}] / \bar{R}_{\perp}^c.$$

Similar to the isotropic case the bi-axial stress-ruled quasi-isotropic M-C curve, located in the quasi-isotropic plane, is oppositely dominated by two modes, IFF2 (SF) with IFF1 (NF). Therefore, attention was paid to the interaction of both these modes in the transition zone in order to finally obtain an ‘accurate’ fracture angle Θ_{fp}° , being the pre-condition to determine the envisaged two Mohr stresses τ_{nt} , σ_n . the shear material stressing effort $Eff^{\tau} = Eff^{\text{SF}}$ must physically become zero at the tensile strength point $(0, R^t)$. This specific shortcoming is brought about by a correction function that defines the decay of Eff^{τ} and is practically performed by setting $Eff^{\tau} = 0$ at $\sigma_{II} = 0$. As decay function was taken an exponential one, namely:

$$f_d = 1 / (1 + \exp(\frac{c_{1d} + \sigma_{II}}{c_{2d}})), \quad \text{with } c_{1d}, c_{2d} \text{ fixed at } (-\bar{R}^c, 0.995), (-0.01, +0.01).$$

The correction changes the formula for the determination of the fracture angle measure C in Table 22-1.

Table 22-1: IFF2-IFF1-interacted Derivation of $\tau_{nt}(\sigma_n), R_{23}^t, \Theta_{fp}$ from a measured curve $\sigma_3^{fr}(\sigma_2)$

$$SF: Eff^{\perp\tau} = [a_{\perp\perp} \cdot (I_2) + b_{\perp\perp} \cdot \sqrt{I_4}] / \bar{R}_{\perp}^c = 1$$

$$= [a_{\perp\perp} \cdot (\sigma_2^{pr} + \sigma_3^{pr}) + b_{\perp\perp} \cdot \sqrt{(\sigma_2^{pr} - \sigma_3^{pr})^2 + 0^2}] / \bar{R}_{\perp}^c = 1 \text{ (lamina stresses)}$$

$$\equiv [a_{\perp\perp} \cdot (\sigma_n + \sigma_t) + b_{\perp\perp} \cdot \sqrt{(\sigma_n - \sigma_t)^2 + 4\tau_m^2}] / \bar{R}_{\perp}^c = 1 \text{ (in Mohr stresses)}$$

$$NF: Eff^{\perp\sigma} = (\sigma_2^{pr} + \sigma_3^{pr}) + \sqrt{(\sigma_2^{pr} - \sigma_3^{pr})^2 + 0^2} / 2 \cdot \bar{R}_{\perp}^t = 1$$

$$\equiv [(\sigma_n + \sigma_t) + \sqrt{(\sigma_n - \sigma_t)^2 + 4\tau_m^2}] / 2 \cdot \bar{R}_{\perp}^t = 1.$$

Known: $\sigma_2^{pr}, \sigma_3^{pr}$. Searched is: $\sigma_n, \tau_m, \Theta_{fp}$ with $C = \cos(2 \cdot \Theta_{fp} \circ \pi / 180^\circ)$.

Use of addition theorems, $\sigma_\lambda = 0$. For lamina stresses ^{pr} now dropped for simplification

$$\sigma_n - \sigma_t = c^2 \cdot (\sigma_2 - \sigma_3) - s^2 \cdot (\sigma_2 - \sigma_3) = C \cdot (\sigma_2 - \sigma_3), \quad S = \sqrt{1 - C^2}$$

$$\sigma_t = \sigma_n - C \cdot (\sigma_2 - \sigma_3), \quad C = c^2 - s^2 = 2c^2 - 1 = 1 - 2s^2, \quad \sigma_n + \sigma_t = \sigma_2 + \sigma_3,$$

$$\tau_m = -0.5 \cdot S \cdot (\sigma_2 - \sigma_3) = -0.5 \cdot \sqrt{1 - C^2} \cdot (\sigma_2 - \sigma_3), \quad \sigma_n = (C + 1) \cdot 0.5 \cdot \sigma_2 + (1 - C) \cdot 0.5 \cdot \sigma_3.$$

Differentiation of structural stresses-linked Mohr stresses delivers (minus due to implicit derivation)

$$\frac{d\tau_m}{d\sigma_n} = \frac{(s^2 - c^2) \cdot (\sigma_2 - \sigma_3)}{-2 \cdot s \cdot c \cdot (\sigma_2 - \sigma_3)} = \frac{C}{S}, \text{ valid } uni\text{- and } bi\text{-axial (like isotropic!).}$$

Fracture (interaction) equation \equiv mathematical equation of the fracture body

$$Eff = [(Eff^{NF})^m + (Eff^{SF})^m]^{m^{-1}} \text{ or computationally simpler}$$

$$(Eff^{NF})^m + (Eff^{SF})^m = 1 = 100\% \text{ total effort fracture curve}$$

From differentiation of the interaction equation (σ_t goes away)

$$\{[(\sigma_n + \sigma_n - C \cdot (\sigma_2 - \sigma_3)) + \sqrt{(\sigma_n - \sigma_n - C \cdot (\sigma_2 - \sigma_3))^2 + 4\tau_m^2}] / 2 \cdot \bar{R}_{\perp}^t\}^m +$$

$$+ \{[a_{\perp\perp} \cdot (\sigma_n + \sigma_n - C \cdot (\sigma_2 - \sigma_3)) +$$

$$+ b_{\perp\perp} \cdot \sqrt{(\sigma_n - \sigma_n - C \cdot (\sigma_2 - \sigma_3))^2 + 4\tau_m^2}] / \bar{R}_{\perp}^c\}^m = 1.$$

are obtained the two equations

$$d[(Eff^{NF})^m + (Eff^{SF})^m] / d\sigma_n =$$

$$m \cdot \{2\sigma_n - C \cdot (\sigma_2 - \sigma_3) + \sqrt{(C \cdot (\sigma_2 - \sigma_3))^2 + 4\tau_m^2} / 2\bar{R}_{\perp}^t\}^{m-1} / \bar{R}_{\perp}^t +$$

$$+ 2a_{\perp\perp} \cdot m \cdot \{a_{\perp\perp}(2\sigma_n - C \cdot (\sigma_2 - \sigma_3)) + b_{\perp\perp} \sqrt{(C \cdot (\sigma_2 - \sigma_3))^2 + 4\tau_m^2} / \bar{R}_{\perp}^c\}^{m-1} / \bar{R}_{\perp}^c,$$

$$d[(Eff^{NF})^m + (Eff^{SF})^m] / d\tau_m =$$

$$\frac{2m \cdot \tau_m \cdot \{2\sigma_n - C \cdot (\sigma_2 - \sigma_3) + \sqrt{(C \cdot (\sigma_2 - \sigma_3))^2 + 4\tau_m^2} / 2\bar{R}_{\perp}^t\}^{m-1}}{\bar{R}_{\perp}^t \cdot \sqrt{(C \cdot (\sigma_2 - \sigma_3))^2 + 4\tau_m^2}} +$$

$$+ 4b_{\perp\perp} \cdot m \cdot \{a_{\perp\perp}(2\sigma_n - C \cdot (\sigma_2 - \sigma_3)) + b_{\perp\perp} \sqrt{(C \cdot (\sigma_2 - \sigma_3))^2 + 4\tau_m^2} / \bar{R}_{\perp}^c\}^{m-1} / \bar{R}_{\perp}^c.$$

Equating above two equations and replacing Mohr stresses by ply stresses

$$\text{via } \sigma_n = (C + 1) \cdot 0.5 \cdot \sigma_2 + (1 - C) \cdot 0.5 \cdot \sigma_3, \quad \tau_m = -0.5 \cdot \sqrt{1 - C^2} \cdot (\sigma_2 - \sigma_3)$$

yields an implicitly to solve equation for the fracture angle measure C

$$\frac{C(\sigma_2, \sigma_3)}{\sqrt{1 - C^2}} = - \left[\frac{A}{\bar{R}_{\perp}^t} + \frac{2 \cdot a_{\perp\perp} \cdot B}{\bar{R}_{\perp}^c} \right] / \left[\frac{2 \cdot A}{\bar{R}_{\perp}^t \sqrt{(\sigma_2 - \sigma_3)^2}} + \frac{4 \cdot b_{\perp\perp} \cdot \tau_m(C)}{\bar{R}_{\perp}^c \sqrt{(\sigma_2 - \sigma_3)^2}} \right] \text{ with}$$

$$A = \left[\frac{\sigma_2 + \sigma_3 + \sqrt{(\sigma_2 - \sigma_3)^2}}{2 \cdot \bar{R}_{\perp}^t} \right]^{m-1}, \quad B = \left[\frac{a_{\perp\perp} \cdot (\sigma_2 + \sigma_3) + b_{\perp\perp} \cdot \sqrt{(\sigma_2 - \sigma_3)^2}}{\bar{R}_{\perp}^c} \right]^{m-1}$$

$$\text{and finally via } C = \cos(2 \cdot \Theta_{fp}) \rightarrow \Theta_{fp} = 0.5 \cdot \arccos C, \quad \Theta_{fp} \circ = \Theta_{fp} \cdot 180^\circ / \pi.$$

By insertion of any stress state the associated 'running' C can be computed.

Cohesion strength \bar{R}_{23}^r is determinable for $\tau_m^{fail}(\sigma_n) = 0$

Therewith, after effortful MathCad programming and implicit numerical computations the desired accurate bi-axial fracture stress M-C-curve $\tau_{nt}(\sigma_n)$ could be derived by the refined IFF2 model and also the results formulated in ply stresses. The fracture angle becomes now the realistic 90° instead 71° .

$$\frac{C}{\sqrt{1-C^2}} = -m \cdot \left[\frac{A}{\bar{R}_\perp^t} + \frac{2 \cdot a_{\perp\perp} \cdot f_{\text{corr}} \cdot B}{\bar{R}_\perp^c} \right] / \left[\frac{2 \cdot A}{\bar{R}_\perp^t \sqrt{(\sigma_2 - \sigma_3)^2}} + \frac{4 \cdot b_{\perp\perp} \cdot f_{\text{corr}} \cdot B \cdot \tau_{nt}}{\bar{R}_\perp^c \sqrt{(\sigma_2 - \sigma_3)^2}} \right],$$

$$B_{\text{corr}} = \left[\frac{a_{\perp\perp} \cdot (\sigma_2 + \sigma_3) + b_{\perp\perp} \cdot \sqrt{(\sigma_2 - \sigma_3)^2}}{\bar{R}_\perp^c} \cdot f_{\text{corr}} \right]^{m-1} \quad \text{with} \quad f_{\text{corr}} = 1 + c_0 \cdot (\bar{R}_\perp^c + \sigma_2)^2.$$

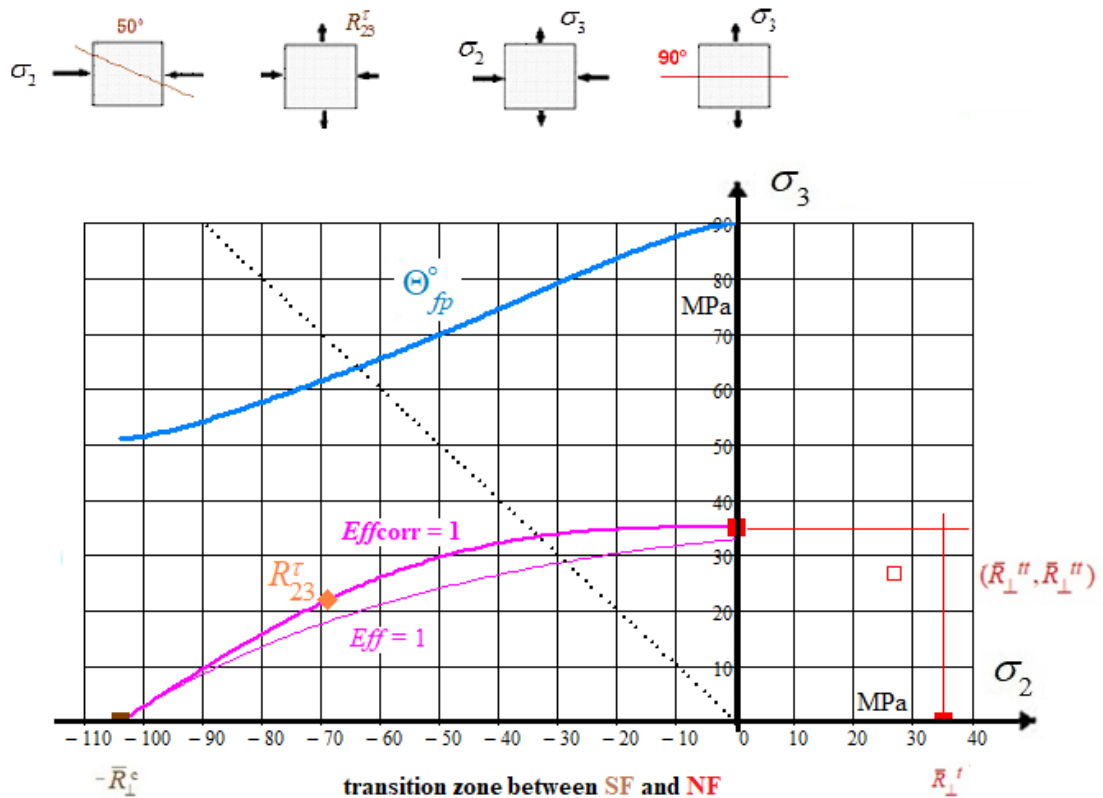


Fig. 22-4: Interaction curve $\sigma_3^{\text{fracture}}(\sigma_2)$ with $\text{Eff} = 1$ Failure stress curve $\sigma_2(\sigma_3)$ with alteration of fracture angle Θ_{fp}° in the transition zone. (Numerical example stems from a measurement of the fracture plane angle Θ_{fp}° in [VDI97, bi-axial failure stress \bar{R}_\perp^{tt}]). Marking of R_{23}^t .

- ▶ IFF2-IFF1- interacted fracture curve (thin, original IFF2. With this 'simple' approach, the curve cannot run through $\bar{R}_\perp^t = 35$ MPa)
- ▶ IFF2-IFF1- interacted fracture curve (bold, IFF2 decay function corrected, which better maps the course of measured fracture stress data) and
- ▶ Course of the fracture plane angle Θ_{fp}° (bold, corrected)

22.4: Determination of Cohesive shear Strength R_{23}^t

The **interaction curve** can be dedicated to the basic Mohr-Coulomb curve which runs from the compression strength point till the tensile strength point $\sigma_3 = \bar{R}_\perp^t$. In order to find all relationships in one diagram the Mohr stresses are also inserted as functions of the lamina stresses $\sigma_2(\sigma_3)$ and not of

σ_n alone, which is the usual diagram form. Fig.22-4 includes the development of the fracture plane angle as function of the lamina stress σ_2 . Fig.22-1 still presented all MathCad-computed Mohr entities providing:

- * Extrapolation from compressive strength point (IFF2-determined Mohr-Coulomb fracture curve)
 - A straight Linear Mohr-Coulomb curve, considering σ_2 (linear Mohr), Cohesive Strength
 - A straight Linear Mohr-Coulomb curve, considering σ_2 and σ_3 ; Cohesive Strength \bar{R}_{23}^τ
- * Full IFF2-IFF1-interacted Mohr-Coulomb fracture curve (bold, decay function- corrected)
- * Course of the fracture plane angle $\Theta_{fp}^\circ/2$ (thin, not decay function corrected) and (bold).

The definition of the cohesive (shear) strength is ($\tau_{nt}^{fail} = \bar{R}_{23}^\tau$, $\sigma_n = 0$). Searching \bar{R}_{23}^τ (C), the derived formulation permits to continuously MathCad-compute the alternating fracture plane measure C with the associate fracture angle Θ_{fp}° . The interpretation of the figures leads to the following conclusions:

- The general macro-mechanical IFF2 approach cannot offer a full accuracy of the realistically predicted Mohr-Coulomb curve. Just the *physically-based* decay function correction delivers the desired fidelity
- A SFC in lamina stresses can be transferred into a Mohr-Coulomb version
- The course of the fracture plane angle Θ_{fp}° can be determined, too
- The idea of the FMC that IFF1 and IFF2 commonly add its *Eff* portions, which leads to the result that Θ_{fp}° is in the sixty degrees $^\circ$ at the cohesive strength point \bar{R}_{23}^τ , with a degree value being the higher the higher the strength ratio $\bar{R}_\perp^c / \bar{R}_\perp^t$ is.

LL:

- Failure stress under pure shear $\tau_{23}^{fracture} = \max \tau_{23} \leq R_\perp^t$, an approach-formalistic introduced quantity
- Mohr-based approach linked so-called cohesive strength $R_{23}^\tau = \tau_{nt} (\sigma_n = 0)$
- Puck's Action plane shear resistance R_{23}^A : Puck formulated a full IFF-SFC and could model-associated dedicate his action plane resistance a relation with the inclination model parameters p and the other strengths reading $\bar{R}_{23}^A = \left[\bar{R}_{\perp\parallel} \cdot \sqrt{1 + 2 \cdot p_{\perp\parallel}^c \cdot \bar{R}_\perp^c / \bar{R}_{\perp\parallel}} - 1 \right] / 2 \cdot p_{\perp\parallel}^c$. Above quantities are not measurable ones
- Generally, assuming a transverse shear failure stress, which would be a sixth strength, will contradict material symmetry demands, which seem to require for UD materials a 'generic' number of 5, meaning 5 measurable strengths and 5 elasticity properties
- The ability for mobilizing friction processes depends on active compression stresses that cause via the friction value μ the necessary shear stress.

Analogous to the saying “If something becomes a fact it is no science anymore”.

Here transferred: **The three shear strength quantities should be no mystery anymore”.**

On the history of the Mohr-Coulomb (M-C) curve = Mohr Envelope: Otto Mohr did not commit himself to the intersection of the envelope with the σ_n -axis. A. Leon was probably the first to use an envelope, taking a parabolic one.

23 Replacing fictitious UD Model Parameters $a_{\perp\perp}, a_{\perp\parallel}$ by measurable Friction Values μ

Aim: Engineers prefer measurable friction values instead of fictitious friction parameters.

23.1: Relation of Friction parameter $a_{\perp\perp}$ to Fracture angle Θ_{fp}^c and Friction value $\mu_{\perp\perp}$

The measurement of a realistic fracture angle is practically not possible, just the determination of the friction curve parameter $a_{\perp\perp}(\mu_{\perp\perp})$ by mapping the course of test data points is a practical approach. Then, from the mapped test curve the relation of the curve parameter $a_{\perp\perp}$ to the friction value $\mu_{\perp\perp}$ and to the fracture angle Θ_{fp}^c can be derived according to the formulas in Table 23-1. This is to perform in the compressive strength point \bar{R}_{\perp}^c , see also the chapter before.

Basic assumption is the *brittle-fracture hypothesis* which goes back to O. Mohr's "The strength of a material is determined by the Mohr stresses on the fracture plane". This means for the Linear Mohr-Coulomb (M-C) formulation $\tau_{nt} = \bar{R}_{23}^c - \mu_{\perp\perp} \cdot \sigma_n$ including the friction value $\mu_{\perp\perp}$ being an intrinsic property of the UD material.

Table 23-1: Determination of the friction curve parameter $a_{\perp\perp}(\mu_{\perp\perp})$

IFF2: $F_{\perp}^c = [a_{\perp} \cdot (\sigma_2 + \sigma_3) + b_{\perp} \cdot \sqrt{(\sigma_2 - \sigma_3)^2 + 4\tau_{23}^2}] / \bar{R}_{\perp}^c = 1$, in Mohr stresses, after inserting \bar{R}_{\perp}^c

$$= [a_{\perp} \cdot (\sigma_n + \sigma_t) + b_{\perp} \cdot \sqrt{(\sigma_n - \sigma_t)^2 + 4\tau_{nt}^2}] / \bar{R}_{\perp}^c = 1 \text{ and } a_{\perp} = b_{\perp} - 1 \text{ is friction parameter}$$

$$\frac{dF}{d\sigma_n} \cdot \bar{R}_{\perp}^c = a_{\perp} + b_{\perp} \cdot (\sigma_n - \sigma_t) / \sqrt{(\sigma_n - \sigma_t)^2 + 4\tau_{nt}^2}, \quad \frac{dF}{d\tau_{nt}} \cdot \bar{R}_{\perp}^c = 4 \cdot b_{\perp} \cdot \tau_{nt} / \sqrt{(\sigma_n - \sigma_t)^2 + 4\tau_{nt}^2}$$

$$\frac{d\tau_{nt}}{d\sigma_n} = -\frac{dF}{d\sigma_n} / \frac{dF}{d\tau_{nt}} = -\left[\frac{b_{\perp} \cdot (\sigma_n - \sigma_t) + (b_{\perp} - 1) \cdot \sqrt{(\sigma_n - \sigma_t)^2 + 4\tau_{nt}^2}}{4 \cdot b_{\perp} \cdot \tau_{nt}} \right], \text{ minus due to implicit derivation}$$

Use of addition theorems ($\sigma_{\lambda} = 0$), gives the relationships $c = \cos(\Theta_{fp}^{\circ} \cdot \pi / 180^{\circ})$

$$\sigma_n - \sigma_t = c^2 \cdot (\sigma_2 - \sigma_3) - s^2 \cdot (\sigma_2 - \sigma_3) = C \cdot (\sigma_2 - \sigma_3), \quad S = \sqrt{1 - C^2}, \quad C = \cos(2 \cdot \Theta_{fp}^{\circ} \cdot \pi / 180^{\circ})$$

$$\sigma_t = \sigma_n - C \cdot (\sigma_2 - \sigma_3), \quad C = c^2 - s^2 = 2c^2 - 1 = 1 - 2s^2, \quad \sigma_n + \sigma_t = \sigma_2 + \sigma_3 \quad \text{and}$$

$$\tau_{nt} = -0.5 \cdot S \cdot (\sigma_2 - \sigma_3) = -0.5 \cdot \sqrt{1 - C^2} \cdot (\sigma_2 - \sigma_3), \quad \sigma_n = (C + 1) \cdot 0.5 \cdot \sigma_2 + (1 - C) \cdot 0.5 \cdot \sigma_3.$$

Stress σ_t has no influence, as Mohr assumed! Failure responsible due to Mohr are just τ_{nt} with σ_n !

$$\frac{d\tau_{nt}}{d\sigma_n} = -\mu_{\perp} = \frac{C}{S} = -\left[\frac{b_{\perp} \cdot (\sigma_n - \sigma_t) + (b_{\perp} - 1) \cdot \sqrt{(\sigma_n - \sigma_t)^2 + 4\tau_{nt}^2}}{4 \cdot b_{\perp} \cdot \tau_{nt}} \right] =$$

$$= -\left[\frac{b_{\perp} \cdot (C \cdot \sigma_2) + (b_{\perp} - 1) \cdot \sqrt{(C \cdot \sigma_2)^2 + 4 \cdot (-0.5 \cdot S \cdot \sigma_2)^2}}{4 \cdot b_{\perp} \cdot (-0.5 \cdot S \cdot \sigma_2)} \right]$$

$$C = -\left[\frac{b_{\perp} \cdot (C \cdot \sigma_2) + (b_{\perp} - 1) \cdot \sqrt{(C \cdot \sigma_2)^2 + 4 \cdot (-0.5 \cdot \sigma_2)^2 \cdot (1 - C^2)}}{4 \cdot b_{\perp} \cdot (-0.5 \cdot \sigma_2)} \right]; C \rightarrow C_{fp}^c \text{ inserting strength } \bar{R}_{\perp}^c$$

$$\rightarrow C_{fp}^c = -\left[\frac{b_{\perp} \cdot C_{fp}^c \cdot (-\bar{R}_{\perp}^c) + (b_{\perp} - 1) \cdot \sqrt{C_{fp}^{c2} \cdot (-\bar{R}_{\perp}^c)^2 + 4 \cdot (-0.5 \cdot (-\bar{R}_{\perp}^c))^2 \cdot (1 - C_{fp}^{c2})}}{4 \cdot b_{\perp} \cdot (-0.5 \cdot (-\bar{R}_{\perp}^c))} \right]$$

Resolving : $b_{\perp} = \frac{1}{C_{fp}^c + 1} \cong \frac{1}{1 - \mu_{\perp}}$, $a_{\perp} \cong \frac{\mu_{\perp}}{1 - \mu_{\perp}}$, $\mu_{\perp} = -\frac{C}{S} = -\frac{C_{fp}^c}{S_{fp}^c} \cong -C_{fp}^c$ (being a small value).

Assuming $\Theta_{fp}^{\circ} = 51^{\circ}$: Example $C_{fp}^c = \cos(\frac{2 \cdot \Theta_{fp}^{\circ}}{180^{\circ}} \cdot \pi) = -0.21$, $\mu_{\perp} = 0.21 \cong -C_{fp}^c$, $a_{\perp} = 0.26$.

If IFF occurs in a parallel-to-fiber plane of the UD lamina, the components of the failure stress vector are the normal Mohr stress σ_n and the two Mohr shear stresses σ_{nt} and σ_{n1} . The shear stress σ_{t1} and the normal stress σ_t will have no influence and this was proven in the derivation. Further, the Mohr stress σ_{n1} belongs to IFF3 and is not of interest, here.

The transformation of the IFF2 SFC in lamina stresses into Mohr stresses-based formulation works via above addition theorems.

During this transformation procedure there are a lot of lessons to learn:

- *The Linear Mohr-Coulomb model can be employed to obtain a sufficiently good relationship for the determination of the friction value μ in the compressive strength point $\sigma_2 = -\bar{R}_{\perp}^c$.*
- *Establishing the relationship $a_{\perp}(\mu_{\perp})$ it is assumed that the tangent of the FMC-curve has the same value as that of the straight Linear Mohr envelope curve $\tau_{nt}(\sigma_n)$ in the touch point of Mohr's circle, see [Fig.23-1](#)*
- *σ_1 is not relevant. The shear stress τ_{23} can be assumed zero because it would anyway vanish after a principal stress transformation. No reduction of generality is caused*
- *The stress σ_t has no influence! It is not representative such as Mohr supposes. Failure responsible are τ_{nt} and σ_n , only. But mind in the differentiation process: the Mohr stress σ_t*

cannot be simply set zero at the beginning of the derivations, it must be considered due to its relation to σ_n .

- Above derivation demonstrates that, if really desired, the fracture plane angle Θ_{fp}^c of an UD-material could be also determined from an invariant-based SFC and not only from Mohr-based formulations
- Viewing Fig.23-1, it is obvious that the cohesive strength \bar{R}_{23}^r (Civil engineers take the letter c) belongs to the transition zone of the normal fracture mode domain IFF1 and therefore not alone to the shear fracture mode domain IFF2. Hence, one cannot simply extrapolate from the compressive strength point.

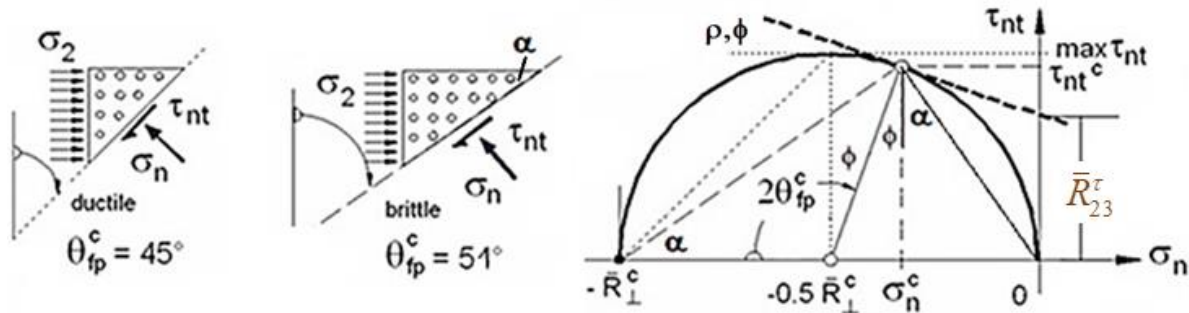


Fig.23-1, Shear stressing situation: Shear fracture plane angle in the touch point and 'linear' Mohr-Coulomb friction curve. The touch point is defined by $(\sigma_n^c, \tau_{nt}^c)$ and linked to \bar{R}_{\perp}^c

23.2: Relation of Friction parameter $a_{\perp\parallel}$ to Friction value $\mu_{\perp\parallel}$

The same procedure is analogously to perform for the mode IFF3, see [Table 23-2](#).

Table 23-2: Relationships for the determination of friction curve parameter $a_{\perp\parallel}(\mu_{\perp\parallel})$

$$F_{\perp\parallel} = \frac{I_3^2}{\bar{R}_{\perp\parallel}^4} + b_{\perp\parallel} \cdot \frac{I_2 \cdot I_3 - I_5}{\bar{R}_{\perp\parallel}^3} = 1 \quad \text{with } I_{23-5} = 2 \cdot \sigma_2 \cdot \tau_{21}^2 + 2 \cdot \sigma_3 \cdot \tau_{31}^2 + 4 \cdot \tau_{23} \tau_{31} \tau_{21} \quad \text{from}$$

$$I_2 = \sigma_2 + \sigma_3, \quad I_3 = \tau_{21}^2 + \tau_{31}^2, \quad I_5 = (\sigma_2 - \sigma_3) \cdot (\tau_{31}^2 - \tau_{21}^2) - 4 \cdot \tau_{23} \cdot \tau_{31} \cdot \tau_{21}.$$

The transfer to a Mohr-shaped SFC is directly possible, because the fracture plane is already known (parallel to the fibre direction), via $(\tau_{n1}, \sigma_n) \equiv (\tau_{21}, \sigma_2)$, $|\tau_{21}| = \bar{R}_{\perp\parallel} - \mu_{\perp\parallel} \cdot \sigma_2$

* FMC: $\frac{\tau_{21}^4}{\bar{R}_{\perp\parallel}^4} + a_{\perp\parallel} \cdot \frac{2 \cdot \sigma_2 \cdot \tau_{21}^2}{\bar{R}_{\perp\parallel}^3} = \frac{\tau_m^2}{\bar{R}_{\perp\parallel}^4} + a_{\perp\parallel} \cdot \frac{2 \cdot \sigma_n \cdot \tau_m^2}{\bar{R}_{\perp\parallel}^3} = 1 \rightarrow \sigma_n = \frac{\bar{R}_{\perp\parallel}^3 \cdot (\tau_{21}^4 / \bar{R}_{\perp\parallel}^4 - 1)}{2 \cdot \tau_m^2 \cdot a_{\perp\parallel}}$

$$\frac{d\tau_{n1}}{d\sigma_n} \rightarrow \text{simpler to perform is } \frac{d\sigma_n}{d\tau_{n1}} = \frac{2 \cdot \tau_{21}}{\bar{R}_{\perp\parallel} \cdot a_{\perp\parallel}} - \frac{\bar{R}_{\perp\parallel}^3 \cdot (\tau_{21}^4 / \bar{R}_{\perp\parallel}^4 - 1)}{\tau_{21}^3 \cdot a_{\perp\parallel}}$$

* Simple linear Mohr: $\tau_{n1} = \bar{R}_{\perp\parallel} - \mu_{\perp\parallel} \cdot \sigma_n \rightarrow \sigma_n = \frac{\bar{R}_{\perp\parallel} - \tau_{21}}{\mu_{\perp\parallel}}$ and $\frac{d\sigma_n}{d\tau_m} = \frac{-1}{\mu_{\perp\parallel}}$.

In the strength point $\tau_{n1} = \bar{R}_{\perp\parallel}$ an equal slope exists, then equating delivers

$$\frac{2 \cdot \tau_{21}}{\bar{R}_{\perp\parallel} \cdot b_{\perp\parallel}} - \frac{\bar{R}_{\perp\parallel}^3 \cdot (\tau_{21}^4 / \bar{R}_{\perp\parallel}^4 - 1)}{\tau_{21}^3 \cdot a_{\perp\parallel}} = \frac{-1}{\mu_{\perp\parallel}} \rightarrow a_{\perp\parallel} = \frac{\mu_{\perp\parallel} \cdot (\bar{R}_{\perp\parallel}^4 + \tau_{21}^4)}{\tau_{21}^3 \cdot a_{\perp\parallel}} \Rightarrow a_{\perp\parallel} = 2 \cdot \mu_{\perp\parallel}.$$

A good guess for $\mu_{\perp\parallel}$ and sufficient for application.

23.3: Evaluation of friction values $\mu_{\perp\perp}$, $\mu_{\perp\parallel}$ from test results

The determination of curve parameters $a(\mu)$ and thereby also of μ can be performed differently:

1. One strength value with one multi-axial failure stress point on the respective pure mode curves, usually applying a linear Mohr friction envelope (*sufficient, see Figs. 23-2 and -3 below, it requires some fitting to optimally map the course*)
2. A more sophisticated fitting optimization process of the test data course in the respective pure domain (*min error square*) in ‘pure’ failure mode domains
3. The so-called Tension/Compression-Torsion test machine delivered the test points in *Fig.23-2 left*. If such a test rig is not available, then, one point on the pure mode Iff2-curve plus one in the transition zone IFF2-IFF1, see *Fig.23-3*, become an approximation basis, see *Fig. 23-3 right*
4. For $\mu_{\perp\perp}$, in addition: Derivation from fracture angle measurements Θ_{fp}^c , see experience in the associate figure in [VDI 97, p. 138], facing a pretty high scatter.

The formulas for the friction values read:

- Linear Mohr envelope: $\mu_{\perp\parallel} \cong (\tau_{21}^{fr} - \bar{R}_{\perp\parallel}) / \sigma_2^{fr}$ from tension-compression/torsion test machine with tube test specimens, evaluating at least two curve points or if sufficient tests from curve fitting .
- From bi-axial compression test in order to compute the friction value from evaluating $\mu_{\perp\perp} = (\bar{R}_{\perp\perp}^c + \sigma_3^{fr}) / \sigma_2^{fr}$. However, the danger to buckle is to face
- If the test machine only allows a $\mu_{\perp\perp}$ -test in the transition zone of the modes, *Fig.23-3*, then, the estimation from strength point $(\sigma_3^{c,fr}, \sigma_2^{t,fr})$ demands for a qualified stress interaction-mapping

SFC. For the evaluation the interaction equation has to be employed, shown by the following MathCad procedure below:

Mathcad implicate calculation: *Vorgabe* $\mu_{\perp} := 0.1$ (estimation)

$$\left(\left[(\sigma_2 + \sigma_3) + \sqrt{(\sigma_2 - \sigma_3)^2 + 0} \right] / 2\bar{R}_1^t \right)^m + \left(\left[\left(\frac{\mu_{\perp}}{1 - \mu_{\perp}} \right) \cdot (\sigma_2 + \sigma_3) + \frac{1}{1 - \mu_{\perp}} \sqrt{(\sigma_2 - \sigma_3)^2 + 4\tau_{23}^2} \right] / \bar{R}_1^e \right)^m = 1 = \text{Eff} = 100\%$$

Search *Suchen* (μ_{\perp})

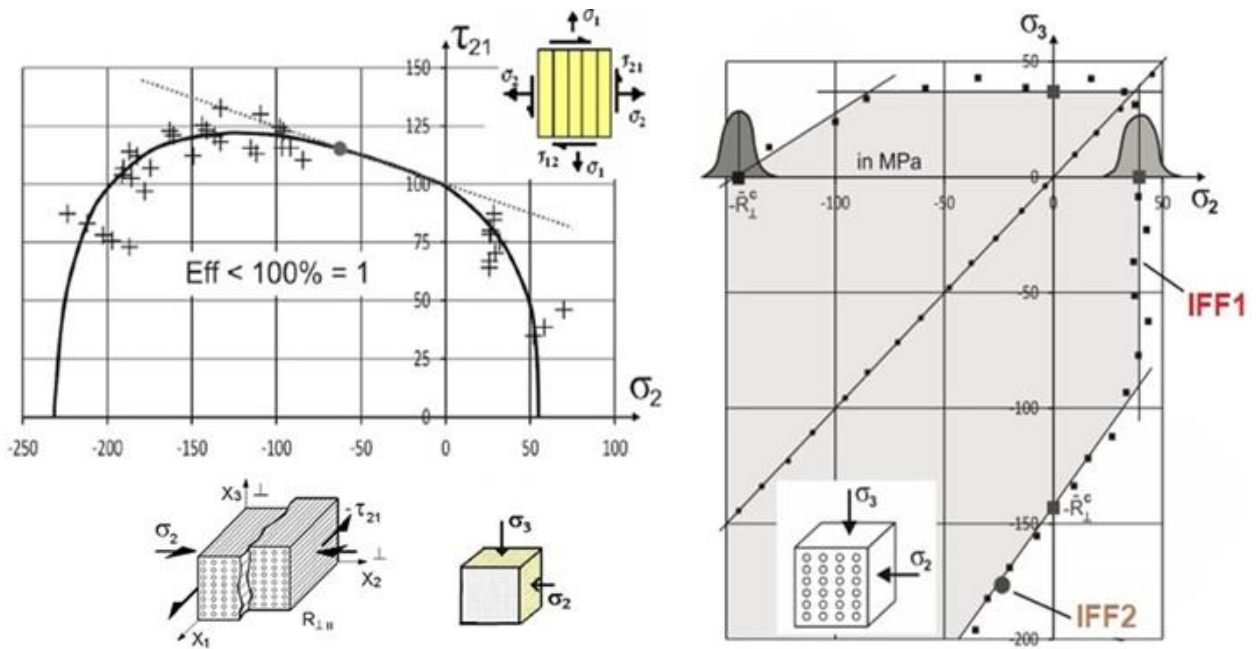


Fig.23-2: Determination of the friction values μ_{\perp} , μ_{\parallel} (own results)

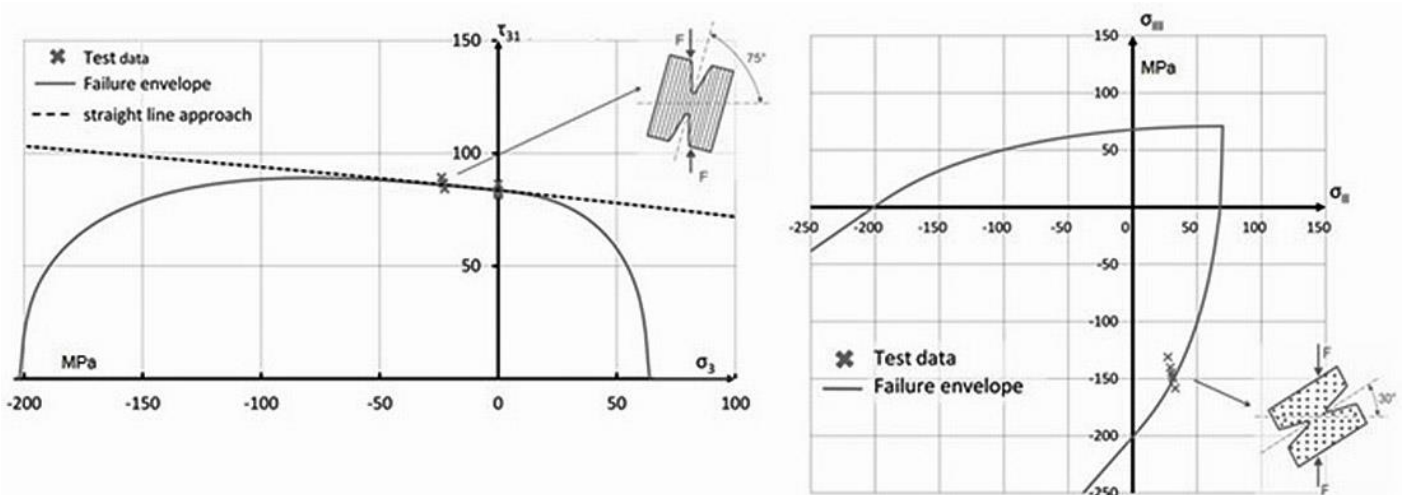


Fig.23-3: ARCAN tests performed on distinct stress paths. UD prepreg [Pet15]

LL:

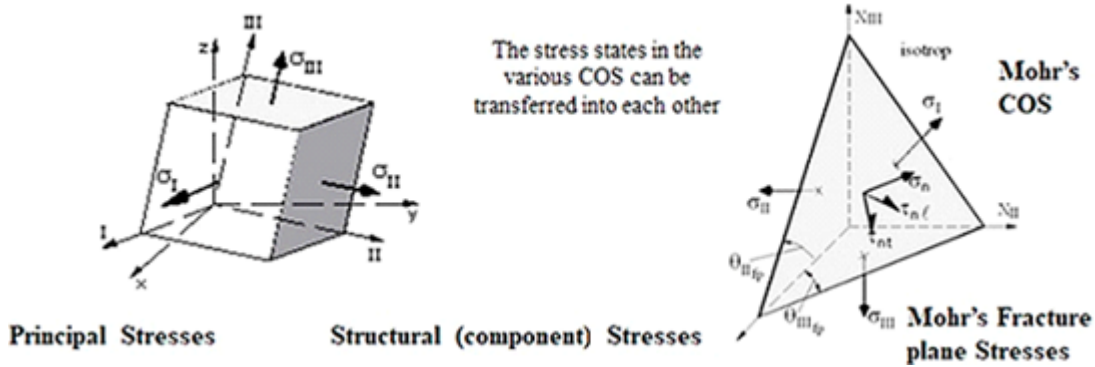
- A relationship of the measurable friction value and the fictitious friction parameter could be derived
- The application of the tension-compression-torsion test machine is recommended.

24 Fracture Bodies of Normal Concrete, UHPC and Foam

Aim: Optical clarification of the multiaxial fracture stresses.

24.1 Isotropic Fracture Body

Used Stresses and Invariants



$$\{\sigma_{\text{principal}}\} = (\sigma_I, \sigma_{II}, \sigma_{III})^T, \quad \{\sigma_{\text{struct}}\} = (\sigma_x, \sigma_y, \sigma_z, \tau_{yz}, \tau_{xz}, \tau_{xy})^T, \quad \{\sigma_{\text{Mohr}}\} = (\sigma_\lambda, \sigma_n, \sigma_t, \tau_{nt}, \tau_{\lambda t}, \tau_{n\lambda})^T$$

In the transformation of structural stresses into Mohr stresses the advantage of invariants fully comes out: *They do not depend on the coordinate system, one can simply switch between the systems.*

Structural Stresses and Invariants, 3D and 2D:

$$I_1 = (\sigma_I + \sigma_{II} + \sigma_{III}) = f(\boldsymbol{\sigma}), \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2 = f(\boldsymbol{\tau}) \text{ 'Mises invariant'}$$

$$27J_3 = (2\sigma_I - \sigma_{II} - \sigma_{III}) \cdot (2\sigma_{II} - \sigma_I - \sigma_{III}) \cdot (2\sigma_{III} - \sigma_I - \sigma_{II}),$$

$$3 \cdot \sigma_{\text{oct}} = \sigma_I + \sigma_{II} + \sigma_{III} = \sigma_t + \sigma_n + \sigma_\lambda; \quad 9 \cdot \tau_{\text{oct}}^2 = 6J_2 = 4 \cdot (\tau_{nt}^2 + \tau_{\lambda t}^2 + \tau_{n\lambda}^2), \quad \tau_{II} = \max \tau(\text{mathem.})$$

$$\sigma_I, \sigma_{II}, \sigma_{III} \text{ are principal stresses, } \sigma_I > \sigma_{II} > \sigma_{III} \text{ are mathematical stresses } (> \text{ means more positive})$$

Mohr Stresses and Invariants, 3D and 2D

$$I_1 = (\sigma_n + \sigma_t + \sigma_\lambda), \quad 6J_2 = (\sigma_n - \sigma_t)^2 + (\sigma_t - \sigma_\lambda)^2 + (\sigma_\lambda - \sigma_n)^2 + 6 \cdot (\tau_{nt}^2 + \tau_{n\lambda}^2 + \tau_{t\lambda}^2), \text{ isotropic } \tau_{nt} \rightarrow \tau_n$$

Strength Failure Criteria (SFC), Eff-linked

At first the 'basic' formulations are displayed. Then, according to the 'proportional (stressing) concept' the relationships *Eff* (*F*) are performed. And finally, how the two shear mode parameters depend on another after having inserted R^c into F^c .

If mode-interaction occurs the SFC *F* is to be replaced by the associated *Eff* in order to enable the interaction of modes in the mode transition zone. Mind: The cohesive strength is located in the transition zone between the two modes.

Table 24-1 summarizes the Eff^{mode} formulations for the usually as rotationally-symmetric assumed fracture failure body, and further the realistic isotropic 120°-rotational symmetry relations.

$$FNF = F_{\sigma} = \frac{\sqrt{4 \cdot J_2 - \frac{I_1^2}{3} + I_1}}{2 \cdot R_t} = Eff_{\sigma} = 1$$

$$FSF = F_{\tau} = c_2 \cdot \frac{I_1}{R_c} + \frac{c_1 \cdot 6J_2}{2 \cdot R_c^2} = 1$$

$$Eff_{\sigma} = \frac{\sqrt{4 \cdot J_2 - \frac{I_1^2}{3} + I_1}}{2 \cdot R_t}$$

$$c_2 \cdot \frac{I_1}{R_c \cdot Eff_{\tau}} + \frac{c_1 \cdot 6J_2}{2 \cdot R_c^2 \cdot Eff_{\tau}^2} = 1$$

$$Eff_{\tau} = \frac{\sqrt{I_1^2 \cdot c_2^2 + 12 \cdot J_2 \cdot c_1 + I_1 \cdot c_2}}{2 \cdot R_c}$$

due to homogeneous F

Insertion of the compressive strength delivers
delivers a parameter-relation : $c_2 \cdot \frac{-R_c}{R_c} + \frac{c_1 \cdot 2 \cdot R_c^2}{2 \cdot R_c^2} = 1$ $c_1 = c_2 + 1$

Monotonic stressing of all stresses

Interaction requires to go from F to Eff , linked due to the 'proportional stressing concept'

for instance
$$c_2^{SF} \cdot \frac{I_1 / Eff}{\bar{R}^c} + c_1^{SF} \cdot \frac{6 \cdot J_2 / Eff^2}{2 \cdot \bar{R}^{c^2}} = 1.$$

Fig.24-0: Some basic relationships

Procedure how to determine the Fracture Body

(1) Fracture failure body is rotationally symmetric (like the Mises yield body)

* Normal Fracture NF, $I_1 > 0$

↔

* Shear Fracture SF, $I_1 < 0$

$$F^{NF} = F^{\sigma} = \frac{\sqrt{4J_2 - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t}$$

$$F^{SF} = F^{\tau} = c_2^{SF} \cdot \frac{I_1}{\bar{R}^c} + c_1^{SF} \cdot \frac{6 \cdot J_2}{2 \cdot \bar{R}^{c^2}}$$

with σ and τ as failure driving stresses. Resistances \bar{R} are average values (We model !).

Strength Failure Criterion (SFC), mode interaction exponent m , friction μ

$$Eff^{NF} = \frac{\sqrt{4J_2 - I_1^2 / 3 + I_1}}{2 \cdot R^t} = \frac{\sigma_{eq}^{NF}}{\bar{R}^t} \leftrightarrow Eff^{SF} = \frac{c_2^{SF} \cdot I_1 + \sqrt{(c_2^{SF} \cdot I_1)^2 + 12 \cdot c_1^{SF} \cdot 3J_2}}{2 \cdot R^c} = \frac{\sigma_{eq}^{SF}}{\bar{R}^c}.$$

with $c_1^{SF} = 1 + c_2^{SF}$, $c_2^{SF} = (1 + 3 \cdot \mu) / (1 - 3 \cdot \mu)$ from $\mu = \cos(2 \cdot \theta_{fp}^c \circ \pi / 180)$

or from fitting of the test data course.

(2) Fracture failure body is 120°-rotationally symmetric = Reality !

In a chapter before we had to learn: Each isotropic material is "120° – rot. symmetric",

which leads to the little more complicate Eff_s below

$$Eff_{\Theta}^{NF} = c^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3 + I_1}}{2 \cdot R^t} \leftrightarrow Eff_{\Theta}^{SF} = \frac{c_2^{SF} \cdot I_1 + \sqrt{(c_2^{SF} \cdot I_1)^2 \cdot \Theta^{SF} + 12 \cdot c_1^{SF} \cdot 3J_2}}{2 \cdot R^c}$$

$c_{1\Theta}^{SF} \cdot \Theta^{SF} = 1 + c_2^{SF}$, and above friction parameter c_2^{SF}

Modelling of the cap is performed by the function $y_{cap} = \frac{I_1}{\sqrt{3} \cdot \bar{R}^t} = s^{cap} \cdot \left(\frac{\sqrt{2J_2 \cdot \Theta^{NF}}}{\bar{R}^t} \right)^2 + \frac{\max I_1}{\sqrt{3} \cdot \bar{R}^t}.$

For $R^c \gg R^t$ can be set : $\Theta^{NF} \rightarrow \Theta^{TM} = \sqrt[3]{1 + d^{NF} \cdot (+1)}$, $c^{NF} = \Theta^{NF} = 1.$

Θ as non-circularity function with d as non-circularity parameter, ($d^{SF} = d\tau$)

$$\Theta^{SF} = \sqrt[3]{1 + d^{SF} \cdot \sin(3\vartheta)} = \sqrt[3]{1 + d^{SF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}}, \text{ compr. angle } -30^\circ \rightarrow \Theta^{SF} = \sqrt[3]{1 + d^{SF} \cdot (-1)}.$$

The tests are performed by adding an axial load, generating a stress σ_{ax} , upon a hydrostatic loading p_{hyd} . The test data sets have been forwarded by Dr.-Ing. Silke Scheerer and Dr.-Ing. Kerstin Speck (IfM, TU-Dresden, Prof. Dr. M. Curbach). From their provided raw data sets as sub-sets the meridian data sets, the constant Lode angles for the envisaged meridians had to be extracted by the author. The usual tests are run along the tensile meridian (TM) and the compressive meridian (CM). This situation causes to apply the realistic isotropic 120°-rotationally-symmetric model in order to account for the Lode angle ϑ .

Determination of the model parameters in the mode domain of F_τ : The measurement of Θ_{fp} – based on the usually small-scale test level - is practically not possible. The determination of the curve parameters c_i by mapping the course of test data points is the better and practical procedure. Then, the relationship of the curve parameter c_2 to the friction value μ and to the fracture angle Θ_{fp} can be derived. These relations are obtained in the *touch point*, pointed out in Fig.22-1.

I Visualization of 3D compression test data: Normal Concrete

In Fig. 24-1, left, the course of test data is mapped. As coordinates, the Lode-Haigh-Westergaard coordinates are used which equally count in all directions of the 3D stress space (for understanding see Fig.14-9). The tensile strength is used for normalization in the case of brittle materials.

The right part figure displays the fracture failure body, on which the 3 main meridians are depicted. For the tensile meridian a Lode angle $\vartheta = +30^\circ$ is valid and for the compressive meridian $\vartheta = -30^\circ$. The shear meridian was chosen by the author as neutral meridian with the Lode angle $\vartheta = 0$. For each mode, the SFC model parameters must be determined in each associated ‘pure’ failure mode domain. In this context physics of slightly porous isotropic materials is to remember: *bi-axial tension = weakest link failure behavior ($R^{tt} < R^t$, which partly seems to be not accepted in civil engineering) and * bi-axial compression = redundant (benign) failure behavior ($R^{cc} > R^c$).

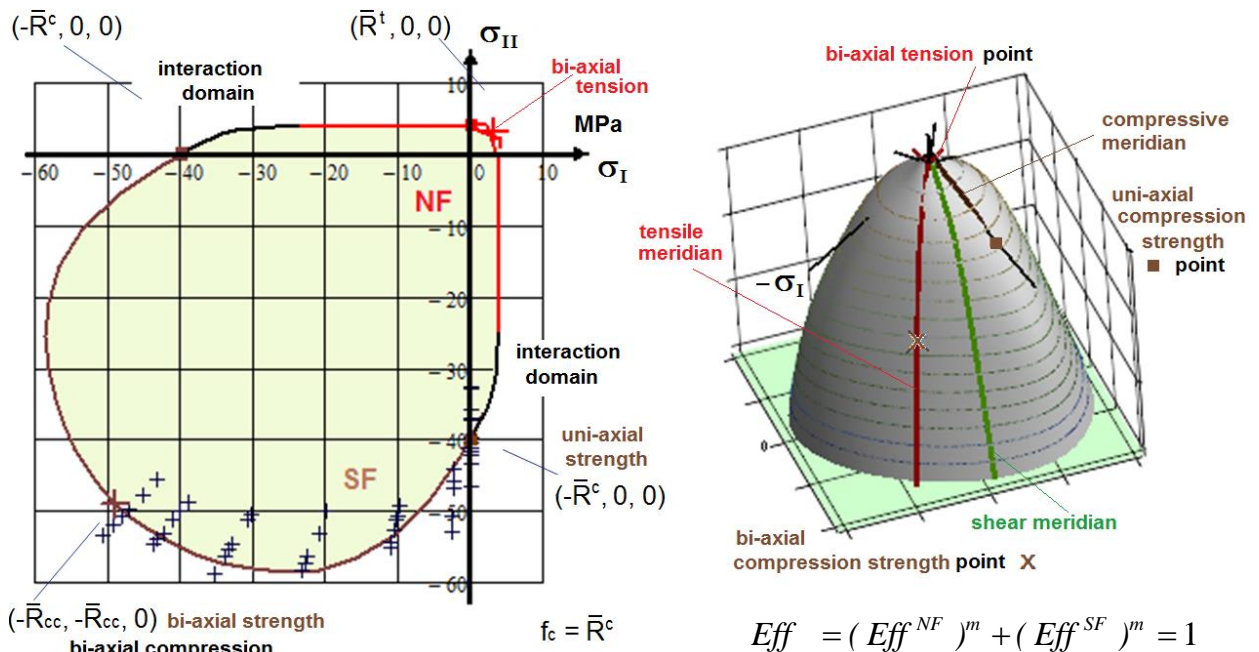


Fig.24-1, Normal Concrete: mapping of 2D-test data in the Principal Stress Plane as the bias cross-section of the fracture body. R = strength, t =tensile, c =compressive; bar over means average (mean) value. $\mu = 0.2$. (test data, courtesy: IfM Dresden,) R^{cc}

Table 24-2, Normal Concrete: Data set

$$F^{NF} = F^\sigma = c^{NF} \cdot \frac{\sqrt{4J_2 - I_1^2/3} + I_1}{2 \cdot \bar{R}^t}, \quad F^{SF} = F^\tau = c_{1\Theta}^{SF} \cdot \frac{6J_2 \cdot \Theta^{SF}}{2 \cdot \bar{R}^{c2}} + c_2(\mu) \cdot \frac{I_1}{\bar{R}^c} = 1$$

Normalizing here with \bar{R}^t and \bar{R}^c . Dent too small to be practically of interest $\rightarrow c^{NF} = 1$.

3 remaining unknowns at least require 3 fix stress state failure points on the failure surface

$$\{\sigma_{fr}\} = (\sigma_1, \sigma_2, \sigma_3)^T : (-\bar{R}^c, 0, 0), (-\bar{R}^{cc}, -\bar{R}^{cc}, 0, 0), (\sigma_1^{frict}, \sigma_1^{frict}, 0), m = 2.9,$$

with the values in MPa = N/mm²: $\bar{R}^t = 4$, $\bar{R}^c = 44$, $\bar{R}^{cc} = 49$, $\sigma_1^{frict} = -20$, $\sigma_{II}^{frict} = -56$

$$c_{1\Theta}^{SF} \cdot \Theta^{CM} = 1 + c_2 \text{ with } c_2 = 1.07 \text{ as friction parameter } (\Theta_{fp}^c = 51^\circ), \vartheta = + - 30^\circ$$

$$c_{1\Theta}^{SF} = 2.45, d^{SF} = 0.39, \Theta^{SF} = \sqrt[3]{1 + d^{SF} \cdot \sin(3\vartheta)} \rightarrow \Theta^{TM} = 0.49, \Theta^{CM} = -0.49.$$

For the determination of the closing cap and the open bottom:

$$\bar{R}^{III} = 3.6 \rightarrow \text{closing cap point } \max I_1 = 3 \cdot \bar{R}^{III}, \bar{R}^{ccc} = 1000 \text{ MPa (set for computation).}$$

With Awaji-Sato the non measurable \bar{R}^{III} is estimated: $\bar{R}^{III} = \bar{R}^t / 3^{1/M}$, $M = \ln(2) / \ln(\bar{R}^t / \bar{R}^{II})$.

$$\bar{R}^{II} = 0.9 \cdot \bar{R}^t \text{ (assumed)}, s^{cap} = -0.57.$$

Fig.24-2 through 24-4 present a hoop cross-section (octahedral stress plane or so-called π -plane), two axial cross-sections, the meridians of the failure body, and two views of the failure body.

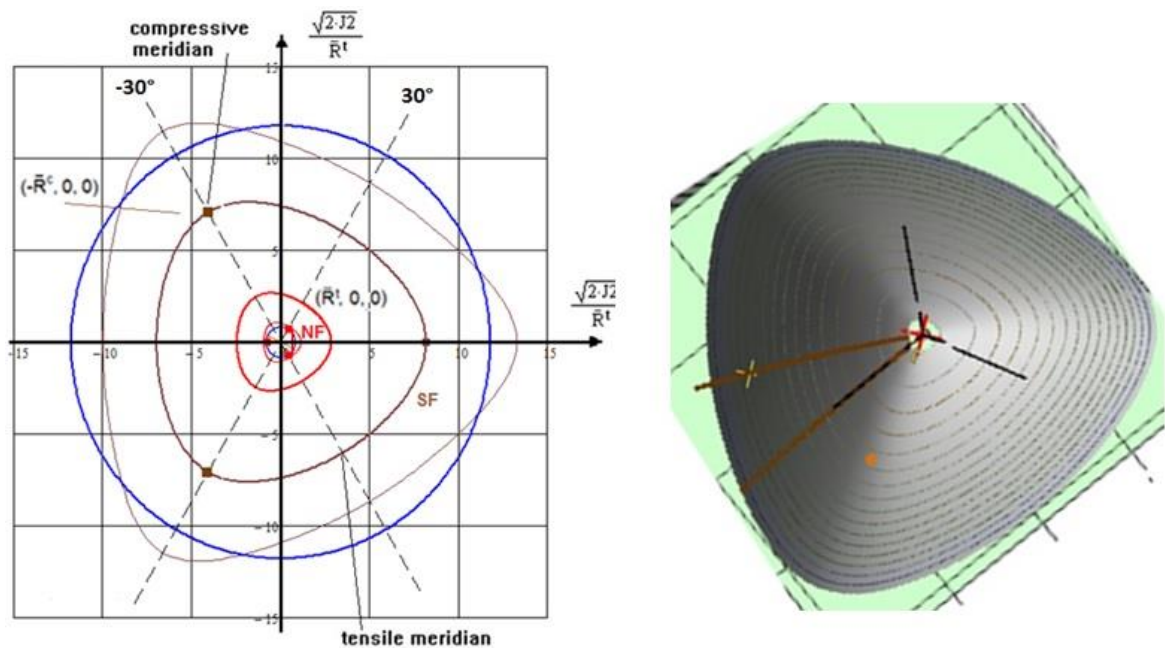


Fig.24-2, Normal Concrete: Top view: Octahedral stress plane (π -plane) exhibiting the constant Eff- lines on the body (the blue line refers to $I_1 = 0$). Right: X R^{cc} , ● R^c .

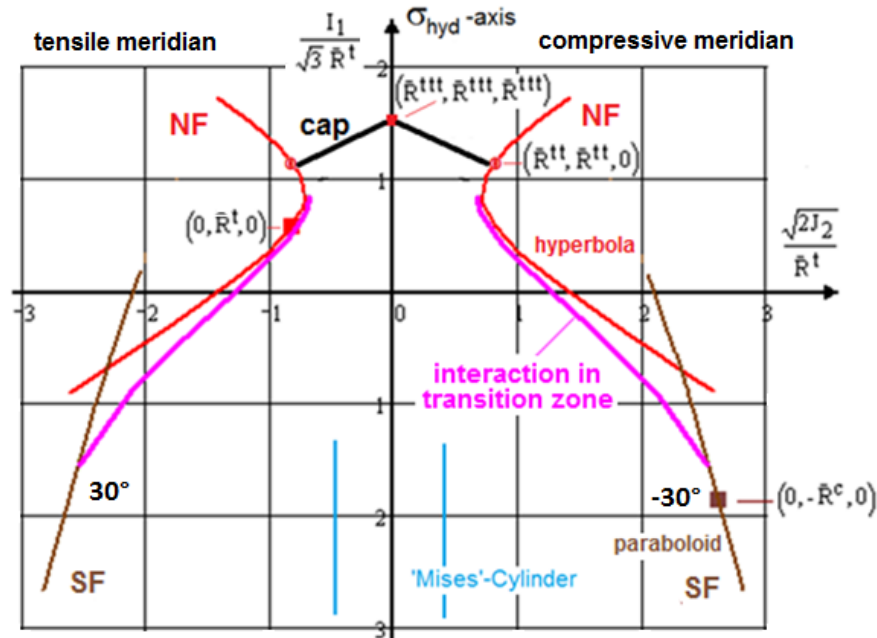
One recognizes that with increasing negative I_1 the hoop shape becomes more and more circular.

In Fig.24-3 the modeling of cap, NF domain (marginal) and of the SF fracture domain is depicted.

Fig.24-4 shows the three basic meridians and two strength points, compressive strength (dot) and bi-axial compressive strength (cross).

Fig.24-5 informs about the test data scatter of the 3D fracture states experienced under hydrostatic pressure when running test on the tensile meridian and on the compressive meridian (-30°), selection of test data performed.

The Neutral Meridian is **dashed**.



*Fig. 24-3, axial cut: Visualization of the courses of the 2 mode mapping functions for **NF** and **SF** along the meridian cross sections of the fracture body (180° cut of the 120°-body) and after **interaction***

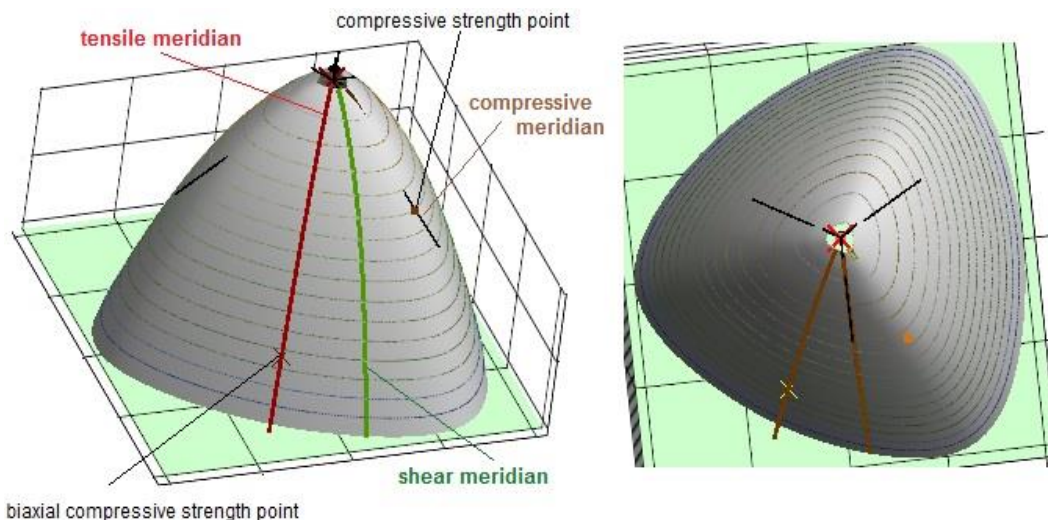


Fig.24-4: Two views of the 120°-rotationally-symmetric fracture body (hoop cross-section) of Normal Concrete with the basic three meridians and the strength points [Cun17]

In Fig.24-6 the meridian failure curves are depicted and CM test points are inserted indicating where the determination of the Mohr quantities τ_n , σ_n , Θ_{fp} has been performed. As coordinates, the Haigh-Lode-Westergaard coordinates are used which equally count in all directions of the space.

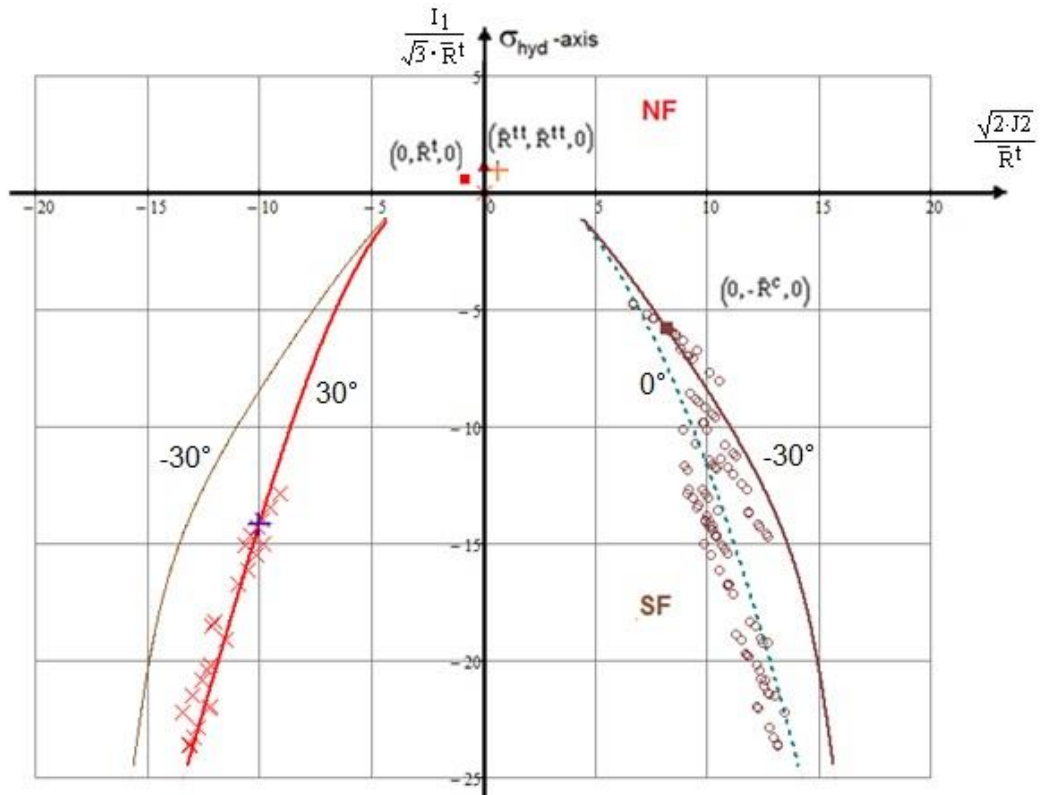


Fig.24-5: (left) Tensile meridian curve (TM) and associated test data (x, 30°), (right) compressive meridian (-30°) curve (CM) and test data on the respective hoop ring o (these circles o are located at different meridian angles ϑ), courtesy IfM Dresden. $\alpha\tau = , \beta\tau =$

Extrapolated guess of the CM-curve on basis of mapped TM test data and vice versa:

Just replace the Lode angle part for 30°, $\sin(3\vartheta) = 1$, by that for -30°, $\sin(3\vartheta) = -1$.

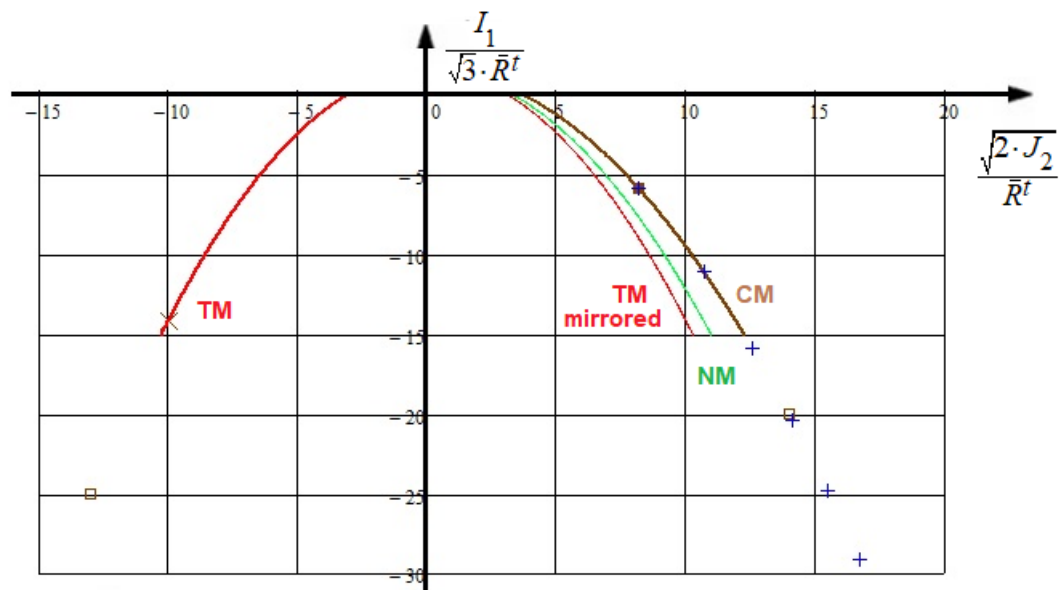


Fig.24-6: Display of all basic meridians of Normal Concrete. The + are the points where the evaluation of $\tau_{nb}, \sigma_n, \Theta_{fp}$ was performed. $p = p_{hyd}$. TM Tensile Meridian, CM Compressive Meridian, NM Normal Meridian. (Mathcad unfortunately did not draw below -15, an often faced Mathcad problem)

The above depicted basic three meridians are: Tensile Meridian TM ($\vartheta = +30^\circ$) inside, Neutral Meridian NM (0°) and Compressive Meridian CM ($\vartheta = -30^\circ$), outside. Test points lie on the respective meridian, determined by ϑ , which means on different distances from the axis for a specific $I_1 / \sqrt{3 \cdot \bar{R}^t}$. For Normal Concrete, *Fig.24-6* significantly supports the existence of the 120° -rotational symmetry of brittle (and ductile) isotropic materials.

III Visualization of 3D compression test data: UltraHighPerformanceConcrete (UHPC)

Fig.24-7, left depicts the separated and later intensively investigated TM and CM test data points.

Fig.24-7, right presents all 3D test points located at different Lode angles.

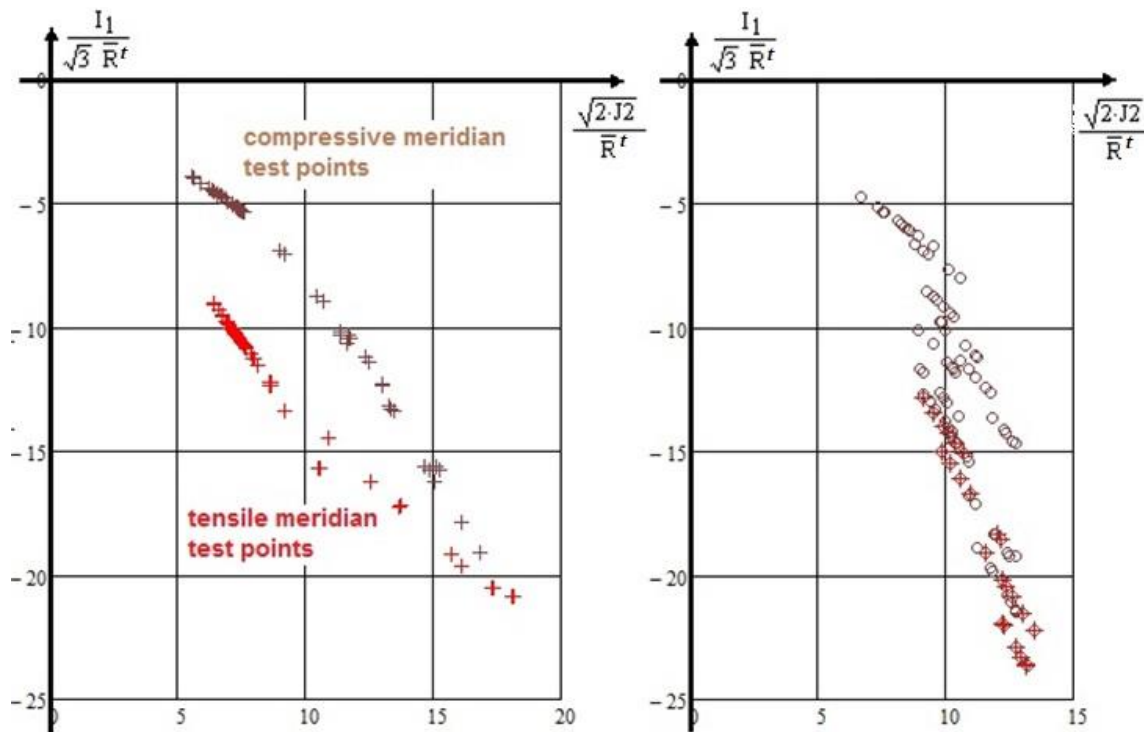


Fig.24-7 UHPC: Compressive and tensile meridian points

UHPC, separated test points: (left) tensile meridian + ($\vartheta = +30^\circ$) and compressive meridian + ($\vartheta = -30^\circ$); (right) all 3D test points are marked by o (hoop ring, ($\vartheta \neq +, - 30^\circ$)), visualizing to be located at different meridians

Fig.24-8 outlines modelling ideas for UHPC.

As could be still recognized for Normal Concrete, the failure body possesses inward dents for $I_1 > 0$ and outward dents for $I_1 < 0$ in contrast to porous concrete stone, where it is also inward, see *Fig.24-9*. These dents become smaller with increasing $|I_1|$.

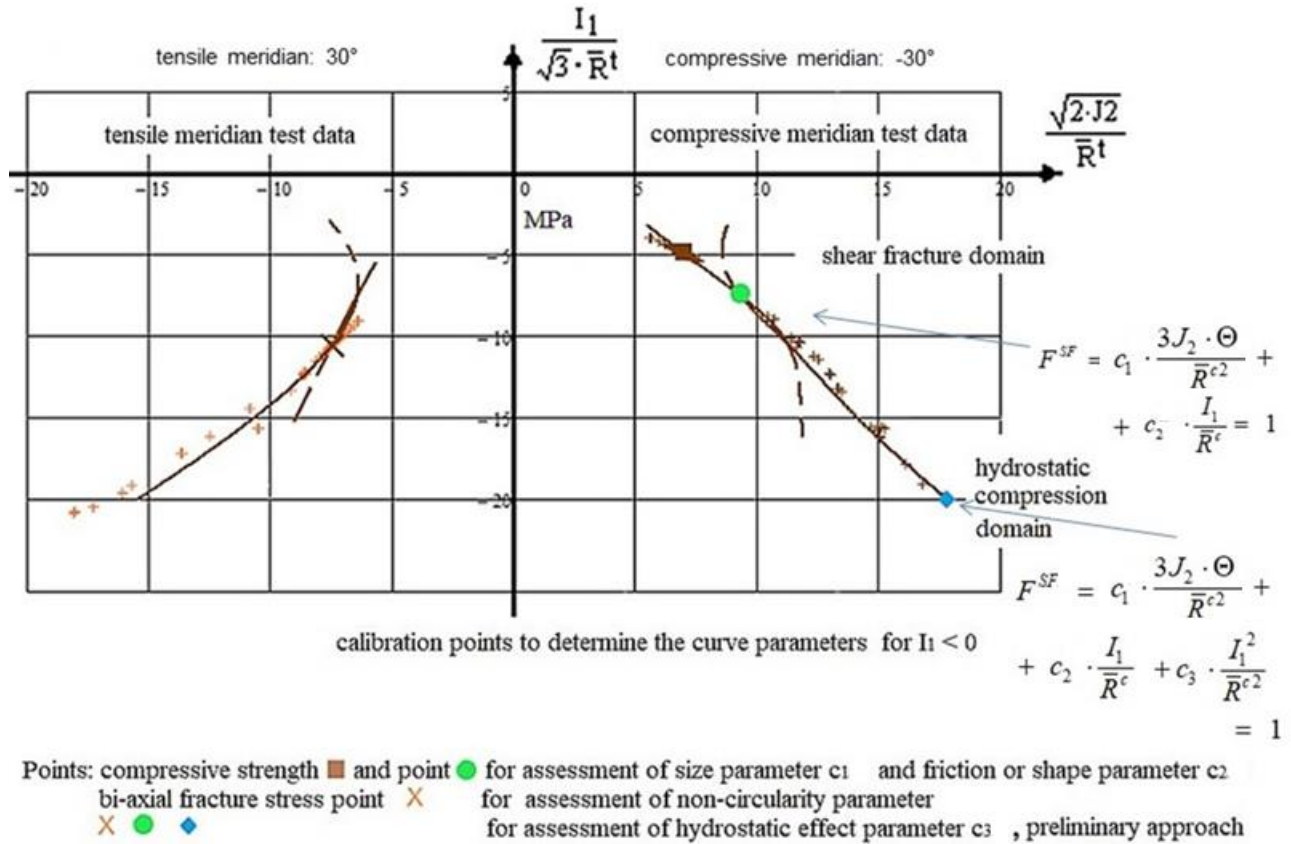


Fig.24-8 Ultra-High-Performance-Concrete (UHPC):

[Test data: Dr. Speck, IfM, TU-Dresden]. From this general data set as sub-sets the meridian data sets (constant Lode angles) have been extracted by the author

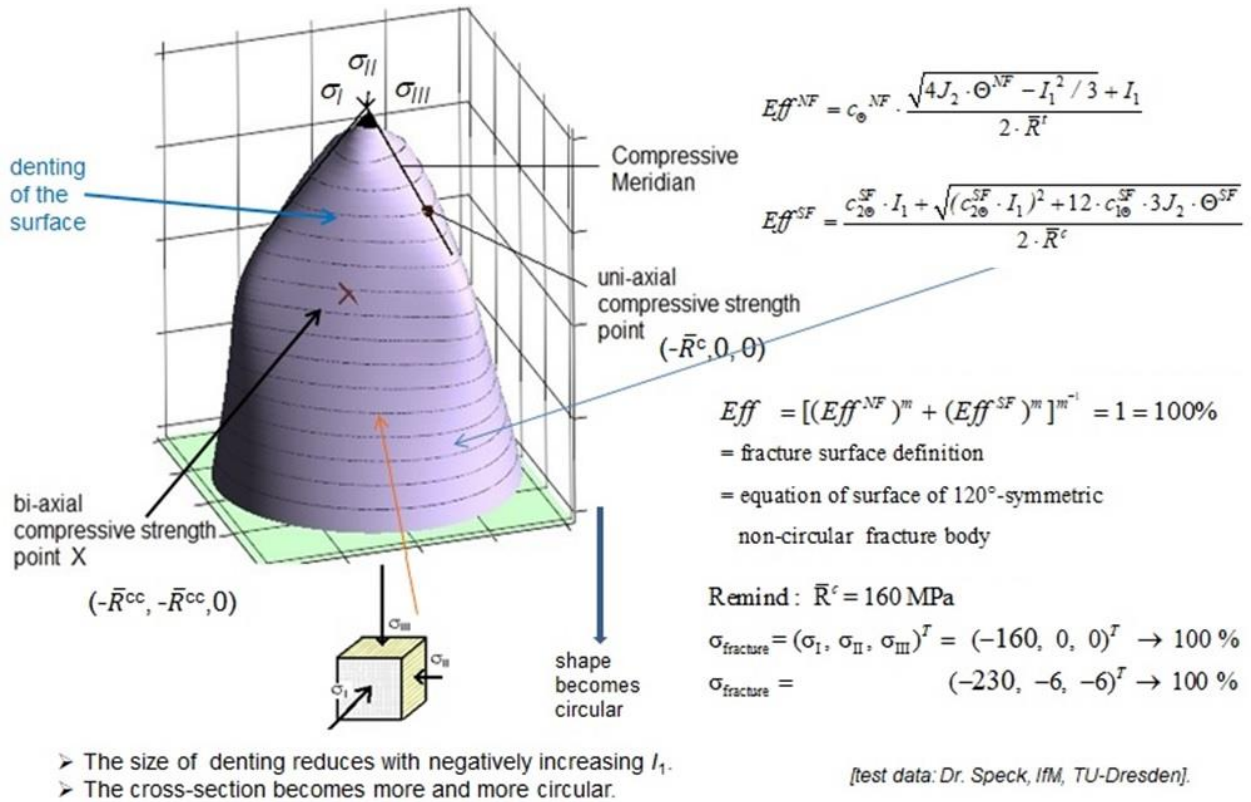
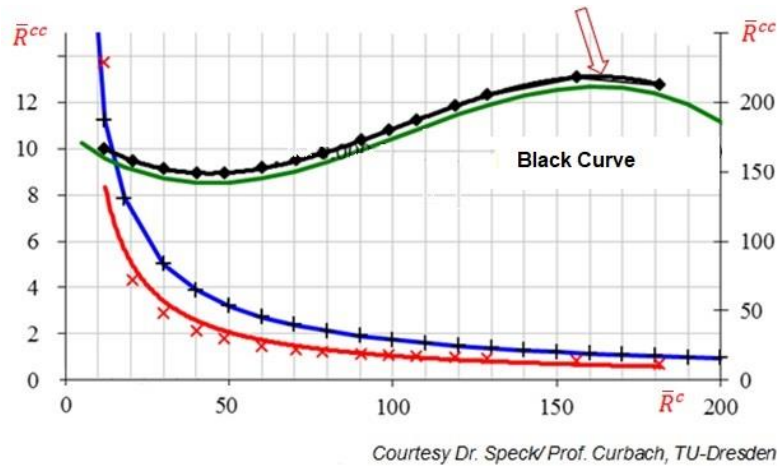


Fig.24-9, UHPC: Fracture body showing decay of denting with a negative I_1

Fig.24-10 shows a graph bi-axial compressive strength over uniaxial compressive strength. It turns out that with increasing uniaxial strength the bi-axial strength approximates the uniaxial strength. The author tries to explain this: The effect of redundancy under hydrostatic loading can be interpreted as an out-smoothing of stress concentrations. In the case of Normal Concrete this effect becomes more chances according to being more roughly grained than UHPC. This explains why the bi-axial strength capacity increase of a roughly grained Normal Concrete is higher than for UHPC.



*Fig.24-10: compressive strength capacity ratio of concrete \bar{R}^{cc} / \bar{R}^c ($R = f$),
 \bar{R}^{cc} / \bar{R}^c (Normal Concrete) $>$ \bar{R}^{cc} / \bar{R}^c (UHPC)*

In *Fig.24-11* are depicted the 2 mode domains and its transition zone obtained with the interaction formula. This task concentrated about performing an interaction in the principal plane $I_1 > -2R^{cc}$.

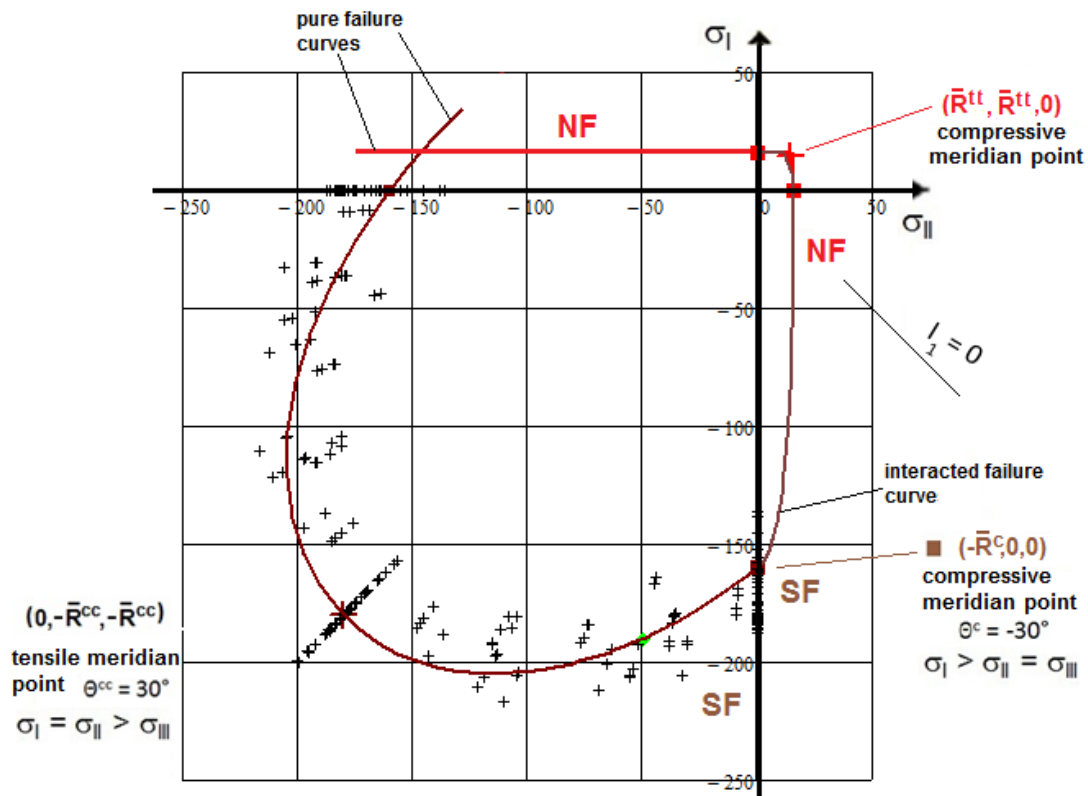


Fig.24-11, UHPC: Principal stress plane with measured test data and evaluated strength points

3D-Visualization of 2D compression test data: Porous Foam Rohacell 71 IG

With the Rohacell Hero (Evonik) a PMI (Poly-Methacryl-Imide) structural foam of an increased tensile fracture strain a light material is available which may replace the expensive honeycombs. Given is ‘only’ a 2D-Test Data Set and therefore just a realistic mapping in the Principal Stress Plane is possible data set used (*thanks to Dr. Kolupaev for the test data set*) reads:

$$\bar{R}^t = 1.8; \bar{R}^{tt} = 1.25; \bar{R}^{ttt} = 1.01; \bar{R}^c = 1.65; \bar{R}^{cc} = 1.4; \bar{R}^{ccc} = 1.53, \max I_1 = 3.03;$$

$$\min I_1 = -4.58, d^{NF} = -0.71; d^{CrF} = 0.21; c^{CrF} = 1.03, s^{cap} = -0.27; s^{bot} = 0.87,$$

$$g^{NF} = -0.57; g^{CrF} = 0.52; \Theta^{NF} = 1.2; \Theta^{CrF} = 1.07, m = 2.5.$$

The Figs. 24-12 and 24-13 show the application of the respective SFC for Rohacell.

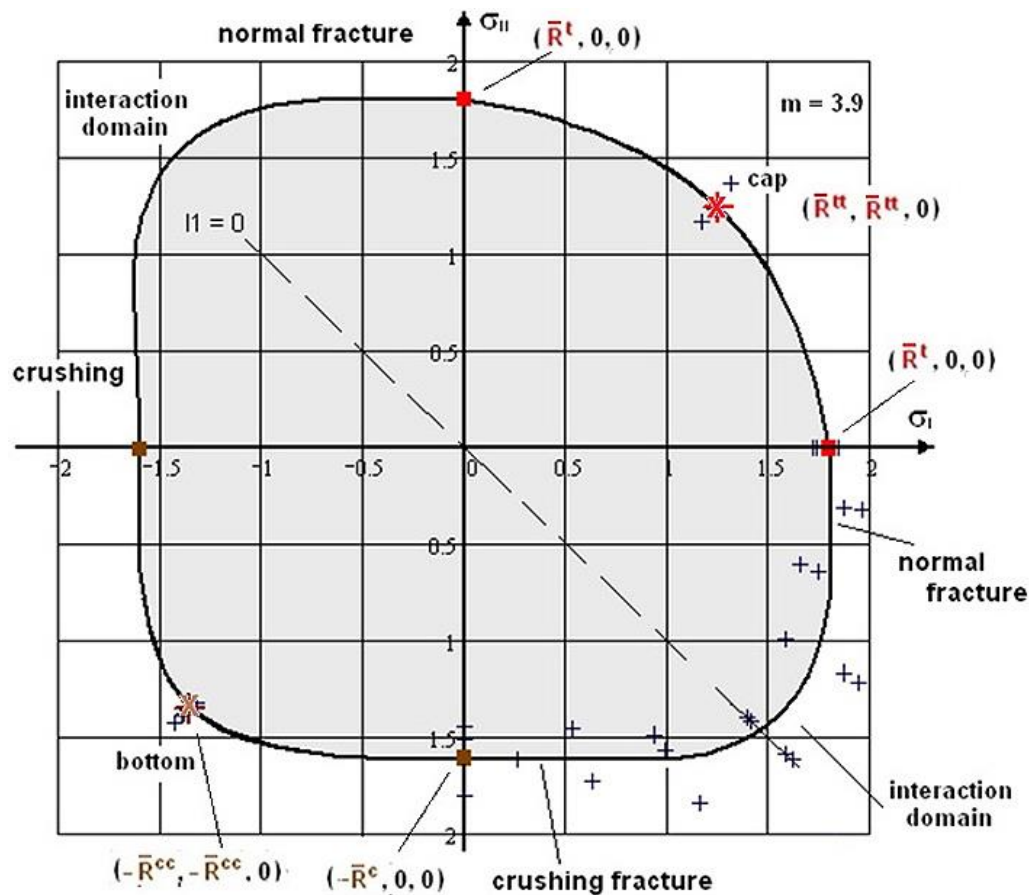


Fig.24-12, Foam Rohacell 71 IG: Mapping of 2D-test data in the Principal Stress Plane.

MathCad plot [test data: courtesy V. Kolupaev, LBF Darmstadt]

LL:

- * 120°-rotational symmetry is inherent for isotropic materials
- * R^c lies on the CM, R^t on the TM and R^t in the transition zone between the two modes F^o and F^t . This indicates, that an estimation of R^t , obtained by just an extrapolation from R^c , will be questionable
- * The failure body possesses inward dents for $I_1 > 0$ and outward dents for $I_1 < 0$ in contrast to porous concrete stone, where it is also inward. These dents become smaller with increasing $|I_1|$.
- * There is a pretty large scatter of the compressive strength data in the 2D-figure

- * Mapping of the course of test data with the SFCs worked very well
 - * Fracture body shows a decay of denting with increasing negative I_1
 - * The higher the strength ratio $SR = R^c/R^t$ becomes, the more the Cohesive Strength value narrows R^t !
 - * The strong influence of IFF1 is fully demonstrated.
- An extrapolation from the compression strength - just applying F^c - cannot be accurate !
- * A smaller μ value is more conservative.

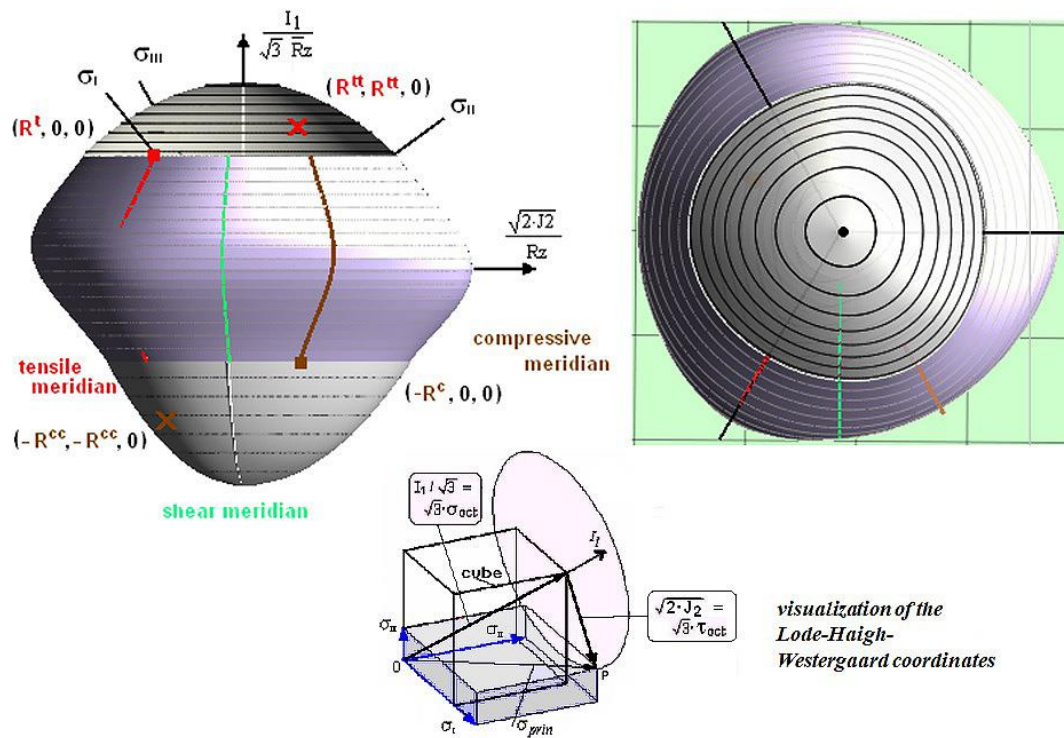


Fig.24-13, Rohacell 71 IG: Fracture body with its different meridians (left) and view from top (right)

25 Accurate Mohr-Coulomb Curve and Cohesive shear Strength \bar{R}^T of Isotropic materials

Aim: Enabling the correct understanding of the cohesive strength value as a bi-axial fracture quantity.

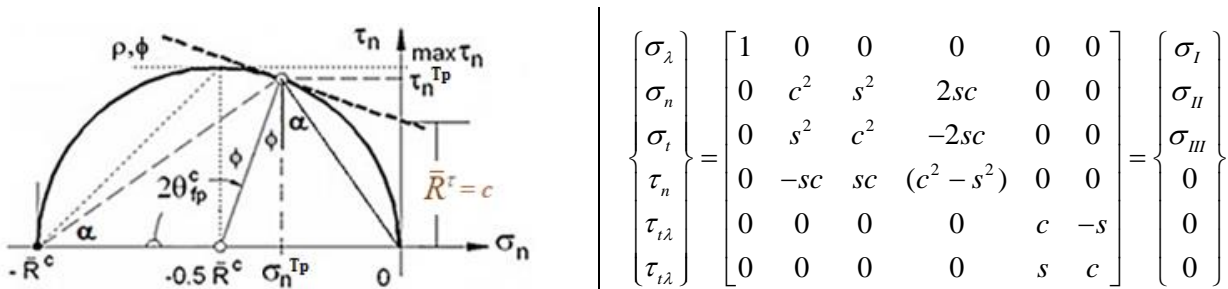
25.1 Accurate Mohr-Coulomb Curve and Cohesive shear Strength \bar{R}^T

As the author had to design with all the three basic material families isotropic, transversely-isotropic and orthotropic for him a conflict comes up, if the used index-letters are not material-dedicated, not self-explaining and not generally used in mechanics. This caused him as civil engineer to publish a Glossar.

In order to not disturb the co-working engineering family in construction the fiber-reinforced polymer matrix-linked terminology (*world-wide applying the suffixes 1, 2*) should be also used with fiber-reinforced mineral matrix-linked Carbon Concrete (*another field of the author*). The following analogous letters will be intentionally proposed to use $c \rightarrow \bar{R}^T$, $\sigma_1 \rightarrow \sigma_{II}$.

As some researchers in construction still began, when investigating Mohr-Coulomb friction, according to general mechanics they attribute usually positive marked compressive stresses a negative sign. Hence, the positive direction is to display rightward, *Fig. 25-1 (left)*. (Historically, civil engineers basically were more faced by compression and mechanical engineers by tension. This explains the different sign choice).

Fig.25-1 outlines the Mohr entities together with the transformation matrix for transforming principal stresses into Mohr stresses.



With $\eta = \sigma_{II} - \sigma_{III}$ and $\sigma_\lambda, \tau_{\iota\lambda}, \tau_{n\lambda} = 0$, the used addition theorems read: $\tau_{nt} \equiv \tau_n$

$$\sigma_n - \sigma_t = c^2 \cdot \eta - s^2 \cdot \eta = C \cdot \eta, \quad S = \sqrt{1 - C^2}, \quad C = c^2 - s^2 = 2c^2 - 1 = 1 - 2s^2,$$

$$\sigma_n + \sigma_t = \sigma_{II} + \sigma_{III} = I_1, \quad \sigma_t = \sigma_n - C \cdot \eta,$$

$$\tau_n = -s \cdot c \cdot \sigma_{II} + s \cdot c \cdot \sigma_{III} = -0.5 \cdot \sqrt{1 - C^2} \cdot \eta, \quad c = \cos \phi, \quad s = \sin \phi,$$

$$\sigma_n = c^2 \cdot \sigma_{II} + s^2 \cdot \sigma_{III} = \frac{(C+1) \cdot \sigma_{II} + (1-C) \cdot \sigma_{III}}{2}.$$

Fig. 25-1: Transformation of Principal Structural Stresses into Mohr Stresses and helpful Addition Theorems

Assumption of Otto Mohr

His basic assumption was: “The strength of a material is determined by the (Mohr) stresses on the fracture plane”. This means for the linear Mohr-Coulomb (M-C) formulation $\tau_n = \bar{R}^T - \mu \cdot \sigma_n$. Herein, the value μ is the intrinsic friction value of the material and \bar{R}^T the so-called cohesion strength. The other two shear stresses $\tau_{\iota\lambda}, \tau_{n\lambda}$, *Fig.25-1, right*), are zero. The normal stress σ_t must be accounted for in the transformation process but will finally have no influence, which has to be proven when

following Mohr and this must be proven. According to Mohr, the stresses σ_n and τ_n are the only fracture-responsible stresses, the normal stress σ_λ can be set zero. **Bild irgendwo**

SFCs regarding the 120°-rotational symmetry of the isotropic fracture body

$$F^{NF} = \frac{\sqrt{4J_2 - I_1^2 / 3 + I_1}}{2 \cdot \bar{R}^t}, \quad F^{SF} = c_2^{SF} \cdot \frac{I_1}{\bar{R}^c} + c_1^{SF} \cdot \frac{6 \cdot J_2}{2 \cdot \bar{R}^{c2}} \cdot \Theta^{SF}$$

$$I_1 = (\sigma_I + \sigma_{II} + \sigma_{III}), \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2$$

$$27J_3 = (2\sigma_I - \sigma_{II} - \sigma_{III}) \cdot (2\sigma_{II} - \sigma_I - \sigma_{III}) \cdot (2\sigma_{III} - \sigma_I - \sigma_{II}),$$

$$I_1 = (\sigma_n + \sigma_t + \sigma_\lambda), \quad 6J_2 = (\sigma_n - \sigma_t)^2 + (\sigma_t - \sigma_\lambda)^2 + (\sigma_\lambda - \sigma_n)^2 + 6 \cdot (\tau_m^2 + \tau_{n\lambda}^2 + \tau_{t\lambda}^2)$$

$$\Theta^{SF} = \sqrt[3]{1 + d^{SF} \cdot \sin(3\vartheta)} = \sqrt[3]{1 + d^{SF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}}, \quad \sigma_t = \sigma_n - C \cdot \eta, \quad \eta = \sigma_{II} - \sigma_{III}.$$

According to Mohr the invariants reduce to: $I_1 = (\sigma_n + \sigma_t)$, $6J_2 = (\sigma_n - \sigma_t)^2 + \sigma_t^2 + \sigma_n^2 + 6 \cdot \tau_m^2$
 $27J_3$ in Mohr stresses is not to be applied.

Note on the Mohr-Coulomb Criterion (also see § 26.1)

In a paper, recently published in *Scientific Reports (2024) nature portfolio [Stress-dependent Mohr-Coulomb shear strength parameters for intact rock [Li24]*, a critical assessment of the M-C criterion is performed. This report shall be not scientifically reworked here. The summarizing private elaboration at hand just tries to inform about the author's procedure to derive an accurate Mohr-Coulomb Envelope including Cohesive Strength. Basis of the procedure is the knowledge, that the M-C Envelope, spanning from the Touch point $(\tau_n^{Tp}, \sigma_n^{Tp}) \rightarrow (\tau_n = R^t, \sigma_n = 0)$, is affected by the shear failure mode together with the tensile failure mode and thus belonging to a transition zone, as a mode interaction domain.

Solution Procedure when an improved M-C curve is to generate

Searched is an equation for the unknown fracture angle measure $C(\theta_{fp})$. This is performed by equating the slopes in the so-called touch point: A first slope equation $d\tau_{nt} / d\sigma_n$ is given by the derivation of the Mohr-model (stress transformation of structural stresses to Mohr stresses) according to the angle of inclination = slope angle in the Touch point, marked Tp . Secondly, one has to find an equation for one of the stresses σ_{III} or σ_{II} . The differentiation of the SFC delivers this equation.

The accurate derivation of the Cohesive Strength R^t and the M-C curve $\tau_n(\sigma_n)$ and further of the fracture angle θ_{fp} requires to consider both the activated modes, according to Cuntze's model of the two activated modes, because R^t lies in the transition zone of the two modes SF and NF.

Differentiation of the Mohr stress relationship generating one of the two required equations

The first equation used is that the tangent of the derived Mohr stress curve is identical to the tangent of the SFC-linked Mohr envelope. An angle-differentiation in the Touch point delivers a relationship for the friction value, as derived below:

$$\frac{d}{d\Theta} \sigma_n = \frac{d}{d\Theta} (c^2 \cdot \sigma_{II} + s^2 \cdot \sigma_{III}) = -2s \cdot c \cdot (\sigma_{II} - \sigma_{III}) ; \frac{d}{d\Theta} \tau_{nt} = \left[\frac{d}{d\Theta} (-s \cdot c \cdot \sigma_{II} + s \cdot c \cdot \sigma_{III}) = (-c^2 + s^2) \cdot \sigma_{II} - (-c^2 + s^2) \cdot \sigma_{III} \right]$$

$$\frac{d}{d\sigma_n} \tau_{nt} = \frac{(-c^2 + s^2) \cdot \sigma_{II} - (-c^2 + s^2) \cdot \sigma_{III}}{-2s \cdot c \cdot (\sigma_{II} - \sigma_{III})} = \frac{c^2 - s^2}{2 \cdot c \cdot s} = \frac{C}{S} = \frac{C}{\sqrt{1 - C^2}} = \tan \rho = \mu$$

$$\cotan(2 \cdot \Theta_{fp}) = \frac{\cos(2 \cdot \Theta_{fp})}{\sin(2 \cdot \Theta_{fp})} = \frac{C}{S} ; \tan(\rho) = -\cot\left(2 \cdot \Theta_{cfr} \cdot \frac{\pi}{180}\right) \quad \rho = -\operatorname{acot}\left(2 \cdot \Theta_{cfr} \cdot \frac{\pi}{180}\right)$$

Strength Failure Criteria considering the 120°-rotational Symmetry of isotropic materials

The generation of a realistic, decaying Mohr-Coulomb curve $\tau_n(\sigma_n)$ requires the determination of the slope *along* the full curve, not a constant value $C = C^c$ in the touch *point* only, being sufficient for the determination of the friction value μ . This means, instead of the single F^{SF}-formulation the SF-NF-interaction managing *Eff*-formulation is to apply when moving from the structural stress formulation to a Mohr stress-based one.

Table 25-1 shows the transformation of the *Eff*s from principal stresses into Mohr stress-based ones. Table 25-2 will later show the full procedure.

Table 25-1: Formulation of Mohr stress-based Effs. TM tensile meridian, CM compressive meridian

$$Eff^{NF} = c^{NF} \cdot \frac{\sqrt{4J_2 \cdot \Theta^{NF} - I_1^2 / 3} + I_1}{2 \cdot \bar{R}^t}, \quad \text{for Normal Concrete can be set } c^{NF}, \Theta^{NF} = 1,$$

$$Eff^{NF} = 1 \cdot \frac{\sqrt{4 \cdot [(\sigma_n - \sigma_t)^2 + \sigma_t^2 + (-\sigma_n)^2 + 6 \cdot \tau_n^2]} \cdot 1 / 6 - (\sigma_n + \sigma_t) / 3 + (\sigma_n + \sigma_t)}{2 \cdot \bar{R}^t}.$$

$$Eff^{SF} = \frac{\sqrt{(c_{2\Theta}^{SF} \cdot I_1)^2 + 12 \cdot c_{1\Theta}^{SF} \cdot J_2 \cdot \Theta^{SF}}}{2 \cdot \bar{R}^c} + \frac{c_2 \cdot I_1}{2 \cdot \bar{R}^c} \quad \text{with } \Theta^{SF} \rightarrow \Theta^{CM} = \sqrt[3]{1 + d^{SF}} \cdot (-1),$$

$$Eff^{SF} = \frac{\sqrt{c_{2\Theta}^{SF2} \cdot (\sigma_n + \sigma_t)^2 + c_{1\Theta}^{SF} \cdot \Theta^{SF} \cdot 2 \cdot [(\sigma_n - \sigma_t)^2 + \sigma_t^2 + (-\sigma_n)^2 + 6 \cdot \tau_n^2]}}{2 \bar{R}^c} + c_2 \cdot \frac{\sigma_n + \sigma_t}{2 \bar{R}^c}.$$

$$\text{with } c_{1\Theta}^{SF} \cdot \Theta^{CM} = 1 + c_2, \quad \Theta^{CM} = \Theta^{SF} = \sqrt[3]{1 + d^{SF}} \cdot \sin(3\vartheta) = \sqrt[3]{1 - d^{SF}}.$$

Fig.25-2 displays the 2nd quadrant of the bi-axial failure curve in structural stresses. This fully represents the Mohr-Coulomb curve domain. The joint mode situation of the Mohr-Coulomb curve - capturing the transition zone between the pure mode domains NF and SF - requires the application of the interaction equation $(Eff^{NF})^m + (Eff^{SF})^m = 1$ and it spans over the regime $0 < \sigma_{III} < \bar{R}^t$, the *transition zone of the modes*, and covers Lode angles $-30^\circ < \vartheta < +30^\circ$.

Improved Mapping of Failure Stress data with Derivation of a more realistic $\Theta_{fp}^\circ(\vartheta)$

As still experienced with the UD-materials, in a chapter before, also here it is to face that a SFC is 'just' a practical approach and therefore cannot sufficiently well map all domain parts in detail. However, when searching a local fracture angle Θ_{fp}° in the transition zone a correction is to be material-dependently applied to numerically determine a better value for Θ_{fp}° one is interested in. In any case, the given SFC calculates a conservative Reserve Factor, the SFC is on the safe side and Design-Verification as well: the Eff^{SF} contribution to *Eff* is not a problem because the interaction is a conservative procedure.

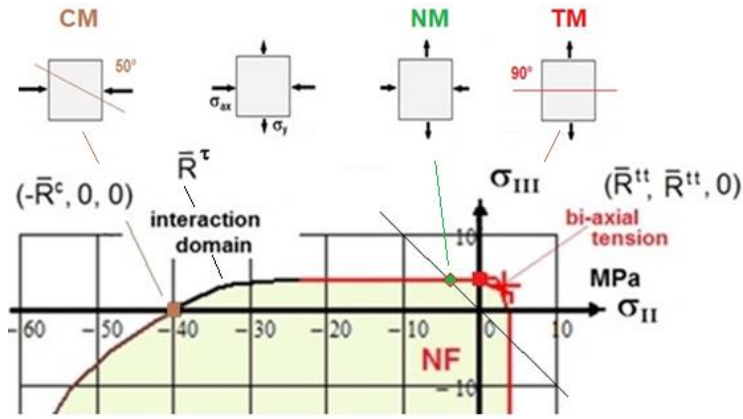


Fig.25-2: Second quadrant and associated stress states, transition zone between the 2 mode domains SF, NF

A characteristic point of the transition zone between the tensile domain and the compressive domain is when the first invariant becomes zero (see the bias grey line in Fig.25-2), meaning pure shear:

$$I_1 = \sigma_I + \sigma_{II} + \sigma_{III} = 0, \text{ for } \sigma_{II}^c = \sigma_{III}^c \rightarrow \sigma_I^t = 2 \cdot \sigma_{III}^c.$$

Mathematically-caused, the Eff^{SF} -curve and the $Eff^{SF\Theta}$ -curve become positive in the pale colored curve part and numerically contribute its effort portion to the total effort. This is physically not accurate.

The Eff^{SF} -curve outlines the local shortcoming of the FMC-based choice of the SF-formula. Negative values of Eff^{SF} are sorted out by a McCauley-procedure, but occurring small positive values have to be made inactive in the low negative σ_{II} domain. Searching a procedure it is helpful to know how the pure mode efforts of the activated modes NF and SF share its influence with σ_{II} . Fig.25-3 shows the courses of the efforts Eff^{NF} ($= Eff^\sigma$) and Eff^{SF} ($= Eff^\tau$) representing the mode components of a measured fracture stress curve. Physical demands are given at the cohesion strength point with $\Theta_{fp} = 90^\circ$ for $\sigma_{III} = R^t$ and $Eff^\tau = 0$ for $\sigma_{II} = 0$). The shear material stressing effort $Eff^\tau = Eff^{SF}$ must physically become zero at the tensile strength point $(0, R^t)$. According to the fact that the compression strength point is located on the compressive meridian and the tensile strength point on the tensile meridian the different Lode angle ϑ is to consider in order to achieve an accurate approach

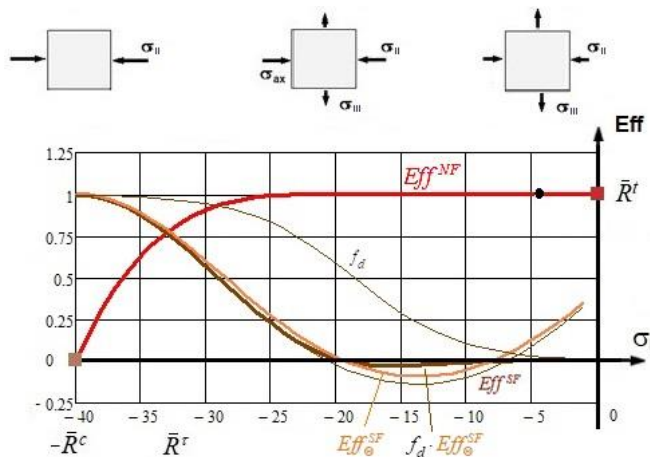


Fig.25-3: Visualisation of the course of the pure mode efforts Eff^{mode} supporting the need to interact

them in the case of bi-axial stress states to not exceed $Eff = 100\%$. Eff^{NF} , Eff^{SF}
when investigating the Mohr-Coulomb envelope curve. This requires to consider the 120°-
rotationally-symmetric $Eff^{SF\theta}$.

Table 25-2: Transformation of a 2D structural stresses-based failure curve into a Mohr-Coulomb one, considering the 120°-rotational symmetry of isotropic materials

| |
|---|
| <p>* Task: Known $\sigma_{II}, \sigma_{III}; \bar{R}^t, \bar{R}^c$; searched $\sigma_n, \tau_n, \Theta_{fp}^\circ(C), C = \cos(2 \cdot \Theta_{fp}^\circ \cdot \pi / 180^\circ)$</p> <p>* Interaction equation, 2 modes are activated: $(Eff^{NF})^m + (Eff^{SF})^m = 100\%$</p> $Eff^{NF} = \frac{\sqrt{4 \cdot [(\sigma_n - \sigma_t)^2 + \sigma_t^2 + \sigma_n^2 + 6 \cdot \tau_m^2] / 6 - (\sigma_n + \sigma_t)^2 / 3 + (\sigma_n + \sigma_t)}}{2 \cdot \bar{R}^t}$ $\frac{Eff^{SF}}{f_d} = \frac{c_2^{SF} \cdot (\sigma_n + \sigma_t)}{2 \cdot \bar{R}^c} + \frac{\sqrt{(c_2^{SF} \cdot (\sigma_n + \sigma_t))^2 + 2 \cdot c_{1\Theta}^{SF} \cdot \Theta^{SF} [(\sigma_n - \sigma_t)^2 + \sigma_t^2 + \sigma_n^2 + 6 \cdot \tau_m^2]}}{2 \cdot \bar{R}^c}$ <p>* Solution of the task means Equalizing the 2 slopes at each stress state $\sigma_{II}(\sigma_{III})$</p> <ul style="list-style-type: none"> Slope 1: Differentiation of the structural stresses-linked Mohr stresses delivers $\frac{d\tau_m}{d\sigma_n} = \frac{(s^2 - c^2) \cdot \eta}{-2 \cdot s \cdot c \cdot \eta} = \frac{C}{S} = \frac{C}{\sqrt{1 - C^2}}, \text{ valid uni-axially and bi-axially, } \eta = \sigma_{II} - \sigma_{III}$ <ul style="list-style-type: none"> Slope 2: Differentiation of the interaction equation, abbreviation $\eta_m = 2\sigma_n - C \cdot \eta$ $Eff^{NF} = \left\{ \sqrt{4 \cdot [(C \cdot \eta)^2 + (\sigma_n - C \cdot \eta)^2 + \sigma_n^2 + 6 \cdot \tau_m^2] / 6 - \eta_m^2 / 3 + \eta_m} \right\} / 2 \cdot \bar{R}^t, \quad \Theta^{NF} = 1$ $Eff^{SF} = \left\{ c_{1\Theta}^{SF} \cdot \eta_m + \sqrt{(c_2^{SF2} \cdot \eta_m^2 + 2 \cdot c_{1\Theta}^{SF} \cdot \Theta^{SF} \cdot [(C \cdot \eta)^2 + (\sigma_n - C \cdot \eta)^2 + \sigma_n^2 + 6 \cdot \tau_m^2])} \right\} / 2 \cdot \bar{R}^c$ $Eff = (Eff^{NF})^m + (Eff^{SF})^m \quad \text{with} \quad dEff / d\sigma_n \quad \text{and} \quad dEff / d\tau_n.$ <p>Equating the 2 equations and replacing Mohr stresses by structural stresses one obtains,</p> <p>considering the implicit differentiation by a negative sign,</p> $\frac{C}{\sqrt{1 - C^2}} = - \frac{dEff / d\sigma_n}{dEff / d\tau_m}, \text{ a huge formula for determining the fracture angle measure } C.$ $\Theta^{SF}(\vartheta) = \sqrt[3]{1 + d^{SF} \cdot \sin(3\vartheta)} = \sqrt[3]{1 + d^{SF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}} \quad \text{inserting}$ $6J_2 = 2 \cdot (\sigma_{II}^2 - \sigma_{II} \cdot \sigma_{III} + \sigma_{III}^2), \quad 27J_3 = (-\sigma_{II} - \sigma_{III}) \cdot (2\sigma_{II} - \sigma_{III}) \cdot (2\sigma_{III} - \sigma_{II}).$ <p>The alternating C depends on the bi-axial fracture stress states $\sigma_{III}^{fr}(\sigma_{II})$, achieved,</p> <p>when inserting σ_{II} into the failure criterion $Eff = 1$</p> $\sigma_n = (C + 1) \cdot 0.5 \cdot \sigma_{II} + (1 - C) \cdot 0.5 \cdot \sigma_{III}, \quad \tau_n = -0.5 \cdot \sqrt{1 - C^2} \cdot (\sigma_{II} - \sigma_{III})$ $a = (12J_2 - I_1^2) / 3, \quad b = \left((c_2^{SF} \cdot I_1 + \sqrt{d}) / 2\bar{R}^c \right)^{m-1},$ $c = 4I_1 \cdot (c_2^{SF2} + c_{1\Theta}^{SF} \cdot \Theta^{SF}), \quad d = c_{2\Theta}^{SF2} \cdot I_1^2 + 2c_{1\Theta}^{SF} \cdot \Theta^{SF} \cdot 6J_2.$ <p>Vorgabe $C := -0.1$ in Mathcad, inserting the running fracture points $\sigma_{III}^{fr}(\sigma_{II})$</p> <p>along the span between Touch point and Cohesive Strength point</p> <p>provides a search equation for the fracture angle measure $C(\Theta_{fp}^\circ)$ and for σ_n, τ_n</p> $\frac{C}{\sqrt{1 - C^2}} = \frac{0.5 \cdot \sqrt{a} \cdot [(\bar{R}^t \cdot b \cdot c) / 4] + \bar{R}^c \cdot \sqrt{d} \cdot \left(\frac{2\sigma_n + \sqrt{a} - C \cdot \eta}{2\bar{R}^t} \right)^{m-1} + \bar{R}^t \cdot c_2^{SF} \cdot b \cdot \sqrt{d}}{\tau_m \cdot [\bar{R}^c \cdot \sqrt{d} \cdot \left(\frac{2\sigma_n + \sqrt{a} - C \cdot \eta}{2\bar{R}^t} \right)^{m-1} + 3\sqrt{a} \cdot \bar{R}^t \cdot c_{1\Theta}^{SF} \cdot \Theta^{SF} \cdot b]}$ $C = C(\sigma_{II}, \sigma_{III}^{fr}) = \cos(2 \cdot \Theta_{fp}^\circ), \quad \Theta_{fp} = 0.5 \cdot \arccos C, \quad \Theta_{fp}^\circ = \Theta_{fp} \cdot 180^\circ / \pi.$ |
|---|

Extrapolated determination of Cohesive strength R^t and Mohr-Coulomb Curve $\tau_n(\sigma_n)$ from R^c -point

Table 25-2 summarizes the relations for the derivation of $\tau_{nt}(\sigma_n)$ and Θ_{fp}° from a given fracture curve $\sigma_{II}(\sigma_{III})$. It is to consider the change of the fracture plane angle Θ_{fp}° with the Lode angle ϑ from Θ_{fp}° at $\vartheta = -30^\circ$ on.

The biggest challenge is the necessary differentiation of the interaction equation $Eff = 1$ within Mathcad. This further produces a giant formula output, that ‘generously’ can be cut down by using addition theorems and by inserting the given structural stresses input via its invariants I_1, J_2, J_3 .

Generally, several variants of M-C envelopes are faced, see the following Tables 25-3 and -4:

Table 25-3: Extrapolation from the compressive strength point, considering the shear mode only

Methods which equalize the two slope angles just in the Touch point.

- R^t by extrapolation from the compressive strength point, rotational symmetry fracture body

$$F_T = c_1 \cdot \frac{(\sigma_n - \sigma_t)^2 + (\sigma_t)^2 + (\sigma_n)^2 + 6\tau_n^2}{2R_c^2} + c_2 \cdot \frac{\sigma_n + \sigma_t}{R_c} = 1$$

$$\tau_n = \frac{\sqrt{3} \cdot \sqrt{-c_1 \cdot (c_2 \cdot R_c \cdot \sigma_n - R_c^2 + c_2 \cdot \sigma_t \cdot R_c + c_1 \cdot \sigma_t^2 - c_1 \cdot \sigma_t \cdot \sigma_n + c_1 \cdot \sigma_n^2)}}{3 \cdot c_1}, \quad \eta := -R_c$$

$$\sigma_n := 0 \quad \sigma_t := -C_c \cdot \eta, \quad c_1 = 1.21 \quad c_2 = 0.21$$

$$R_T := \sqrt{3} \cdot \sqrt{-c_1 \cdot c_2}$$

* Touch point coordinates and friction value from a simple linear Mohr Approach

- Touch point coordinates: $\tau_n = \tau_n^{Tp} + \sigma_n \cdot \mu$

$$s := \sin(\Theta_{fpc}) \quad c := \cos(\Theta_{fpc}) \quad \Theta_{fpc} := 0.5 \cdot \arccos(C_c) \quad \Theta_{fpc}^\circ := \frac{180}{\pi} \cdot \Theta_{fpc}$$

$$\sigma_n^{Tp} := c^2 \cdot (-R_c) \quad \tau_n^{Tp} := -s \cdot c \cdot (-R_c)$$

- Friction value relationship:

$$F_{linMohr} = \frac{\tau_n}{R_T - \mu \cdot \sigma_n} = 1 \quad \tau_n = R_T - \mu \cdot \sigma_n \quad \text{friction angle } \rho$$

$$\frac{d}{d\sigma_n} \frac{\tau_n}{R_T - \mu \cdot \sigma_n} \rightarrow \frac{\mu \cdot \tau_n}{(R_T - \mu \cdot \sigma_n)^2} \quad \frac{d}{d\tau_n} \frac{\tau_n}{R_T - \mu \cdot \sigma_n} \rightarrow 0$$

$$\frac{d}{d\sigma_n} \tau_n = -1 \cdot \frac{\mu \cdot \tau_n}{R_T - \mu \cdot \sigma_n} = \frac{d}{d\sigma_n} \tau_n = \frac{-\mu \cdot \tau_n}{R_T - \mu \cdot \sigma_n} = \frac{C}{S} = \frac{-\mu \cdot \tau_n}{\tau_n} = -\mu$$

Accurate determination of Cohesive strength R^t and Mohr-Coulomb Curve $\tau_n(\sigma_n)$, 180°

In order to capture the 180° -symmetry requirement a 3-parameter approach is to employ. Its model parameters are calculated in Fig.25-4.

Table 25-4: Determination of SF-model parameters

The chosen 3-parameter approach requires minimally 3 fix points

The first point is the compressive strength point on the CM: $I1c := -Rc$, $J2c := \frac{Rc^2}{3}$

The second point is the bi-axial compressive strength point on TM: $I1cc := -2 \cdot Rcc$, $J2cc := \frac{Rcc^2}{3}$

A third point is searched in the principal stress plane, index f.

Here, the meridian angle is unknown and to be determined:

$$\Theta_{\tau} = \sqrt[3]{1 + d_{\tau} \cdot \sin(3\theta)}, \quad \sin(3\theta) = 1.5 \cdot 3^{0.5} \cdot J3 \cdot J2^{-1.5}$$

Θ_{τ} is non-circularity function, d_{τ} is non-circularity parameter

$$\theta f := \frac{1}{3} \cdot \arcsin(1.5 \cdot 3^{0.5} \cdot J3f \cdot J2f^{-1.5}), \quad \sin(3\theta f) = -0.85 \cdot \theta f^p := \frac{180}{\pi} \cdot \theta f$$

$$I1f := \sigma_{1f} + \sigma_{11f}, \quad J2f := \frac{2 \cdot \sigma_{1f}^2 - 2 \cdot \sigma_{1f} \cdot \sigma_{11f} + 2 \cdot \sigma_{11f}^2}{6}, \quad J3f := \frac{(2 \cdot \sigma_{1f} - \sigma_{11f}) \cdot (2 \cdot \sigma_{11f} - \sigma_{1f}) \cdot (-\sigma_{11f} - \sigma_{1f})}{27}$$

Vorgabe $c1 := 2$ $d_{\tau} := 0.6$ $c2 := 4$

$$c1 \cdot \frac{3 \cdot J2c \cdot \sqrt[3]{1 - 1 \cdot d_{\tau}}}{Rc^2} + c2 \cdot \frac{I1c}{Rc} = 1 \quad \text{point on CM}$$

$$c1 \cdot \frac{3 \cdot J2cc \cdot \sqrt[3]{1 + 1 \cdot d_{\tau}}}{Rc^2} + c2 \cdot \frac{I1cc}{Rc} = 1 \quad \text{point on TM}$$

$$c1 \cdot \frac{3 \cdot J2f \cdot \sqrt[3]{1 + 1 \cdot d_{\tau} \cdot \sin(3\theta f)}}{Rc^2} + c2 \cdot \frac{I1f}{Rc} = 1$$

$A_{\tau} := \text{Suchen}(c1, d_{\tau}, c2)$

$$A_{\tau} = \begin{pmatrix} 29.973 \\ 0.601 \\ 21.068 \end{pmatrix} \quad \begin{matrix} c1 := A_{\tau 0} \\ c1 = 30 \end{matrix} \quad \begin{matrix} d_{\tau} := A_{\tau 1} \\ d_{\tau} = 0.6 \end{matrix} \quad \begin{matrix} c2 := A_{\tau 2} \\ c2 = 21.1 \end{matrix}$$

Another improvement point is that the shear material stressing effort $Eff^{\tau} = Eff^{SF}$ must physically become zero at the tensile strength point $(0, R^t)$ the σ_{III} -coordinate. This specific shortcoming cannot be by-passed by an increased interaction exponent m . This is brought about by a correction function that defines the decay of Eff^{τ} and is practically performed by setting $Eff^{\tau} = 0$ at $\sigma_{II} = 0$. As function an exponential one was taken, namely

$$f_d = 1 / (1 + \exp(\frac{c_{1d} + \sigma_{II}}{c_{2d}})), \quad \text{with } c_{1d}, c_{2d} \text{ fixed at } (-\bar{R}^c, 0.995), (-0.01, +0.01).$$

Final task is equalizing the two slope angles along the full domain, generated by the joint action of the two modes and considering the degradation function f_d .

Table 25-4: Determination of R^{τ} , $\tau_n(\sigma_n)$, considering both activated modes and degradation

$$Eff^{NF} = \frac{\sqrt{4 \cdot [(\sigma_n - \sigma_t)^2 + \sigma_t^2 + \sigma_n^2 + 6 \cdot \tau_n^2] / 6 - (\sigma_n + \sigma_t)^2 / 3 + (\sigma_n + \sigma_t)}}{2 \cdot \bar{R}^t},$$

$$\frac{Eff^{SF}}{f_d} = \left[\frac{c_2^{SF} \cdot (\sigma_n + \sigma_t)}{2 \cdot \bar{R}^c} + \frac{\sqrt{(c_2^{SF} \cdot (\sigma_n + \sigma_t))^2 + 2 \cdot c_{1\Theta}^{SF} \cdot \Theta^{SF} [(\sigma_n - \sigma_t)^2 + \sigma_t^2 + \sigma_n^2 + 6 \cdot \tau_n^2]}}{2 \cdot \bar{R}^c} \right]$$

with $c_{1\Theta}^{SF} \equiv c_1$, $c_2^{SF} = c_2$, $c_{1d} = 18.6$, $c_{2d} = 4.04$, $\Theta^{SF} = \Theta^c = \Theta^{CM} = 0.51$.

$R^t =$

$\tau_n(\sigma_n) =$

Touch point Coordinates : $\sigma_{nTp} := c_2 \cdot (-Rc)$ $\tau_{nTp} := -s \cdot c \cdot (-Rc)$ $\tau_{nTp} = 19.6$ $\sigma_{nTp} = -15.8$

Envelope Curve

$$c_2 \cdot \frac{\sigma_n + \sigma_t + \sigma_t}{Rc} + c_1 \cdot \frac{(\sigma_n - \sigma_t)^2 + (\sigma_t - \sigma_t)^2 + (\sigma_t - \sigma_n)^2 + 6(\tau_n^2 + \tau_n^2 + \tau_n^2)}{2Rc^2} = 1$$

$\sigma_{III} = 0, \sigma_{II} = -Rc$
 $\eta = (-Rc)$
 $\sigma_n - \sigma_t = C \cdot \eta = Cc \cdot (-Rc)$
 $\sigma_n + \sigma_t = \sigma_{II} + \sigma_{III} = -Rc$
 $\sigma_t = -Cc \cdot \eta + \sigma_n = -Cc \cdot (-Rc) + \sigma_n$

$$c_2 \cdot \frac{\sigma_n + 0 + \sigma_t}{Rc} + c_1 \cdot \frac{(\sigma_n - \sigma_t)^2 + (\sigma_t - 0)^2 + (0 - \sigma_n)^2 + 6(\tau_n^2 + 0)}{2Rc^2} = 1$$

$$\tau_n^2 := \frac{c_2 \cdot Rc \cdot \sigma_n - Rc^2 + c_2 \cdot \sigma_t \cdot Rc + c_1 \cdot \sigma_t^2 - c_1 \cdot \sigma_t \cdot \sigma_n + c_1 \cdot \sigma_n^2}{3 \cdot c_1}$$

Cohesive (shear) Strength Value :

Simple Mohr-Coulomb: $\sigma_n := 0$ $R_{\tau I} := [\tau_{nTp} + \mu \cdot (\sigma_{nTp})] \sigma_t = -8.3$ $c_1 = 30$ $c_2 = 21.1$

Linear M-C Extrapolation: $\sigma_t = -8.3$ $R_{\tau} := \sqrt{\frac{-Rc^2 + c_2 \cdot \sigma_t \cdot Rc + c_1 \cdot \sigma_t^2}{3 \cdot c_1}}$ $R_{\tau I} = 16.2$

Simple attempt: 120° Rotational Symmetry: $c_1 = 30$ $R_{\tau\Theta} := \sqrt{\frac{-Rc^2 + c_2 \cdot \sigma_t \cdot Rc + c_1 \cdot \sigma_t^2}{3 \cdot c_1}}$ $R_t := 4$
 $R_{\tau\Theta} = 9$

Fig.25-4 displays several M-C failure curves **and the course of the altering fracture plane angle** Θ_{fp}° . The angle changes from 51° up to 90° . left part presents the entities in a structural stresses diagram and the right part in a Mohr stresses diagram. This involves the Linear Mohr-Coulomb fracture curve and the real SF-NF interaction curve $\tau_{nt}(\sigma_n)$. The 3 Mohr half-circles are incorporated.

Fig.25-5 presents model-dependent cohesive strength values R^t , the interaction-based $\tau_n(\sigma_n)$ -curve, topped by the course of the increasing **fracture angle**, factored by 10. **Datensatz Check + angle**

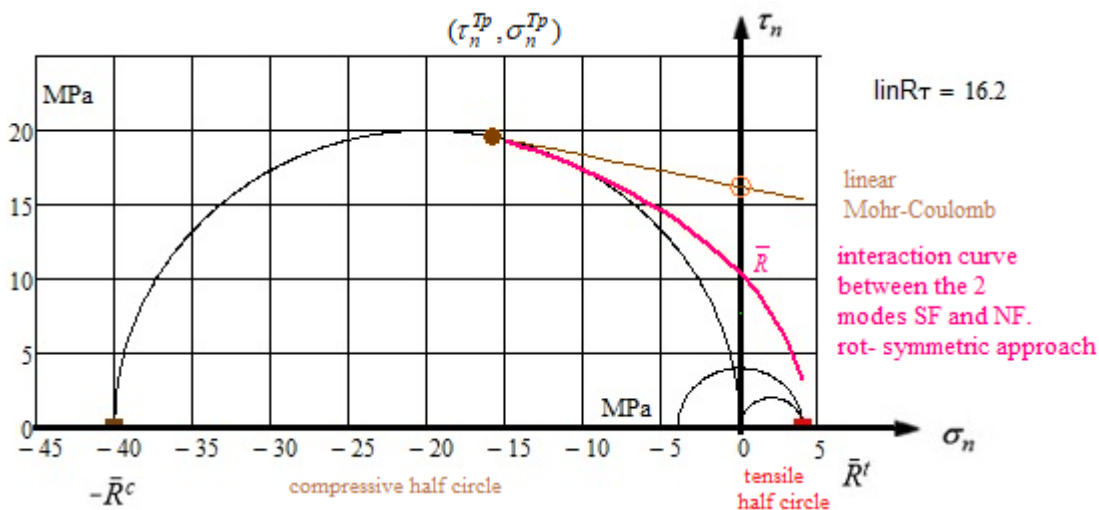


Fig.25-4: Joint display of the *magenta* failure curve in principal stresses (left) and in Mohr stresses (right). Fracture angle increase Θ_{fp}° , scaled by twenty (left) and ten (right), is incorporated. 120°-rotational-symmetric model and improved mapping of the measured failure curve due to f_d . The Mohr half-circles are incorporated

Corrected data set:

$$c_{1\Theta}^{SF} \cdot \Theta^{SF} = 1 + c_2^{SF} \text{ with } c_2^{SF} \text{ as friction parameter} = 3.70, c_{1\Theta}^{SF} = 5.88, d^{SF} = 0.49,$$

$$\Theta_{fp}^\circ = 50.5^\circ, C^c = -0.192, \text{ friction value } \mu = 0.2, \bar{R}^c = 40 \text{ MPa}$$

$$\text{For } \bar{R}^\tau = 11 \text{ MPa}, \Theta_{fp}^\circ = 72^\circ, \sigma_{II} = -32 \text{ MPa}, \sigma_{III} = 3.3 \text{ MPa}, \delta = -25^\circ.$$

$$\text{Touch point data: } \tau_n^c = 19.6 \text{ MPa}, \sigma_n^c = -16.2 \text{ MPa}.$$

LL from interpretation of the diagrams above leads to the following results:

- ✓ The interaction equation Eff - representing the surface of the fracture body in structural stresses - can be transferred into Mohr stresses
- ✓ It was possible to determine the alteration of the not linear M-C curve and of the fracture plane angle Θ_{fp}°
- ✓ Complete failure danger is composed of portions Eff^{NF} and Eff^{SF} , following the idea of the FMC that NF and SF commonly add its Eff portions. This leads to the conclusion that the Θ_{fp}° is approximately 70° at the cohesive strength point, compared to $\Theta_{fp}^\circ = 51^\circ$ at the Touch point, representing the compressive strength point
- ✓ An approach (*for this sub-task too simple*), just employing SF cannot give a fully accurate fracture plane angle Θ_{fp}° and not a good Mohr-Coulomb curve
- ✓ For this fundamental task Eff^{SF} had to be adjusted by a decay function f_d according to two ‘physical musts’. The corrected Eff^{SF} -curve delivers the desired improvement of Θ_{fp}° and lowers \bar{R}^τ from **12.5** → 11 MPa. With this correction, then the envisaged transformation was performed for Normal Concrete.

Note, please:

Applying - instead of the present Modal SFC - a so-called Global SFC, which globally maps by one single mathematical equation all test data and modes, the same bottleneck for the execution of this specific task would be found, too!

In the context of the chapters 24 and 25, before going into further investigations on isotropic concrete and later rock material this is a good place to guideline the physical thinking in the following chapters by reminding of the author’s FMC-based failure mode thinking. This includes the action confining pressure and the action bi-axial compressive stress.

And further: Designing hydraulic borehole fracturing Jaeger et al. are still writing in 2007 (*cited in [You15]*) “Tensile strengths predicted by both the Coulomb criterion (onset-of cracking, solid mechanics) and the Griffith criterion (cracked, fracture mechanics) are much higher than the

measured magnitudes of almost all rocks, although the two criteria have clear physical backgrounds”. This is unbelievable for the author : Each mode is dominated by one technical strength R . And, it is known that the cohesive (shear) strength is the result of two acting modes, namely SF with NF. This clearly leads to the conclusion : just a “ SF-based criterion cannot give a prediction for the tensile strength R^t .”

26 Mapping 3D Test Results of Concrete and Rocks obtained on the Meridians TM and CM

Aim: Provision of a test data evaluation formula for the test meridian, being a cross-section of a physically to be defined fracture body surface.

26.1 General

Sufficient strength of tunnels and dam slopes are vital Design Verification requirements in geo-engineering. In order to achieve this, the course of the measured fracture data on Tensile (TM) and Compressive Meridian (CM) is to map. For this design task several SFC approaches are applied:

“Linear Mohr-Coulomb shear curve”

Shear stresses below the curve mean ‘No fracture’, or ‘Stress states’ below the τ^n -curve are not dangerous. In civil engineering this well-known simple SFC reads: $\tau^n(\sigma_I, \sigma_{III}) = \sigma \cdot \tan \phi + c$.

→ values for cohesive shear strength c and friction angle $\phi(\mu)$ are required.

This is an extrapolation from the compressive strength point.

“Linear Mohr-Coulomb criterion in geo-mechanics”

In order to achieve Design Verification in several numerical Rock mechanics Codes the use of the widely applied ‘Mohr-Coulomb (M-C) Criterion’ is recommended in order to map the course on that meridian where the tests have been run, on TM or CM. The criterion below says that a stress below the M-C curve is conservative.

$$\sigma_1 = \sigma_3 \cdot \tan^2(\Phi) + 2 \cdot c \cdot \tan(\Phi), \text{ applying } \Phi = \pi / 4 + \phi / 2$$

where c = cohesive shear strength $\equiv R^f$, ϕ = internal friction angle, and

σ_1 = most negative principal stress → σ_{III}, σ_3 = most positive principal stress → σ_I ,

which are transferred due to the mathematical principal stress convention $\sigma_I > \sigma_{II} > \sigma_{III}$.

The final formulation reads: $\sigma_{III} = \sigma_I \cdot \tan^2(\Phi) + 2 \cdot c \cdot \tan(\Phi)$.

However in application, a difficult to be answered question arises: Which parameters are to insert? This concerns the fracture angle ϕ and the cohesive strength R^f .

→ values for cohesive shear strength c and friction angle are required.

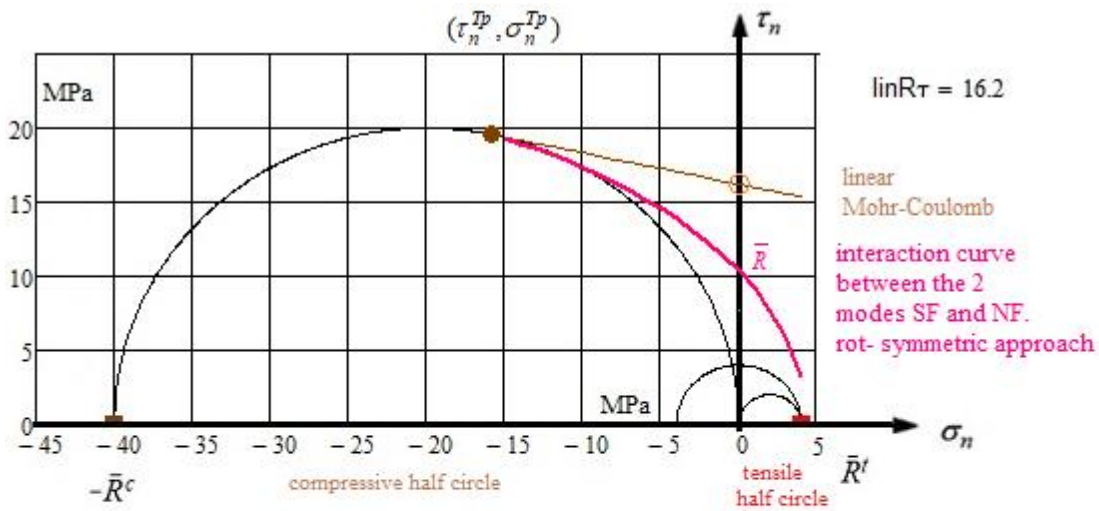
This also is usually done as an extrapolation from the compressive strength point.

In Fig.26-1 the derivation of the associated input data set is provided. Concerning R^f it is referred to a previous chapter where the cohesive strength has been investigated.

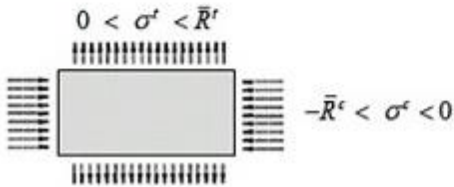
“Cuntze’s FMC-based SFCs regarding the common acting of SF and NF”

The SFC model, spanning up the isotropic fracture body, is shown in Table 26-1, too.

As still described before, the first part of the SFC in Table 26-1 represents the shape change, the second the friction effect, the third the volume change and Θ^f the 120°-symmetry of isotropic materials. Mapping the test data in the very high negative compression domain of UHPC could require a fifth part, which may be dedicated to a further effect: discontinuous densification including at first a failure body hoop reduction and later widening.



bi-axial stress domain of Mohr-Coulomb curve



from Mohr-Coulomb curve: R_{τ}

$$\Theta_{fpc}^{\circ} := 52 \quad \phi^{\circ} := 2 \cdot \Theta_{fpc}^{\circ} - 90$$

$$\phi^{\circ} = 14 \quad \phi := \phi^{\circ} \cdot \frac{\pi}{180}$$

$$\Phi := \left(\frac{\pi}{4} + \frac{\phi}{2} \right) \quad \phi = 0.24 \quad \Phi = 0.91$$

$$\sigma_{III} := \sigma_I \cdot (\tan(\Phi))^2 - 2 \cdot R_{\tau} \cdot \tan(\Phi)$$

Fig.26-1, example Normal Concrete: (left) Derivation of a data set for the M-C criterion. (right) Model dependent cohesive strength values (from Normal Concrete)

The SFC contains five un-known parameters. For their determination, mathematically at minimum, five reliable test points on the surface of the fracture body are to provide by tests along the TM and CM. Better fitting procedures could be applied, of course.

Good Mapping requires to capture physics and to apply SFCs being as simple as possible.

Cuntze's approach includes a multifold mapping task, which can be a compromise, only:

- (1) Mapping the 2D test data in the principal stress plane, considering here the friction effect.
- (2) Mapping the 3D test data along tensile test meridian (TM) and compressive test meridian (CM) along these two axial (180°-opposite) cross-sections of the fracture body with TM $(\sigma_I, \sigma_{II} = \sigma_{III}) \rightarrow \sigma_I(2\sigma_{III})$ and CM $(\sigma_I = \sigma_{II}, \sigma_{III}) \rightarrow 2\sigma_I(\sigma_{III})$.

All subfigures, principal stress plane and meridian cross-sections must be able to be derived from the well mapped fracture body and this with sufficient precision.

→ cohesive shear strength $c = R^t$ and friction angle ϕ are not required.

Points on the fracture body surface are used to fix the model parameters.

Reminder: All isotropic materials possess a more or less significant 120°-rotational symmetry of the fracture body most often depicted in Haigh-Lode-Westergaard coordinates, see Fig.26-2. Thereby, the well-known invariant J_3 is an excellent function to map this type of rotational symmetry (caused by $R^{cc} \neq R^c$ or $R^{tt} < R^t$) and to determine the Lode angle ϑ . Well-known is that the tests are run on the CM and on the TM, meaning that data sets are given for CM with $\vartheta = -30^\circ$ and for TM with $\vartheta = 30^\circ$.

For the chosen Model, depicted in *Table 26-1*, the tricky procedure how to obtain the required model parameters c_i is shown in *Table 26-2* by three steps. For the cohesive strength, required by the extrapolation approaches, numerical values are determined in the third step. Different models deliver different values improved models deliver a lower value, because these consider both damaging modes SF and NF.

Table 26-1: UHPC-SFC model

| shape change | friction | volume change |
|--|----------|---------------|
| $I_1 < 0: F^{SF} = F^\tau = c_1^{SF} \cdot \frac{6J_2 \cdot \Theta^{SF}}{2 \cdot \bar{R}^c} + c_2^{SF}(\mu) \cdot \frac{I_1}{\bar{R}^c} + c_3 \cdot \left(\frac{I_1}{\bar{R}^c}\right)^2 + c_4 \cdot \left(\frac{I_1}{\bar{R}^c}\right)^3 = 1$ | | |
| <p>'Mises Cylinder' formula</p> | | |
| <p>Above SFC is here normalized by the compressive strength \bar{R}^c. J_2 is the 'Mises' invariant.</p> | | |
| $I_1 = (\sigma_I + \sigma_{II} + \sigma_{III}) = f(\boldsymbol{\sigma}), \quad 6J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2 = f(\boldsymbol{\tau})$ | | |
| $27J_3 = (2\sigma_I - \sigma_{II} - \sigma_{III}) \cdot (2\sigma_{II} - \sigma_I - \sigma_{III}) \cdot (2\sigma_{III} - \sigma_I - \sigma_{II}),$ | | |
| $3 \cdot \sigma_{oct} = \sigma_I + \sigma_{II} + \sigma_{III}; \quad 9 \cdot \tau_{oct}^2 = 6J_2 = 4 \cdot (\tau_{III}^2 + \tau_I^2 + \tau_{II}^2), \quad \tau_{II} = \max \tau(\text{mathem.})$ | | |
| <p>$\sigma_I, \sigma_{II}, \sigma_{III}$ are principal stresses, $\sigma_I > \sigma_{II} > \sigma_{III}$ are mathematical stresses (> more positive)</p> | | |
| <p>with $I_1 = (\sigma_I + \sigma_{II} + \sigma_{III})$, $6 \cdot J_2 = (\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2$</p> | | |
| <p>Consideration: 120°-rotational symmetry of isotropic materials: $d^{SF} \equiv d_\tau$ later</p> | | |
| $\Theta^{SF}(J_3, J_2) = \sqrt[3]{1 + d^{SF} \cdot \sin(3\vartheta)} = \sqrt[3]{1 + d^{SF} \cdot 1.5 \cdot \sqrt{3} \cdot J_3 \cdot J_2^{-1.5}}$ | | |
| <p>with the non-circularity function Θ^{SF} including d^{SF} as non-circularity parameter.</p> | | |
| <p>Compr. Meridian: $\Theta^{SF} = \sqrt[3]{1 - d^{SF}}$, Tensile Meridian: $\Theta^{SF} = \sqrt[3]{1 + d^{SF}}$, Neutral Meridian: $\Theta^{SF} = 1$.</p> | | |

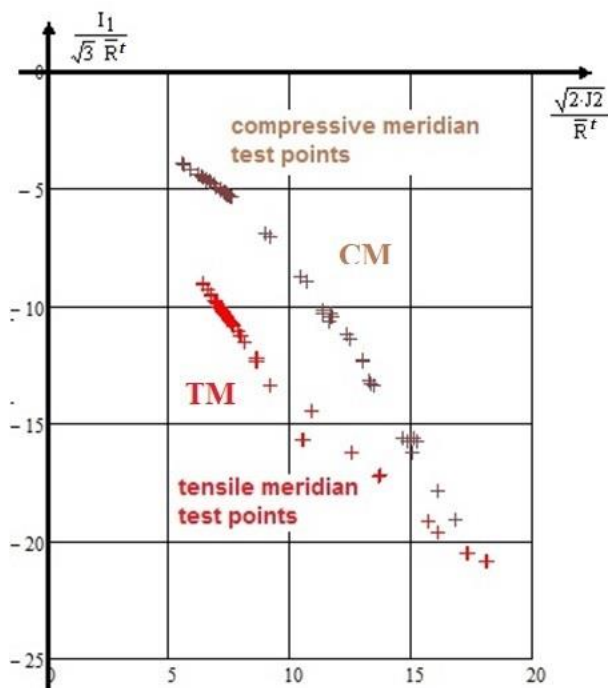


Fig.26-2, example UHPC:
Compressive + and
tensile meridian + test points.

Intentionally depicted on the positive abscissa
to outline the difference stemming from the
brittle isotropic material's inherent 120°-
rotational symmetry of the onset-of-fracture
failure body

Table 26- 2, example Normal Concrete: Procedure to obtain model parameters. Material properties in § 24

1 Relationship of friction parameter and value considering the simple Two Parameter Model

$$F^\tau = c_2 \cdot \frac{I_1}{\bar{R}^c} + c_{1\Theta\tau} \cdot \frac{6 \cdot J_2 \cdot \Theta^{SF}}{2 \cdot \bar{R}^{c^2}} = 1 \leftarrow \text{insertion of the compression point } c_2 \cdot \frac{-\bar{R}^c}{\bar{R}^c} + c_{1\Theta\tau} \cdot \frac{6 \cdot \bar{R}^{c^2} / 3 \cdot \Theta^{SF}}{2 \cdot \bar{R}^{c^2}} = 1$$

($c_{2\Theta} = c_2$, because the friction parameter does not depend on 120°-rotational symmetry)

$$\Rightarrow c_{1\Theta} = \frac{1+c_2}{\Theta^{CM}} \text{ with non-circularity function } \Theta^{CM} = \sqrt[3]{1-d_\tau} \text{ (if rotationally-symmetric, } d_\tau = 0, \Theta^{SF} = 1),$$

Estimation of c_2 also by a guess of friction value μ from $c_2 \approx (3\mu + \sqrt{1+\mu^2}) / (-3\mu + \sqrt{1+\mu^2})$.

Estimation of c_2 possible by a guess of friction value μ from $c_2 \approx (3\mu + \sqrt{1+\mu^2}) / (-3\mu + \sqrt{1+\mu^2})$.

2 Combined Determination of non-circularity parameter d_τ , c_2 and $c_{1\Theta\tau}$ (Mathcad Coding)

If no test value available, this requires an estimate for \bar{R}^{cc} (lies on the Tensile Meridian).

Vorgabe $c_{1\Theta\tau} := 5$ $d_\tau := 0.6$ $c_2 := 4$ $\sigma_{If} := -11.5$ $\sigma_{IIIIf} := -73$ $R_c = 43$ $R_{cc} = 49$

compression strength point R_c on CM

$$c_{1\Theta\tau} \cdot \frac{3J_2 c \cdot \sqrt[3]{1-d_\tau}}{R_c^2} + c_2 \cdot \frac{I_1 c}{R_c} = 1$$

point $(-R_{cc}, -R_{cc})$ on TM

$$c_{1\Theta\tau} \cdot \frac{3J_2 c c \cdot \sqrt[3]{1+d_\tau}}{R_c^2} + c_2 \cdot \frac{I_1 c c}{R_c} = 1$$

Insertion of a far 3D point on TM

$$\frac{c_2 + 1}{\sqrt[3]{1+d_\tau(-1)}} \cdot \frac{[(\sigma_{If} - \sigma_{IIIIf})^2 + (\sigma_{IIIIf} - \sigma_{If})^2] \cdot \sqrt[3]{1+d_\tau(1)}}{2R_c^2} + c_2 \cdot \frac{\sigma_{If} + 2\sigma_{IIIIf}}{R_c} = 1$$

$A_\tau := \text{Suchen}(c_{1\Theta\tau}, d_\tau, c_2)$

$$A_\tau = \begin{pmatrix} 12.462 \\ 0.633 \\ 7.922 \end{pmatrix}$$

$$c_{1\Theta\tau} := A_{\tau 0}$$

$$c_{1\Theta\tau} = 12.46$$

$$d_\tau := A_{\tau 1}$$

$$d_\tau = 0.633$$

$$c_2 := A_{\tau 2}$$

$$c_2 = 7.92$$

$$C_c := \frac{-1}{3} \cdot \frac{(c_2 - 1)}{c_2 + 1}$$

$$\mu := -\frac{C_c}{\sqrt{1 - C_c^2}}$$

$$C_c = -0.26$$

$$\mu = 0.21$$

3 Estimation of the Cohesive Strength for application of the Mohr-Coulomb Criterion

touch point coordinates: $\sigma_{nTp} := c^2 \cdot (-R_c)$ $\tau_{nTp} := -s \cdot c \cdot (-R_c)$ $c_{cc} := \sqrt{\frac{C_c}{2} + \frac{1}{2}}$ $s_{cc} := \sqrt{\frac{1}{2} - \frac{C_c}{2}}$

$$\sigma_{nTp} = -16$$
 $\tau_{nTp} = 21$

Simple Mohr-Coulomb: $\text{lin}R_\tau := [\tau_{nTp} + \mu \cdot (\sigma_{nTp})]$ $C = C_c = \text{constant along the M-C curve}$ $\text{lin}R_\tau = 17.4$

Linear M-C Extrapolation:

$$R_\tau := \sqrt{\frac{-R_c^2 + c_2 \cdot \sigma_\tau \cdot R_c + c_1 \cdot \sigma_\tau^2}{3 \cdot c_1}}$$

$$\sigma_\tau := -C_c \cdot (-R_c)$$

$$c_{1\Theta\tau} = 12.5$$
 $C_c = -0.26$

$$R_\tau = 13$$

Andere daten, passen nicht zusammen

26.2 Ultra-High-Performance-Concrete UHPC (relatively dense)

UHPC principally behaves similarly to Normal Concrete unless the normalized hydrostatic compression does not become larger than $I_1 / R^t \cdot \sqrt{3} \approx -10$ (> -300 MPa), see *Fig.26-2*. However, in contrast to Normal Concrete with usually relatively low hydrostatic pressure loadings the UHPC experiences a hydrostatically activated effect, ‘densification with volume shrinkage’. Therefore, the volume change must be considered by I_1^2 . This explains why for the less ‘dense’ Normal Concrete R^{cc}/R^c is higher than with UHPC according to the possible higher densification. Combined with this a ‘healing’ of the flaw effects can be faced.

The fracture body of a theoretically dense concrete matrix possesses in the high hydrostatic compressive domain ($I_1 < 0$) an open fracture surface due to the densification. Practically however, the fracture body does only exist once according to the final spatial micro-crack state and cannot be loaded a second time after deloading. Further, the bi-axial compressive strength $R^{cc} \equiv f^{ec}$ (internationally used letter in construction, stems from the German term Festigkeit) may be not only linked to SF but also to NF due to the Poisson’s ratio activated tensile strain in the axial direction despite $\sigma_{ax} = 0$.

An SFC-model validation requires reliable test results. These have been provided as original data sets by the IMb Dresden (many thanks to Prof. Curbach and his co-workers). Hence, the author had to search out of the huge test data package which test points belong to TM and which to CM. Therefore, the full bunch of obtained 3D test data had to be processed. Such a separation bases on the use of the Lode angle (meridian angle) ϑ values: Which test point belongs more to the tensile meridian $\sin(3\vartheta) = 1$ or to the compressive meridian $\sin(3\vartheta) = -1$, see *Table 26-3* and *Fig.26-4*. For the shear meridian (neutral meridian NM) angle is valid $\sin(3\vartheta) = \vartheta = 0$.

Considering all physical aspects, cited before, the basic SFC reads:

$$F^{\tau} = c_1 \cdot \frac{6J_2 \cdot \Theta^{SF}}{2 \cdot \bar{R}^{c2}} + c_2(\mu) \cdot \frac{I_1}{\bar{R}^c} + c_3 \cdot \left(\frac{I_1}{\bar{R}^c} \right)^2 + c_4 \cdot \left(\frac{I_1}{\bar{R}^c} \right)^3 = 1 \text{ with}$$

5 unknowns, which at least require 5 fix stress state failure points on the failure surface

$$\{\sigma\} = (, ,)^T : (-\bar{R}^c, 0, 0), (-\bar{R}^{cc}, -\bar{R}^{cc}, 0, 0), (\sigma_I^{frict}, \sigma_I^{frict}, 0), (\bar{R}^c, 0, 0), (\bar{R}^c, 0, 0).$$

The values in MPa = N/mm² are: $\bar{R}^c = 175$, $\bar{R}^{cc} = 183$, $\sigma_I^{frict} = -195$, $\sigma_{II}^{frict} = -50$,

Points: $\sigma_I^{TM} = -40$, $\sigma_{II}^{TM} = \sigma_{III}^{TM}$, $\sigma_{III}^{TM} = -375$; $\sigma_I^{CM} = -61$, $\sigma_{II}^{CM} = \sigma_I^{CM}$, $\sigma_{III}^{CM} = -420$,.

As proven mode interaction coefficient is taken again $m = 2.7$.

Properties applied for UHPC: $\ln R^t = 70$ MPa, $\phi^{\circ} = 14^{\circ}$.

Table 26-3 presents the essential numbers of some measured failure stress states. The table indicates the Lode angle ϑ° , too. On basis of redundancy effects it may be concluded that with increasing hydrostatic pressure both the meridians run into a common scatter band, ending with a circle shape of the hoop. Then, the effect of flaws generating micro-damaging in this heterogeneous material reduces. Thereby, the fracture failure body becomes more and more cylindrical.

Fig. 26-4 links multi-axial stress states to the Lode angles -30° (CM) and $+30^{\circ}$ (TM). Only stress states on the two meridians can be really depicted in the cross-sections. All other test points lie on the fixed hoop radius on a Lode angle different to $+30^{\circ}$ and -30° . These points are marked by o.

Table 26-3: Characteristic material data when evaluating UHPC fracture tests. $E = 20000 \text{ MPa}$, $\nu = 0.2$,

$$\tau_{\max} = \sigma_I - \sigma_{III}$$

| | Stress state in MPa | $I_1/\sqrt{3}$ /R' | $\sqrt{2 \cdot J_2}$ /R' | τ_{\max} in MPa | $(\sigma_{\text{hyd}}; \sigma_{\text{ax}})$ in MPa | ϑ° | $\varepsilon_{\text{ax,el}}$ in 10^{-3} | remarks |
|------------------------|------------------------|-----------------------|-----------------------------|-------------------------|---|-------------------|--|-----------------------|
| R^{tt} | (14, 14, 0) | 1 | 0.7 | - | - | -30 | 0.6 | two-fold NF |
| R^c | (0, 0, -160) | -5.8 | 8.2 | 80 | (-0; -160) | -30 | -8 | Eff_{SF} |
| Compressive comput. | (-6, -6, -230) | -8.7 | 11.4 | 111 | (-6; -224) | -30 | -11 | |
| | (-16, -16, -272) | -11.0 | 13.1 | 128 | (-16; -256) | -30 | -13 | |
| | (-35, -35, -350) | -15.2 | 16.1 | 157 | (-35; -315) | -30 | -17 | |
| | (-83, -83, -490) | -23.7 | 20.8 | 204 | (-83; -407) | -30 | -23 | |
| | (-23, -23, -305) | -12.6 | 14.4 | 141 | (-23; -282) | -30 | -15 | |
| R^{cc} | (0, -175, -175) | -12.6 | 8.9 | 88 | (0; -) | +30 | 3.5 | two-fold SF |
| Tensile R^t | (-2, -210, -210) | -15.2 | 10.6 | 104 | (-2; -) | +30 | 4.1 | |
| | (-24, -310, -310) | -23.2 | 14.6 | 143 | (-24; -) | +30 | 5.0 | |
| | (-54, -388, -388) | -30 | 17 | 167 | (-54; -) | +30 | 5.1 | |
| shear change | (16, 0, 0) | 0.6 | 0.8 | - | - | +30 | 0.8 | Eff_{NF} |
| | (9, -9, 0) | 0 | 0.8 | 9 | - | 0 | 0.5 | $Eff_{NF} > Eff_{SF}$ |
| | (0, -52, -193) | -8.8 | 8.8 | 97 | - | -15 | -9.1 | |

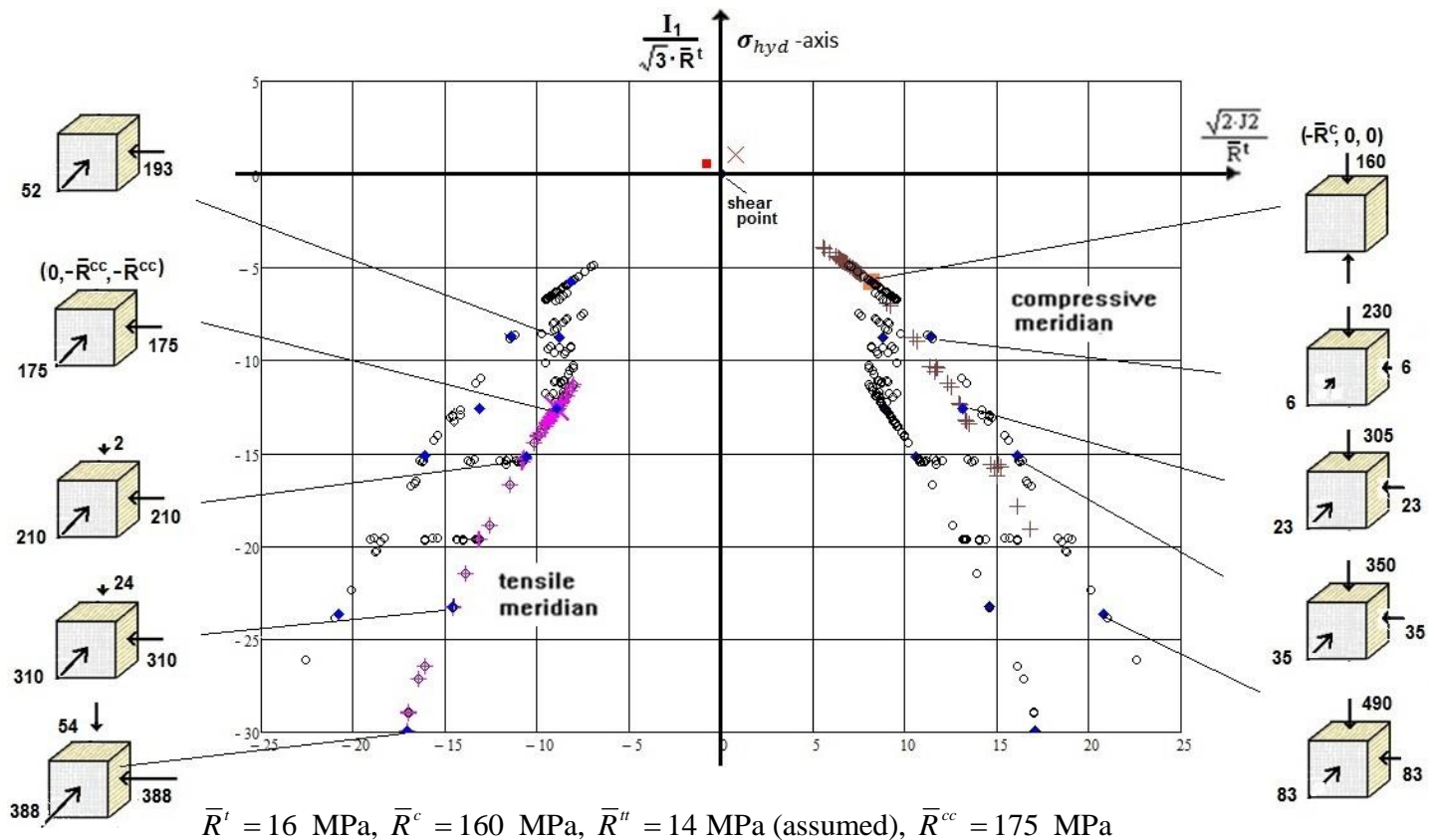


Fig.26-4, UHPC: Compressive and tensile meridian of the fracture body with associated stress states. (left) mirrored TM test points + with CM ones +; (right) all 3D test points are marked by o (hoop ring), visualizing to be located at different meridians ϑ .

Fig.26-5 displays the mapping quality in the principal stress plane. For comparison the [elliptical curve](#), as the bias cross-section of a cylinder is included. The figure indicates that there is no Mises cylinder given, 120°- rotational symmetry acts.

Fig.26-6 (left) displays the mapping of the TM and the CM data course on the cross section of the fracture body in the traditional Haigh-Lode-Westergaard coordinates.

Fig.26-6 (right) depicts the mapping of the TM data set in a diagram using the ‘rock mechanics coordinates’ (σ_I, σ_{III}) for TM and $(2\sigma_I, \sigma_{III})$ for CM. How the effortful programming has been performed is compiled in [Table 26-4](#). Unfortunately Mathcad did not compute the CM curve. The ‘Mohr-Coulomb (M-C) Criterion’, using the calculated UHPC-parameters, produces a straight line. The author could not find any explanation for this unacceptable mapping.

According to the 180°-material symmetry the TM curve could be mirrored from the CM curve by switching from θ^{CM} to θ^{TM} .

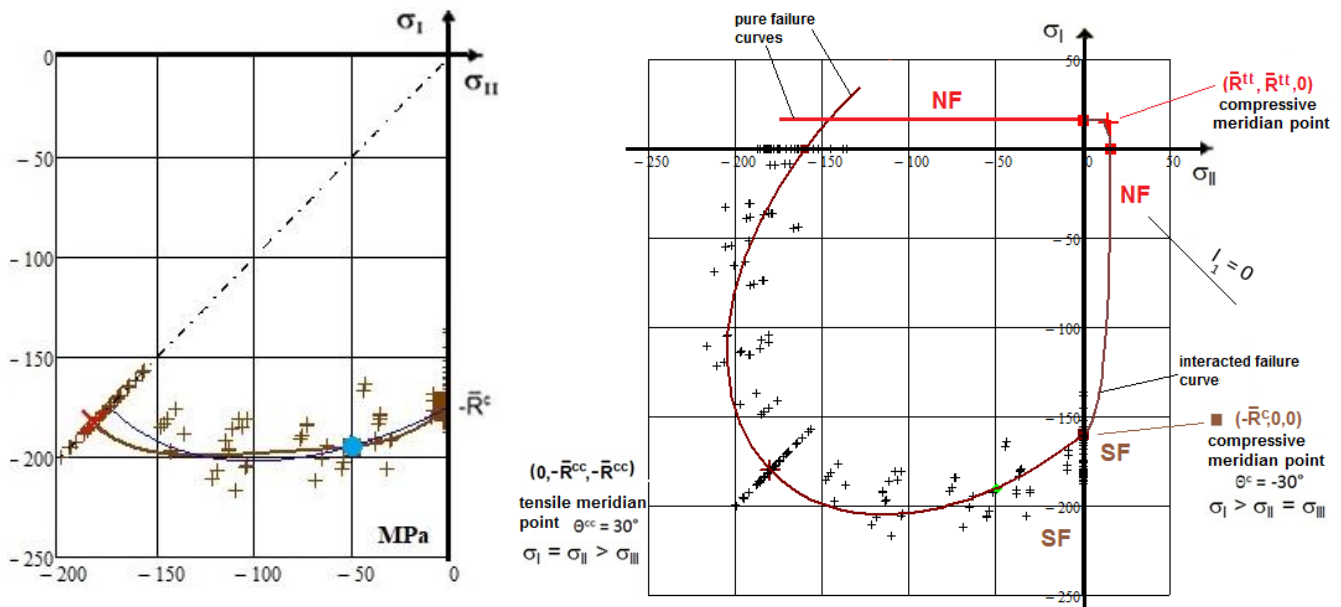


Fig.26-5, UHPC: (left) Mapping the course of 2D test data in the principal stress plane. The blue fix point serves for friction quantification, mapping course of test data in the SF-domain only (normalization by R^c .) considering the alternating Lode angle θ (right) Full principal stress plane view, mapping interaction NF with SF in their transition zone (normalization by R^t).

LL:

* *Reliable mapping requires an approach which shall be physically-based and ‘practical’. Such an approach should equally well map (1) the course of test data fixing the 3D fracture body, (2) the course of test data in the Principal Stress plane (bias 2D cross-section of the 3D fracture body), and (3) of the test data course along the two 3D-test meridians TM and CM.*

In this context: Engineering mapping has basically to capture physics, must be simple and understandable and shall use measurable parameters. Therefore, concerning all part figures 2D and 3D, these show that SFC-models applied for mapping can be good compromises, only. All theoretical approaches have their applicability limits and the very difficult 3D-testing as well

- * Fracture initiation in solid mechanics is given, if the multi-stress state vector touches the surface of the fracture body which represents the surface of all failure 1D-, 2D- and 3D-failure stress vectors. The fracture body surface is defined by a material stressing effort $E_{ff} = 100\% = 1$.
- * A display using a confining stress (TM: $\sigma_{II} = \sigma_{III}$, CM: $\sigma_I = \sigma_{II}$) as coordinate leads to another mapping figure than the Haigh-Lode-Westergaard coordinates give.
- * Using just TM or just CM test data cannot result in a reliable physical fracture body.
- * Of course, general 3D-failure stress states may not lie on TM (30°) or CM (-30°) but on another Lode angle around the hoop.
- * The fracture planes of TM and CM are different.
- * Both, the different course of the test data points compared to the also incorporated ellipse in the principal stress plane and the difference of the TM and the CM-curve document the inherent 120° -rotational symmetry of isotropic materials. ($360^\circ/3 = 120^\circ$ is given, because all 3 principal stresses are of equal mechanical importance, see Fig.14-9).
- * The fracture body of a dense isotropic material has an open bottom fracture surface!

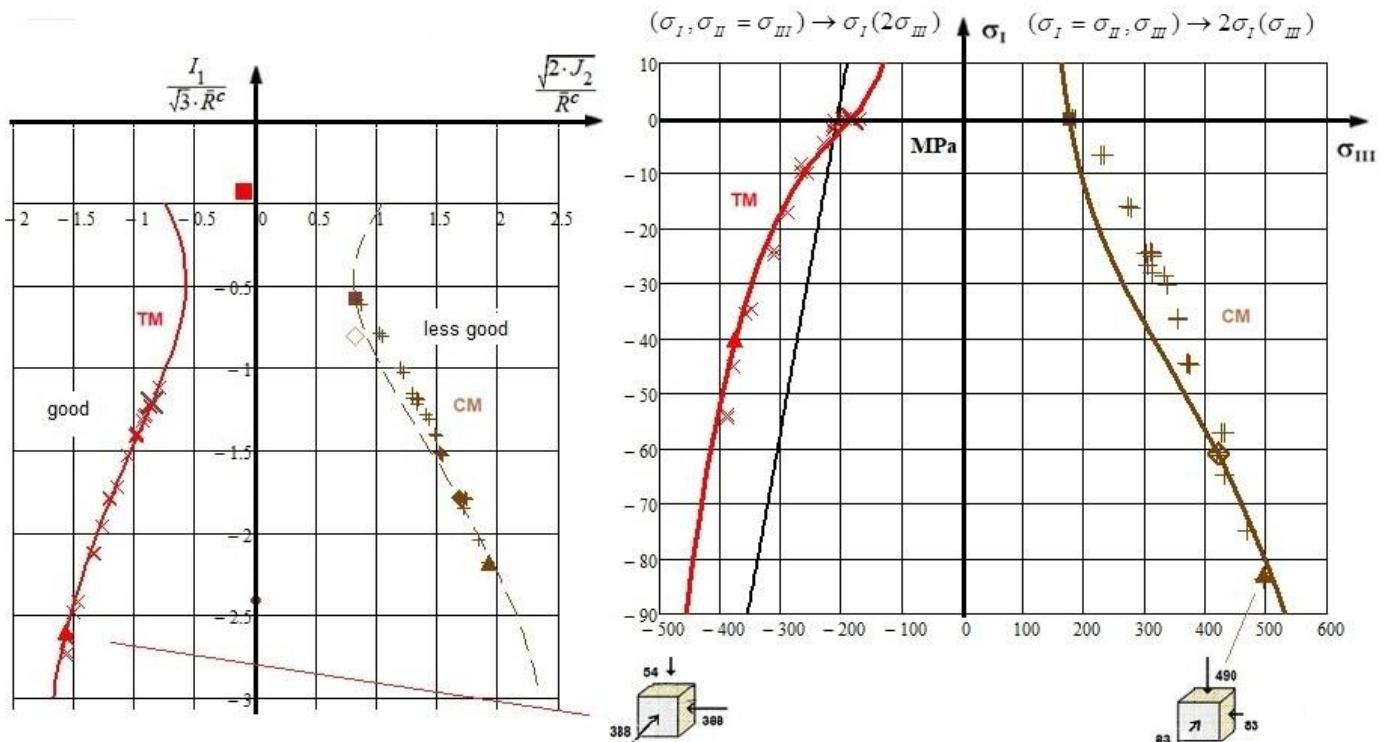


Fig. 26-6, UHPC: (left) mapping display of the two test data sets in Haigh-Lode-Westergaard coordinates. (right) Display using confining stress coordinates

Eventually, Table 26-4 presents the determination of the 5 UHPC model parameters.

Table 26-4, UHPC: Determination of the SFC model parameters

$$F_{\tau} = \frac{c1 \cdot 6J2 \cdot \Theta_{\tau}}{2Rc^2} + c2 \cdot \frac{I1}{Rc} + c3 \cdot \left(\frac{I1}{Rc}\right)^2 + c4 \cdot \left(\frac{I1}{Rc}\right)^3 = 1$$

$$I1 = \sigma I + \sigma II + \sigma III \quad J2 = \frac{(\sigma I - \sigma II)^2 + (\sigma II - \sigma III)^2 + (\sigma III - \sigma I)^2}{6}$$

$$J3 = \frac{(2 \cdot \sigma I - \sigma II - \sigma III) \cdot (2 \cdot \sigma II - \sigma I - \sigma III) \cdot (2 \cdot \sigma III - \sigma I - \sigma II)}{27}$$

$$\Theta_{\tau} = \sqrt[3]{1 + d\tau \cdot 1.5 \cdot 3^{0.5} \cdot J3 \cdot J2^{-1.5}} = \sqrt[3]{1 + d\tau \cdot \sin(3\theta)}$$

$$\Theta_{\tau TM} = \sqrt[3]{1 + d\tau \cdot \sin(3\theta t)} = \sqrt[3]{1 + d\tau} \quad \Theta_{\tau CM} = \sqrt[3]{1 + d\tau \cdot \sin(3\theta c)} = \sqrt[3]{1 - d\tau}$$

$$\theta = \frac{1}{3} \cdot \text{asin} \left[1.5 \cdot 3^{0.5} \cdot \frac{(2 \cdot \sigma I - \sigma II) \cdot (2 \cdot \sigma II - \sigma I) \cdot (-\sigma II - \sigma I)}{27} \cdot \left(\frac{2 \cdot \sigma I^2 - 2 \cdot \sigma I \cdot \sigma II + 2 \cdot \sigma II^2}{6} \right)^{-1.5} \right]$$

Vorgabe $c1_{\text{www}} := 2$

$$c1 \cdot \frac{3 \cdot J2c \cdot \sqrt[3]{1 - 1 \cdot d\tau}}{Rc^2} + c2 \cdot \frac{I1c}{Rc} + c3 \cdot \left(\frac{I1c}{Rc}\right)^2 + c4 \cdot \left(\frac{I1c}{Rc}\right)^3 = 1$$

$$c1 \cdot \frac{3 \cdot J2cc \cdot \sqrt[3]{1 + 1 \cdot d\tau}}{Rc^2} + c2 \cdot \frac{I1cc}{Rc} + c3 \cdot \left(\frac{I1cc}{Rc}\right)^2 + c4 \cdot \left(\frac{I1cc}{Rc}\right)^3 = 1$$

$$c1 \cdot \frac{3 \cdot J2f \cdot \sqrt[3]{1 + 1 \cdot d\tau \cdot \sin(3\theta f)}}{Rc^2} + c2 \cdot \frac{I1f}{Rc} + c3 \cdot \left(\frac{I1f}{Rc}\right)^2 + c4 \cdot \left(\frac{I1f}{Rc}\right)^3 = 1$$

$$c1 \cdot 3 \cdot \frac{J2vc1}{Rc^2} \cdot \sqrt[3]{1 - 1 \cdot d\tau} + c2 \cdot \frac{I1vc1}{Rc} + c3 \cdot \left(\frac{I1vc1}{Rc}\right)^2 + c4 \cdot \left(\frac{I1vc1}{Rc}\right)^3 = 1$$

$$c1 \cdot 3 \cdot \frac{J2v}{Rc^2} \cdot \sqrt[3]{1 + 1 \cdot d\tau} + c2 \cdot \frac{I1v}{Rc} + c3 \cdot \left(\frac{I1v}{Rc}\right)^2 + c4 \cdot \left(\frac{I1v}{Rc}\right)^3 = 1$$

$A_{\tau} := \text{Suchen}(c1, d\tau, c2, c3, c4)$

Table 26-5 follows with the derivation of the mapping curves in confining stress coordinates from the model parameters for the full UHPV fracture body.

Table 26-5: Relationships to derive mapping with the confining stress coordinates

| TM | Transfer to confing stress coordinates | CM |
|--|---|---|
| $\sigma_{II} = \sigma_{III}$ | $J_2 = \frac{(\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2}{6}$ | $\sigma_{II} = \sigma_I$ |
| | same function | |
| | $J_2 = \frac{(\sigma_I - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2}{6}$ | for TM and CM equal ! |
| $J_2 = \frac{Rc^2 \cdot x^2}{2}$ | | |
| $I_{It} = \sqrt{3} \cdot Rc \cdot y_t$ | $x^2 = \frac{[(\sigma_I - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2]}{3 \cdot Rc^2}$ | |
| $I_{II} = \sigma_I + \sigma_{II} + \sigma_{III}$ | $y_t = \frac{I_{It}}{\sqrt{3} \cdot Rc} = \frac{\sigma_{It} + 2\sigma_{III}}{\sqrt{3} \cdot Rc}$ | $y_c = \frac{\sigma_{Ic} + 2\sigma_{IIIc}}{\sqrt{3} \cdot Rc}$ |
| | $\sigma_{III} = \frac{\sqrt{3} \cdot Rc \cdot y_t}{2} - \frac{\sigma_{It}}{2}$ | |
| | $x_{TM} = \sqrt{\frac{\left[\sigma_{It} - \left(\frac{\sqrt{3} \cdot Rc \cdot y}{2} - \frac{\sigma_{It}}{2}\right)\right]^2 + \left(\frac{\sqrt{3} \cdot Rc \cdot y}{2} - \frac{\sigma_{It}}{2} - \sigma_{It}\right)^2}{3 \cdot Rc^2}}$ | $= \sqrt{\frac{\frac{1}{2} \cdot (3 \cdot \sigma_{It} - \sqrt{3} \cdot Rc \cdot y)^2}{3 \cdot Rc^2}}$ |
| | $x_{CM} = \sqrt{\frac{\left[\sigma_{Ic} - (\sqrt{3} \cdot Rc \cdot y - 2 \cdot \sigma_{Ic})\right]^2 + \left(\sqrt{3} \cdot Rc \cdot y - 2 \cdot \sigma_{Ic} - \frac{\sigma_{Ic}}{2} - \sigma_{Ic}\right)^2}{3 \cdot Rc^2}}$ | $= \sqrt{\frac{2 \cdot (3 \cdot \sigma_{Ic} - \sqrt{3} \cdot Rc \cdot y)^2}{3 \cdot Rc^2}}$ |
| Values for x are given by inserting the y-coordinate into the SFC | | |
| | $j = 0, 1.. 40$ | $\frac{y_j}{10} = \frac{j}{10} - 1$ y runs negative |
| $x_{TM_j} = \sqrt{\frac{2 \cdot [c_2 \cdot Rc^2 \cdot I_{Ij} - Rc^3 + c_3 \cdot Rc \cdot (I_{Ij})^2 + c_4 \cdot (I_{Ij})^3]}{3 \cdot Rc^3 \cdot c_1 \cdot \Theta_{TM}}}$ | | $x_{CM_j} = \sqrt{\frac{2 \cdot [c_2 \cdot Rc^2 \cdot I_{Ij} - Rc^3 + c_3 \cdot Rc \cdot (I_{Ij})^2 + c_4 \cdot (I_{Ij})^3]}{3 \cdot Rc^3 \cdot c_1 \cdot \Theta_{CM}}}$ |
| Finally the y-value is inserted into the confing stress version of x x_{TM}, x_{CM} are known | | |
| Vorgabe $\sigma_{It} := -100$ | | $x_{TM} = \sqrt{\frac{\left[\sigma_{It} - \left(\frac{\sqrt{3} \cdot Rc \cdot y}{2} - \frac{\sigma_{It}}{2}\right)\right]^2 + \left(\frac{\sqrt{3} \cdot Rc \cdot y}{2} - \frac{\sigma_{It}}{2} - \sigma_{It}\right)^2}{3 \cdot Rc^2}}$ |
| $\sqrt[3]{V(x_{TM}, y)} := \text{Suchen}(\sigma_{It})$ | $\sqrt[3]{V_j} := V(x_{TM_j}, y_j)$ | $\sigma_{III_j} := \frac{\sqrt{3} \cdot Rc \cdot y_j}{2} - \frac{\sigma_{It_j}}{2}$ |
| Mathcad did not compute $\sqrt[3]{V_j}$, however the formula can be simplified and σ_I extracted | | |
| $\sigma_{It_j} := \frac{\sqrt{6} \cdot Rc \cdot x_{TM_j}}{3} + \frac{\sqrt{3} \cdot Rc \cdot y_j}{3}$ | $\sigma_{Ic_j} := \frac{\sqrt{3} \cdot Rc \cdot y_j}{3} + \frac{\sqrt{2} \cdot \sqrt{3} \cdot Rc \cdot x_{CM_j}}{6}$ | $\sigma_{IIIc_j} := \sqrt{3} \cdot Rc \cdot y_j - 2 \cdot \sigma_{Ic_j}$ |
| $\sigma_{III_t_j} := \frac{\sqrt{3} \cdot Rc \cdot y_j}{2} - \frac{\sigma_{It_j}}{2}$ | | |

25.3 Rock Material, example Sandstone

As for concrete the properties for Underground Rock Failure Stress Analysis are also provided by tests on the tensile and the compressive meridian.

Tensile domain:

Also in rock materials in the vicinity of excavations and boreholes tensile stresses will occur. Further, an undesirable brittle sudden failure is to prevent when a bore-hole is drilled. Therefore, a tensile strength proof requires a tensile strength \bar{R}^t for the distinct rock material.

An estimation for the tensile strength value delivers the Brazilian splitting test (*indirect* tensile strength test) because a classical tensile test specimen is merely to obtain. A solid cylinder or disk (short cylinder) test specimens is used for the initially crack-free (intact) material, see [Fig.26-7](#). The evaluation is performed via the formula $f_{sp} = \bar{R}^t = 2 \cdot q / (\pi \cdot d \cdot \ell)$, from [[The constructor.org](#)].

This ‘indirect’ measurement caused researchers to predict a value by using a Mohr-Coulomb-based SFC but the determined value must be wrong. In this context the author fully supports Mingqing You [[You15](#)] that a tensile strength R^t is a separate parameter and cannot be estimated by models working in the tensile-compressive transition zone. A real value for \bar{R}^t is only to obtain by a uniaxial tensile stress test $\{\sigma\} = (\sigma_{ax}^t = F^t/A, 0, 0)^T$

Compressive domain:

Usual test series for concrete material (*see the concrete applications before*) are most often performed along CM and not so often along TM. For the general demonstration of the strength capacity, however, the full fracture failure body is required because all mixed 3D-compressive stress states are principally possible and their failure stress vectors determine the surface of the fracture body.

In rock mechanics the stress situation seems to be normally just linked to stress states along the compressive meridian. This explains why no bi-axial strength \bar{R}^{cc} is provided in rock literature an entity that enables to describe the 120°-symmetry. Mapping just the course of test data along a meridian simplifies the task: Just the functional description of the test meridian remains of interest.

A stress state in a material, formulated in Mohr’s *mathematical* stresses, reads

$$\{\sigma\} = (\sigma_I, \sigma_{II}, \sigma_{III})^T \text{ with } \sigma_I \text{ becoming the smallest failure stress (most positive)}$$

$$\sigma_I > \sigma_{II} > \sigma_{III} \text{ mathematically and } \sigma_{III} \text{ the largest compressive failure stress (most negative).}$$

Tensile stresses must be signed positive in this context, otherwise confusion becomes extreme!

$$\text{For the tensile meridian follows } \{\sigma\} = (\sigma_I, \sigma_{II}, \sigma_{III} = \sigma_{II})^T \text{ with } \sigma_I = \sigma_{ax}^t - p_{hyd}$$

$$\text{and the compressive meridian } \{\sigma\} = (\sigma_I = \sigma_{II}, \sigma_{II}, \sigma_{III})^T \text{ with } \sigma_{III} = \sigma_{ax}^c - p_{hyd}.$$

TM captures \bar{R}^{cc} (and \bar{R}^t , in the domain of the Normal Fracture mode) and CM captures \bar{R}^c (and principally also \bar{R}^{tt} in the domain of the Normal Fracture mode).

In rock mechanics, being one part of civil engineering, hydrostatic pressure is used, when testing concrete and UD material, but is to replace by the term Confining Pressure CP. This makes to

introduce some definitions of rock mechanics terms: In construction, tensile stress is usually still negative, but not always. This makes literature interpretations difficult!

- Multi-axial rock compressive strength capacity [You15]
(the stress-sketch in Figure 1 of [Lan19] must be corrected. It does not fit to the provided failure stress states. In Fig.26-7 this is corrected)

$$\sigma_{III} \equiv \sigma_s \equiv \sigma_1 \text{ termed here min or principal stress}$$

- 1D uniaxial strengths: $UTS = \bar{R}^t$, $UCS = \bar{R}^c$
Unfortunately the author found different meanings: In engineering design dimensioning UTS means Ultimate Tensile Strength and not Uniaxial Tensile Strength and UCS ultimate compressive strength (still also applied in 'geo engineer'! Why is it not generally used in rock mechanics?) and not for instance Unconfirmed Compressive Strength [Wikipedia]. UCS stands for the maximum axial compressive stress that a specimen can bear under zero Confining Pressure (compressive stress), which means it is nothing else than the usual simple standardized technical compression strength \bar{R}^c in engineering.

- ✓ Confining pressure CP: maximum level of hydrostatic compression applied in a tri-axial compression test of a concrete, a rock material or a neat resin test specimen defined by

$$\{\sigma\} = (\sigma_{ax}^t - CP, -CP, -CP) \text{ or with } \sigma_{ax}^c \text{ (tensile meridian)}$$

$$\{\sigma\} = (\sigma_{ax}^c - CP, -CP, -CP) \text{ (induced by test rig brushes in case of concrete)}$$

- ✓ Confining lithostatic pressure: $CP = p_{hyd} + \text{overlying weight}$.

The author would like to conclude: *Using usual mathematical stresses it reads*

- Sealed, polished dog-bone test specimens deliver the failure stress points
($-\bar{R}^{ccc}, -\bar{R}^{ccc}, -\bar{R}^{ccc}$) no pore pressure, ($-\bar{R}^c, 0, 0$), ($\bar{R}^t, 0, 0$) and further multi-axial compressive failure stresses on the compressive meridian.
- A bi-axial compressive failure stress ($-\bar{R}^{cc}, -, 0$) is obtainable by the dog-bone test specimen for $\sigma_1 = 0$ or $\sigma_{ax}^t = -CP$. However, the author did not find one single bi-axial strength value \bar{R}^{cc} in the papers he examined!
However, the UHPC fracture stress data set, thankfully left by IFM Dresden, brought a statistically good base which should have a similar tendency as rock material
- A bi-axial tensile failure stress ($\bar{R}^{tt}, \bar{R}^{tt}, 0$) can be obtained by cube test specimens prepared by a good gluing in order to load the needed bi-axial tensile stresses.

Test procedure: The confining pressure CP is achieved and then kept constant during the test. The axial stress σ_1 is increased at a certain rate until the test specimen fails at $\max \sigma_1$. It is to consider whether the porosity of the rock or the soil material and the moisture saturation plays a role.

Fig.26-7 presents fracture pictures of the investigated Berea sandstone. Essential is that the fracture angle increases with CP.

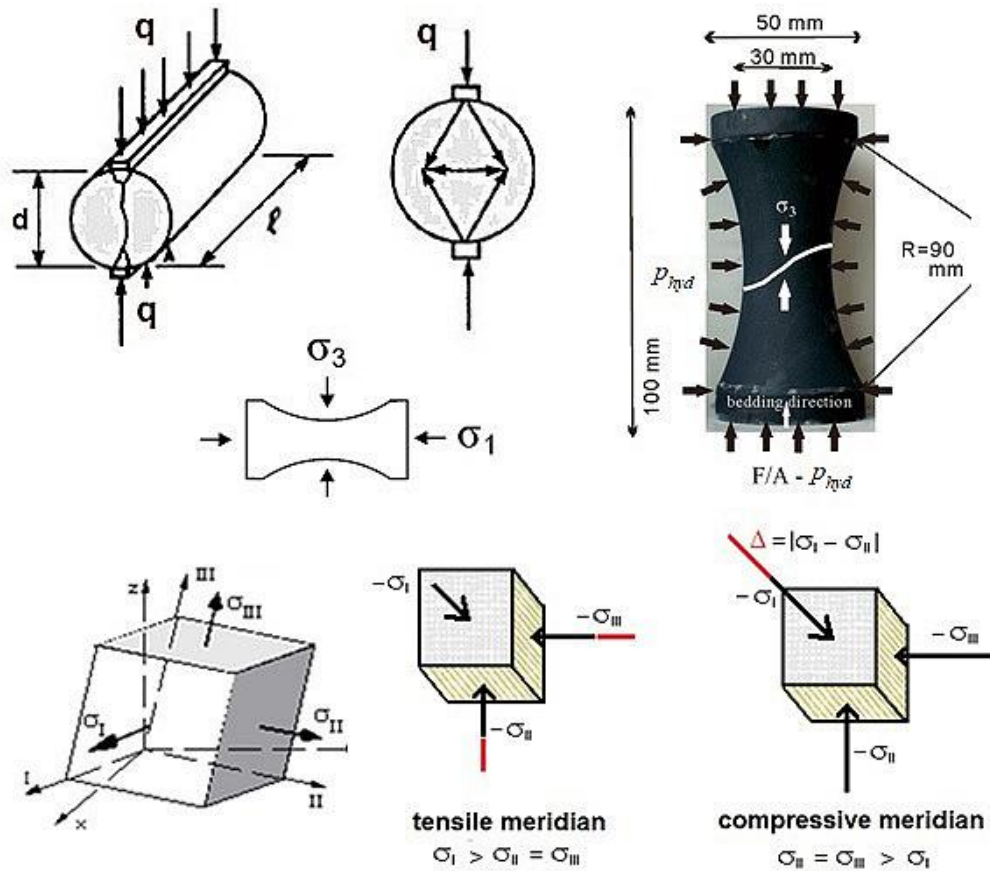


Fig. 26-7: Brazilian cylinder or disk (short length) for an indirect estimation of \bar{R}^I and dog-bone (sealed, highest preparation effort, grinding from solid block with axial bedding layers) test specimen for direct measurement of tri-axial fracture stress states along compressive meridian including the tension-compression domain.

(Δ depicts the differential stress entity causing shear stress with shear deformation)

Similarly to other brittle materials the task always is the full (onset-of-fracture) body surface capturing NF and SF and not by SF just $I_1 < 0$ and thereby not regarding the intrinsic 120° -rotational symmetry. The TM and CM test data points are two oppositely located cross-sections of the body. The classical type of visualization is to use the Haigh-Lode-Westergaard coordinates count equally in all directions. Visualization by using a confining stress cannot lead to the same mapping curve (see Fig. 26.6).

LL:

The interpretation of the concrete-diagrams above leads to the following results for rock materials:

- Using just TM or just CM test data incorporates a bottleneck concerning the achievement of a reliable physical fracture body
- The use of the geo-Mohr-Coulomb Criterion leads to a straight mapping of the course of test data along the tensile meridian. The model of the author captures the curved course
- Engineers in other disciplines become pretty stressed because we civil engineers unfortunately use construction design tools which still call tensile stresses negative stresses. This completely disturbs the logic of the well-known 'civil engineer' A. Mohr in context with his use of mathematical stresses!



Fig.26-8, Sandstone: Fracture pictures of Berea sandstone from [Lan19].

Personal experience with a dangerous shear strength value and an associated critical sliding angle, were leading to a land slide at the West-East Main road of Bhutan.
A video clip taken by me would show how huge rocks were 'travelling' down.



This dramatic situation, depicted in the figure above, led to my most dangerous car trip, on gravel roads, along gorges up to 1000 m deep, from the Central Himalaya down to the plain and back up into the high mountains. AND, there was with just one driver from 7 until 23 o'clock.

27 UD-Strength Failure criteria: Which one should I take?

Aim: Assisting the user not to follow the FE Manual recommendation "Take the worst result of all".

In the future, we will be forced to compute 3D-based reserve factors in static component Design Verification. The 2D-based Classical Laminate Theory for unidirectional fiber-reinforced matrices is not sufficient for this. For these reasons, the author has tried to compare those SFCs that were 'contributing' to the World Wide Failure Exercises (WWFE) for UD materials, namely Tsai-Wu, Hashin, Puck and Cuntze. The comparison carried out (*generally too little test data is available*) looks at the necessary input, shows the received failure envelopes for three 2D stress combinations and tries to evaluate the results, so that FE Manual recommendations "Take the worst result of all" is not to be followed anymore!

Regarding the chapters before, the SFCs of Hashin and Tsai-Wu will be presented, only, and some missing things of Puck's SFC.

27.1 SFC Hashin

* Hypothesis 2, valid for Cuntze's FMC-bases SFC-formulations:

"For UD-material the SFCs should be invariant under any rotation around the fiber direction."

Hashin with the Hypothesis 2 also proposed an invariant-based global quadratic approach with two different stress invariants:

$$I_1 = \sigma_1, I_2 = \sigma_2 + \sigma_3, I_3 = \tau_{31}^2 + \tau_{21}^2, I_4 = \tau_{23}^2 - \sigma_2 \cdot \sigma_3, I_5 = 4\tau_{23}\tau_{31}\tau_{21} - \sigma_2 \cdot \tau_{31}^2 - \sigma_3 \cdot \tau_{21}^2.$$

Table 27-1 compiles the four SFCs of Zvi Hashin.

Table 27-1: Four SFCs, for FF1, FF2, IFF1 and IFF2

| |
|--|
| $\{\sigma\} = (\sigma_1, \sigma_2, \sigma_3, \tau_{23}, \tau_{31}, \tau_{21})^T, \quad \{\bar{R}\} = (\bar{R}_{\parallel}^t, \bar{R}_{\parallel}^c, \bar{R}_{\perp}^t, \bar{R}_{\perp}^c, \bar{R}_{\perp\parallel}^c; \bar{R}_{23})^T ; 6 \text{ strengths, principally}$ <p>Interaction of the 4 modes necessary.</p> <p>Hypothesis 1: $F(\{\sigma^A\}, \{\bar{R}^A\}, \theta_{fp}) = 1$, Puck's way</p> <p>Hypothesis 2: $F(\{\sigma\}, \{\bar{R}\}) = 1$, Cuntze's way, below</p> <p>FF1, $\sigma_1 > 0$: $\left(\frac{\sigma_1}{\bar{R}_{\parallel}^t}\right)^2 + \frac{\tau_{31}^2 + \tau_{21}^2}{\bar{R}_{\perp\parallel}^2}$; FF2, $\sigma_1 < 0$: $\left(\frac{-\sigma_1}{\bar{R}_{\parallel}^c}\right)^2 = 1$,</p> <p>IFF1, $\sigma_2 + \sigma_3 > 0$: $\frac{(\sigma_2 + \sigma_3)^2}{\bar{R}_{\perp}^t} + \frac{(\tau_{23}^2 - \sigma_2 \cdot \sigma_3)}{\bar{R}_{23}^2} + \frac{(\tau_{31}^2 + \tau_{21}^2)}{\bar{R}_{\perp\parallel}^2} = 1$,</p> <p>IFF2, $\sigma_2 + \sigma_3 < 0$: $\left(\frac{\bar{R}_{\perp}^c}{4 \cdot \bar{R}_{23}^2} - 1\right) \cdot \frac{(\sigma_2 + \sigma_3)}{\bar{R}_{\perp}^c} + \frac{(\sigma_2 + \sigma_3)^2}{4 \cdot \bar{R}_{23}^2} + \frac{(\tau_{23}^2 - \sigma_2 \cdot \sigma_3)}{\bar{R}_{23}^2} + \frac{(\tau_{31}^2 + \tau_{21}^2)}{\bar{R}_{\perp\parallel}^2} = 1$,</p> <p>Interlaminar failure: $\sigma_3 > 0$: $\left(\frac{\sigma_3}{\bar{R}_3^t}\right)^2 = 1$; $\sigma_3 < 0$: $\left(\frac{-\sigma_3}{\bar{R}_3^c}\right)^2 = 1$.</p> |
|--|

* Hypothesis 1, valid for Puck's Action Plane IFF formulation:

"In the event that a failure plane under a distinct fracture angle can be identified, the failure is produced by the normal and shear stresses on that plane".

Hashin proposed this modified Mohr-Coulomb IFF approach but did not pursue this idea due to numerical difficulties (*A. Puck succeeded on this way*).

Question: What about the determination of $\bar{R}_{23} \neq \bar{R}_{23}^t$? See *Technical Terms*, please.

27.2 SFC Tsai-Wu, global SFC

A general anisotropic tensor polynomial expression of Zakharov and Goldenblat-Kopnov with the parameters F_i, F_{ij} as strength model parameters was the basis of the Tsai-Wu SFC $\sum_{i=1}^6 (F_i \cdot \sigma_i) + \sum_{j=1}^6 \sum_{i=1}^6 (F_{ij} \cdot \sigma_i \cdot \sigma_j) = 1$. From this tensor formulation, Tsai-Wu used the linear and quadratic terms, see Table 27-2:

Table 27-2: 3D SFCs of Tsai-Wu

| |
|--|
| $\{\sigma\} = (\sigma_1, \sigma_2, \sigma_3, \tau_{23}, \tau_{31}, \tau_{21})^T, \quad \{\bar{R}\} = (\bar{R}_{\parallel}^t, \bar{R}_{\parallel}^c, \bar{R}_{\perp}^t, \bar{R}_{\perp}^c, \bar{R}_{\perp\parallel}, \bar{R}_{23})^T, \text{ 6 strengths}$ $F(\{\sigma\}, \{\bar{R}\}) = 1. \text{ The interaction is global SFC - inert}$ $F_i \cdot \sigma_i + F_{ij} \cdot \sigma_i \cdot \sigma_j = 1 \quad \text{with } (i,j = 1,2,..6) \quad \text{or executed}$ $F_{11} \cdot \sigma_1^2 + F_1 \cdot \sigma_1 + 2F_{12} \cdot \sigma_1 \cdot \sigma_2 + 2F_{13} \cdot \sigma_1 \cdot \sigma_3 + F_{22} \cdot \sigma_2^2 + F_2 \cdot \sigma_2 +$ $+ 2F_{23} \cdot \sigma_2 \cdot \sigma_3 + F_{33} \cdot \sigma_3^2 + F_3 \cdot \sigma_3 + F_{44} \cdot \tau_{23}^2 + F_{55} \cdot \tau_{13}^2 + F_{66} \cdot \tau_{12}^2 = 1$ <p style="text-align: center;">with the strength model parameters</p> $F_1 = 1/\bar{R}_{\parallel}^t - 1/\bar{R}_{\parallel}^c, \quad F_{11} = 1/(\bar{R}_{\parallel}^t \cdot \bar{R}_{\parallel}^c), \quad F_2 = 1/\bar{R}_{\perp}^t - 1/\bar{R}_{\perp}^c, \quad F_{22} = 1/(\bar{R}_{\perp}^t \cdot \bar{R}_{\perp}^c) = F_{33},$ $F_{13} = F_{12}, \quad F_{55} = F_{66} = 1/\bar{R}_{\perp\parallel}^2, \quad 2F_{23} = 2F_{22} - 1/\bar{R}_{23}^2, \quad F_{44} = 2 \cdot (F_{22} + F_{23})$ <p>and - in order to avoid an open failure surface - the so-called interaction term</p> $F_{12} = \bar{F}_{12} \cdot \sqrt{F_{11} \cdot F_{22}} \quad \text{with } -1 \leq \bar{F}_{12} \leq 1; \quad \text{usually applied } F_{12} = -0.5.$ |
|--|

Question, again: What about the determination of \bar{R}_{23} and the value for F_{12} for 3D applications?

27.3 SFC Puck

Some history:

*As early as 1969 A. Puck recognized to separate FF from IFF (not Hashin as is sometimes said). Since the mid-eighties Puck from Uni Kassel, Cuntze from MAN and colleagues of the DLR-Braunschweig looked together for an improved IFF-SFC.

* H. Schuermann, Uni Darmstadt, found the article [Has80] with the Hashin Hypothesis 1 which Puck could successfully execute. Cuntze recommended to use the matrix formulation to mathematically simpler convince the reader, which was more successful than his excellently written model description.

* Beside several dissertation works, Puck's IFF model was further developed in a founded research project 1994. Results were published in VDI Progress Reports Series 5 Vol.506, VDI-Verlag, Düsseldorf, 1997, [VDI97]. The investigations for this book gave valuable results for Puck's book, 1996.

* Due to the still highly established Puck IFF model Cuntze invited Puck to put his SFC into the [VDI 2014] German Guideline, Sheet 3, Development of Fibre-Reinforced Plastic Components, Analysis.

Puck's so-called Action Plane IFF Conditions (1991) base on Mohr-Coulomb and Hashin.

In his interaction approach for the 3 IFF modes Puck interacted the 3 Mohr stresses σ_n , τ_{nt} , τ_{nl} on the IFF fracture plane, see [Fig.27-4](#). He uses parabolic or elliptic polynomials to formulate a so-called master fracture body in the $(\sigma_n, \tau_{nt}, \tau_{nl})$ space. Thereby he assumes that a compressive σ_n cannot cause fracture on its action plane and that the stress σ_1 does not have any influence on the angle of the IFF fracture plane. The stresses on the fracture plane are decisive for fracture: A tensile stress σ_n supports the fracture, while in contrast a compressive stress makes the material 'stronger'. In other words: A compressive σ_n impedes IFF which is caused by the action plane shear stresses τ_{nt} and τ_{nl} , or – in other words - cannot cause fracture on its action plane. Fracture-responsible are only those stresses which act on a common action plane.

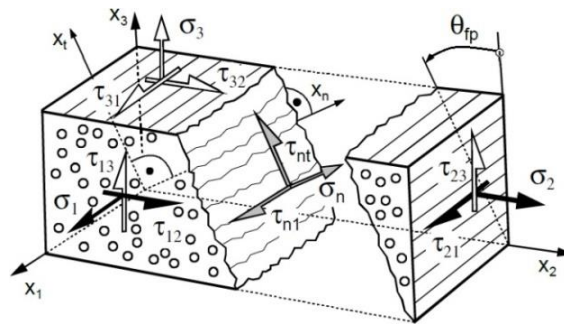


Fig.27-4, UD-composite element: Lamina and action plane stresses at an inclined failure angle θ_{fp} (from [Lut05, SAMPE])

[Fig.27-5](#) presents Puck's 3 IFF modes: mode A (= IFF1), mode B (= IFF3), mode C (= IFF2). The modes A and B lead to transversal fracture planes with $\theta_{fp} = 0$, whereas in mode C inclined planes occur $0^\circ < \theta_{fp} < 55^\circ$ (for CFRP). The determination of the unknown IFF action plane angle θ_{fp} is

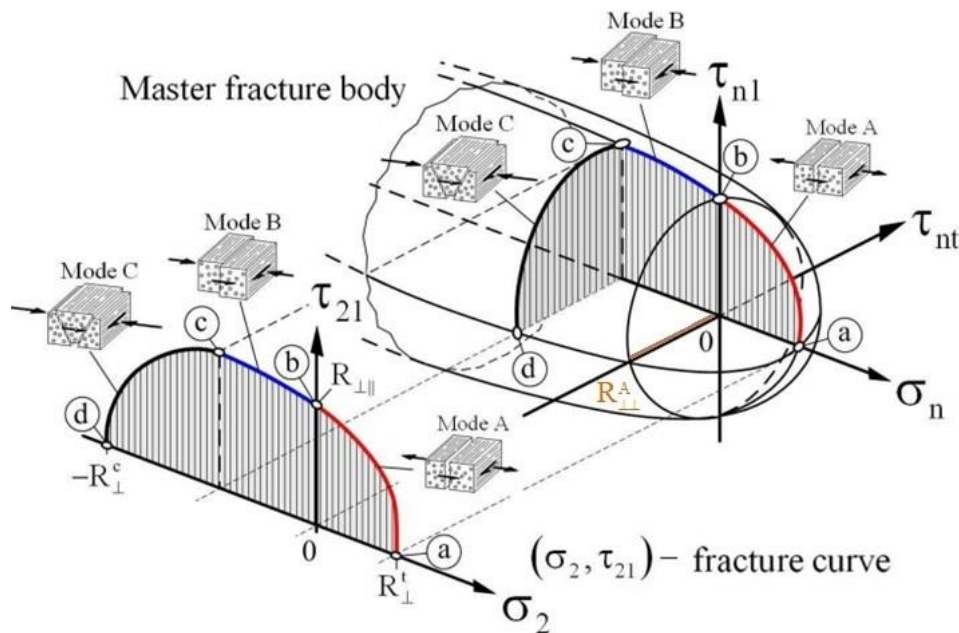


Fig.27-5: Master fracture body with Puck's IFF modes and action plane stresses $(\sigma_n, \tau_{nt}, \tau_{nl})$. (left) Lamina stresses and main IFF cross section of the fracture body in lamina stresses (σ_2, τ_{21}) [courtesy H. Schürmann]

performed by a search process in the domain $-90^\circ < \theta_{fp} < 90^\circ$. For the in-plane stress state $\tau_{21}(\sigma_2)$ which is dominant in many structural components, Puck found an analytic solution for the angle of

the fracture plane [Puc02]:
$$\cos \theta_{fp} = \sqrt{\frac{1}{2 + 2 \cdot p_{\perp\perp}^c} \cdot \left[\left(\frac{\bar{R}_{23}^A}{\bar{R}_{\perp\parallel}} \right)^2 \cdot \left(\frac{\tau_{21}}{\sigma_2} \right)^2 + 1 \right]}$$

In Table 27-3 Puck's Action Plane Mohr-Coulomb-linked (global) IFF SFCs discriminate 3 IFF domains and are completed by the simple maximum stress modes FF1 and FF2. Two IFF fracture plane resistances (superscript ^A) directly are technical strengths.

Table 27-3: SFCs for FF1, FF2, IFF1, IFF2 and IFF3

$\{\sigma\} = (\sigma_1, \sigma_2, \sigma_3, \tau_{23}, \tau_{31}, \tau_{21})^T$, $\{\bar{R}\} = (\bar{R}_{\parallel}^t, \bar{R}_{\parallel}^c, \bar{R}_{\perp}^t, \bar{R}_{\perp}^c, \bar{R}_{\perp\parallel}; \bar{R}_{23}^A)^T$, 6 strengths, principally

In Mohr's action plane stresses the IFF-SFC reads $F(\sigma_n, \tau_n, \bar{R}_\sigma, \bar{R}_\tau, \theta_{fp}) = 1$,

$F(\{\sigma^A\}, \{\bar{R}^A\}, \theta_{fp}) = 1$ with $\{\bar{R}\} = (\bar{R}_{\parallel}^t, \bar{R}_{\parallel}^c, \bar{R}_{\perp}^A = \bar{R}_{\perp}^t, \bar{R}_{23}^A, \bar{R}_{\perp\parallel}^A = \bar{R}_{\perp\parallel}^t)^T$; Puck: $\bar{R}_{23}^A \neq \bar{R}_{23}$!

FF1, $\sigma_1 > 0$: $\left(\frac{\sigma_1}{\bar{R}_{\parallel}^t} \right)^2$; FF2, $\sigma_1 < 0$: $\left(\frac{-\sigma_1}{\bar{R}_{\parallel}^c} \right)^2 = 1$, (maximum stress criteria)

and due to the IFF hypotheses, two different equations are provided [Puc 96, p.118]

IFF: $\sigma_n > 0$: $\varepsilon = \left(\frac{\tau_{nt}}{\bar{R}_{23}^A} \right)^2 + \left(\frac{\tau_{nl}}{\bar{R}_{\perp\parallel}^A} \right)^2 + \left(\frac{\sigma_n}{\bar{R}_{\perp}^A} \right)^2 = 1$, $\tau_n = \sqrt{\tau_{nt}^2 + \tau_{nl}^2}$

IFF: $\sigma_n < 0$: $\varepsilon = \left(\frac{\tau_{nt}}{\bar{R}_{23}^A - p_{\perp\perp}^c \cdot \sigma_n} \right)^2 + \left(\frac{\tau_{nl}}{\bar{R}_{\perp\parallel}^A - p_{\perp\parallel}^c \cdot \sigma_n} \right)^2 = 1$, [Puc96, p.143]

\Rightarrow from originally assumed 6 material strengths down to 5 action plane resistancies which capture all 3 sub-modes IFF1, IFF2 and IFF3.

The following transfer relationship is to apply above ($_{fp}$ = failure plane)

$$\begin{Bmatrix} \sigma_n(\theta_{fp}) \\ \tau_n(\theta_{fp}) \\ \tau_{nl}(\theta_{fp}) \end{Bmatrix} = \begin{bmatrix} c^2 & s^2 & 2sc & 0 & 0 \\ -sc & sc & c^2 - s^2 & 0 & 0 \\ 0 & 0 & 0 & s & c \end{bmatrix} \cdot \begin{Bmatrix} \sigma_2 \\ \sigma_3 \\ \tau_{23} \\ \tau_{31} \\ \tau_{21} \end{Bmatrix}, \quad c = \cos \theta_{fp} \quad \text{and} \quad s = \sin \theta_{fp} .$$

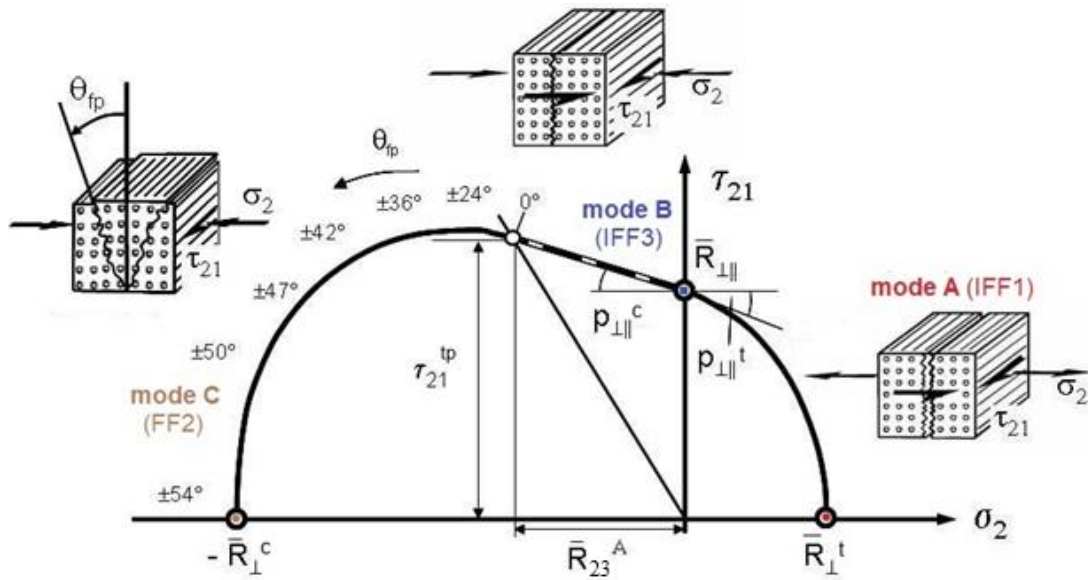


Fig. 27-6: Fracture modes of the (σ_2, τ_{21}) -failure envelope; index^{tp} marks the touchpoint between mode B and C, [Lut13, Puc96]

Table 27-4: 2D-IFF [VDI2014]

| |
|---|
| <p>Mode A (= IFF1) : $\varepsilon = \frac{1}{\bar{R}_{\perp\parallel}} \cdot \sqrt{\left(\frac{\bar{R}_{\perp\parallel}}{\bar{R}_{\perp}^c} - p_{\perp\parallel}^t\right)^2 \cdot \sigma_2^2 + \tau_{21}^2 + p_{\perp\parallel}^t \cdot \sigma_2}$;</p> <p>Mode B ($\cong$ IFF3) : $\varepsilon = \frac{1}{\bar{R}_{\perp\parallel}} \cdot \sqrt{p_{\perp\parallel}^c \cdot \sigma_2^2 + \tau_{21}^2 + p_{\perp\parallel}^c \cdot \sigma_2}$;</p> <p>Mode C ($\cong$ IFF2) : $\varepsilon = \frac{\tau_{21}^2}{4 \cdot (\bar{R}_{\perp\parallel} + p_{\perp\parallel}^c \cdot \bar{R}_{23}^A)^2} \cdot \frac{\bar{R}_{\perp}^c}{-\sigma_2} + \frac{-\sigma_2}{\bar{R}_{\perp}^c}$</p> <p>$\varepsilon$ is also termed f_E [Lut05, VDI2014]</p> <p>$\tau_{21}^{tp} = \bar{R}_{\perp\parallel} \cdot \sqrt{1 + 2 \cdot p_{23}^c}$, $\sigma_2^{tp} = -\bar{R}_{23}^A$, $\bar{R}_{23}^A = \left[\bar{R}_{\perp\parallel} \cdot \sqrt{1 + 2 \cdot p_{\perp\parallel}^c \cdot \bar{R}_{\perp}^c / \bar{R}_{\perp\parallel}} - 1 \right] / 2 \cdot p_{\perp\parallel}^c$,</p> <p>* The action plane resistance \bar{R}_{23}^A depends on the chosen fracture body model such as the parabolic Mohr envelope and not just the linear Mohr approach.</p> <p>* Assumption on coupling the inclination parameters: $p_{23}^c = p_{\perp\parallel}^c \cdot \bar{R}_{23}^A / \bar{R}_{\perp\parallel}$.</p> |
|---|

R_{23}^A is found in the horizontal cross-section of Puck's Master failure body. It is a IFF-Mohr model-linked quantity and consequently a given model strength parameter and not a technical strength. It finally did vanish therefore as a measurable technical strength. Puck's R_{23}^A is a model parameter and defined by Puck's Mohr-Model using two strength and the so-called inclination parameters p , depicted in Fig.6.

Practically, 5 independent failure activating stresses are left, which would support Cuntze's material symmetry-based 'generic' number of 5 he elaborated for UD materials.

Of course, an interaction of IFF with the two FF modes is also with Puck mandatory in order to capture the combined (joint) failure danger. This procedure is documented in detail in the VDI 2014, sheet 3. One reason to do that is that experiments demonstrate micro-damage activation at the ends of broken filaments. Puck terms this ‘weakening of the matrix’ and uses a so-called weakening factor. Applying Cuntze’s interaction equation $Eff = 1$ this is automatically performed in the foreseen comparison.

27.4 Comparison of the obtained different SFC Failure Envelopes

In consequence of the rare test data sets just 2D-models of Tsai-Wu, Hashin, Puck and Cuntze could be numerically investigated.

A comparison is only possible if the interaction can be equally performed for each model and the same interaction. This could be realized for the 4 models by a transfer to the single 2D- Eff -formulation, example Tsai-Wu:

$$\frac{\sigma_1^2/Eff^2}{\bar{R}_{||}^t \cdot \bar{R}_{||}^c} + \frac{\sigma_1}{Eff} \cdot \left(\frac{1}{\bar{R}_{||}^t} - \frac{1}{\bar{R}_{||}^c} \right) + \frac{2F_{12} \cdot \sigma_1 \cdot \sigma_2 / Eff^2}{\sqrt{\bar{R}_{||}^t \cdot \bar{R}_{||}^c \cdot \bar{R}_{\perp}^t \cdot \bar{R}_{\perp}^c}} + \frac{\sigma_2^2/Eff^2}{\bar{R}_{\perp}^t \cdot \bar{R}_{\perp}^c} + \frac{\sigma_2}{Eff} \cdot \left(\frac{1}{\bar{R}_{\perp}^t} - \frac{1}{\bar{R}_{\perp}^c} \right) + \frac{\tau_{12}^2/Eff^2}{\bar{R}_{\perp||}} = 1$$

The investigation focuses mapping of the curves of test data by SFCs. In these formulations each single strength is an average strength consequently indicated by a bar over.

The following figures present the failure envelopes of investigated three plane stress combinations.

SFC Failure Envelopes

Fig.27-6 visualizes, how the four models map the most interesting cross-section of the UD fracture body, namely $\tau_{21}(\sigma_2)$.

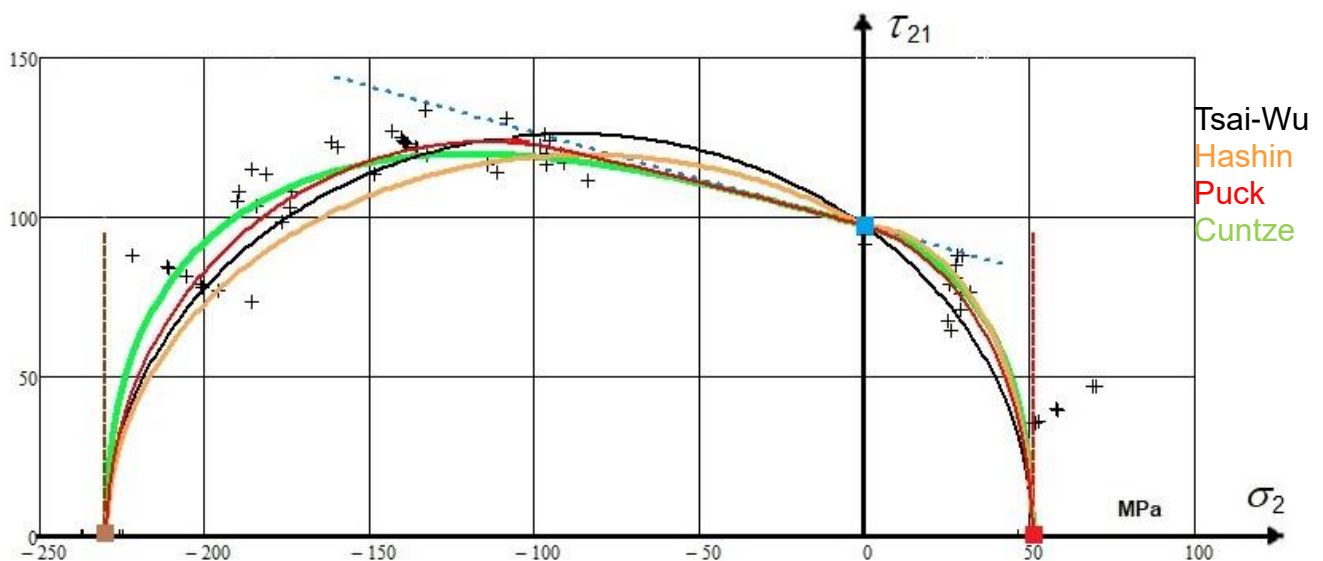


Fig. 27-6: CFRP test results (MAN Technologie research project with A. Puck, IKV Aachen et al.)

$$\{\bar{R}\} = (1280, 800, 51, 230, 97)^T \text{ MPa}, \mu_{\perp||} = 0.3 \text{ [VDI 97]}$$

Fig.27-7 depicts the failure envelope $\sigma_2(\sigma_1)$, being the WWFE Test Case 3. In this test case below, just the 4 strength values were provided together with the not R_{\perp}^c -matching test data of a Russian test data provider showing a test discrepancy (*passed away, no request possible*) in the fourth quadrant of Fig.27-7. However, the tendency of the two different (*assumed*) test sets can be carefully used for validation.

The global Tsai-Wu SFC lies fully outside, which would be of high effect for stability analysis. The test data set in the quadrant III was provided by M.. Knops, IKV Aachen, [*Kno03*]. Mapping tells, that modal modelling is the better choice.

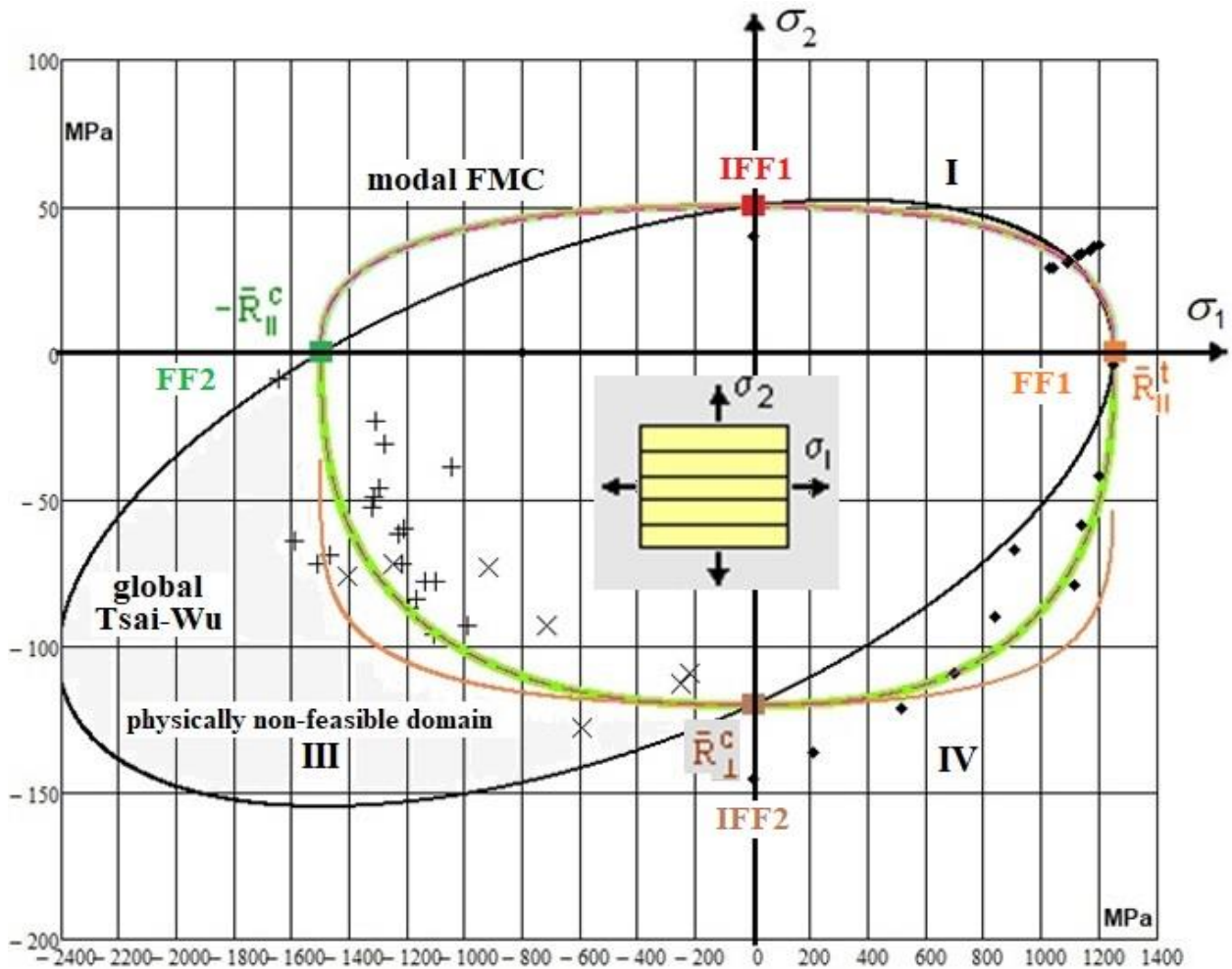


Fig.27-7, WWFE-I, TC3: Hoop wound tube lamina. E-glass/MY750 Ep.

$$\{\bar{R}\} = (1280, 800, 51, 230, 97)^T$$

In order to be able to generate above different envelopes the author had to harmonize terminology and to make them to apply his interaction formula for the modal SFC ones.

This limited the amount of further numerical comparisons. $\tau_{21}(\sigma_1)$ could be investigated.

From Fig.27-8 can be concluded that the envelope of Puck and Cuntze lie upon another. Modal Hashin and Global Tsai-Wu are lying inside.

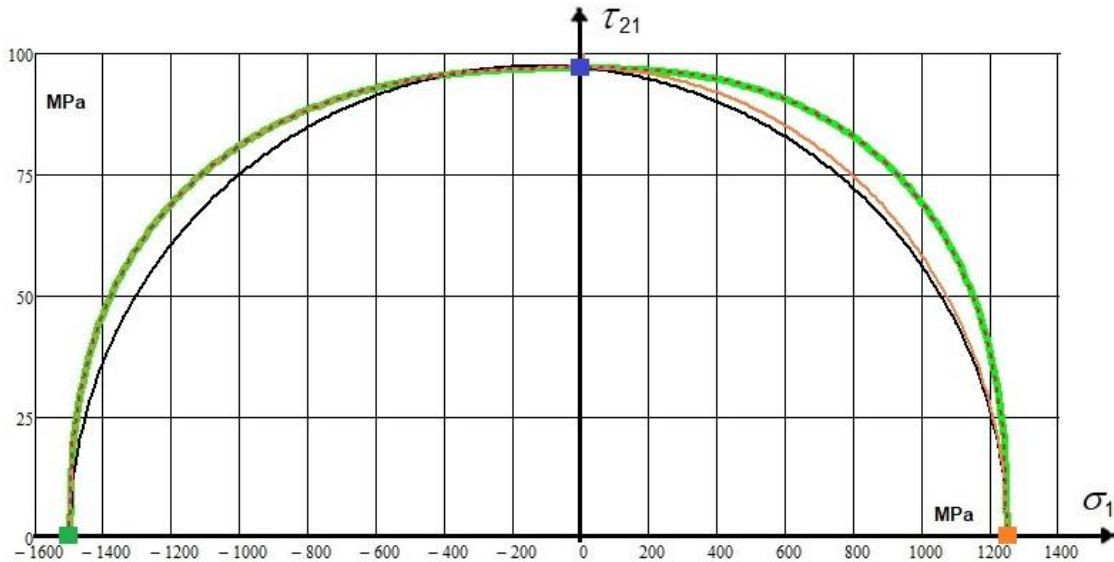


Fig.27-8: $\{\bar{R}\} = (1280, 800, 51, 230, 97)^T$

27.5 Computation of a SFC-linked Reserve Factor

Principally, in order to avoid either to be too conservative or too un-conservative, a separation is required of the always needed ‘analysis of the average structural behavior’ in Design Dimensioning (using average properties and average stress-strain curves) in order to obtain optimum structural information (= 50% expectation value) from the mandatory single Design Verification analysis of the final design, where statistically minimum values for strength and minimum, mean or maximum values for the task-demanded other properties are applied as Design Values. There it is to demonstrate that ‘A relevant Limit State is not met yet’.

A very simple example of the Design Verification of a critical UD lamina in a distinct wall design shall depict the RF-calculation procedure: BeispielCHECK Neu mit Rechnung machen

Assumption: Linear analysis permitted, design Factor of Safety $j_{ult} = 1.25$

- * Design loading (action): $\{\sigma\}_{design} = \{\sigma\} \cdot j_{ult}$
- * 2D-stress state: $\{\sigma\}_{design} = (\sigma_1, \sigma_2, \sigma_3, \tau_{23}, \tau_{31}, \tau_{21})^T \cdot j_{ult} = (0, -60, 0, 0, 0, 50)^T$ MPa
- * Residual stresses: 0 (effect vanishes with increasing micro-cracking)
- * Strengths (resistance): $\{\bar{R}\} = (1378, 950, 40, 125, 97)^T$ MPa averages from measurement
statistically reduced $\{R\} = (R'_{||}, R'_c, R'_\perp, R'_c, R'_{\perp\perp})^T = (1050, 725, 32, 112, 79)^T$ MPa
- * Friction value(s): $\mu_{\perp\perp} = 0.3$, ($\mu_{\perp\perp} = 0.35$), Mode interaction exponent: $m = 2.7$
 $\{Eff^{mode}\} = (Eff^{c\parallel\sigma}, Eff^{r\parallel\tau}, Eff^{c\perp\sigma}, Eff^{r\perp\tau}, Eff^{c\perp\perp})^T = (0.88, 0, 0, 0.21, 0.20)^T$
 $Eff^m = (Eff^{c\parallel\sigma})^m + (Eff^{r\parallel\tau})^m + (Eff^{c\perp\sigma})^m + (Eff^{r\perp\tau})^m + (Eff^{c\perp\perp})^m = 100\%$

The results above deliver the following material reserve factor $f_{RF} = 1 / Eff$

- * $Eff^{c\perp\sigma} = \frac{\sigma_2 + |\sigma_2|}{2 \cdot \bar{R}'_\perp} = 0$, $Eff^{r\perp\tau} = \frac{-\sigma_2 + |\sigma_2|}{2 \cdot \bar{R}'_\perp} = 0.60$, $Eff^{c\perp\perp} = \frac{|\tau_{21}|}{\bar{R}'_{\perp\perp} - \mu_{\perp\perp} \cdot \sigma_2} = 0.55$
 $Eff = [(Eff^{c\perp\sigma})^m + (Eff^{r\perp\tau})^m + (Eff^{c\perp\perp})^m]^{1/m} = 0.80$

$\Rightarrow f_{RF} = 1 / Eff = 1.25 > 1 \rightarrow RF = f_{RF}$ (if linearity permitted): Design Verification achieved!

The certification–relevant load-defined Reserve Factor RF corresponds in the linear case to the material reserve factor f_{RF} . Its value here is $RF = 1.25 > 1 \rightarrow$ Laminate wall design is verified!

The multiple Lessons Learned and conclusions are incorporated in the following list:

LL

- ✓ *Considering FE-results and necessary properties: We must more and more 3D-design! However properties, especially for composites is 3D-property data test sets, are seldom sufficiently available*
- ✓ *So-called global SFCs couple physically different failure modes whereas the modal SFCs describe each single failure mode and therefore will better map the course of test data*
- ✓ *First-Ply-Failure (FPF) envelopes are searched by these SFCs, which means determination of ‘Onset-of-damage’ and includes both Inter Fiber Failure (IFF) and Fiber Failure (FF). Last Ply Failure (LPF) usually requires a non-linear analysis, which can be used to save a design*
- ✓ *Material symmetry seems to require for UD materials a ‘generic’ number of 5, valid for strengths and elasticity properties and the distinct SFCs.*
- ✓ *In this context: The Standard 3D SFCs of Tsai-Wu and Hashin employ the so-called cohesive (shear) strength R_{23} and regard it as a technical strength and not as a general strength quantity. The mystery behind the various interpretations is tried to be unlocked by the author. Because most of the published applications are 2D-ones the employed SFCs do not require R_{23} and its determination by tests needed not to be presented*
- ✓ *Often, SFCs employ just strengths and no friction value. This is physically not accurate and the undesired consequence in Design Verification is: RF may be not on the safe side*
- ✓ *SFCs are ‘just’ necessary but not sufficient for the prediction of strength failure. Basically, due to internal flaws, also an energy criterion is to apply. The novel approach ‘Finite Fracture Mechanics (FFM)’ offers a hybrid criterion to more realistically predict the stress-based crack initiation in brittle isotropic and UD materials.*
- ✓ *The physically clear-based quantity E_{ff} gives an impressive interpretation of the failure envelope or what 100% strength capacity in 3D stress states physically really means.*
- ✓ *From the nevertheless well performed WWFEs the author had to learn that provided test results can be far away from the reality like an inaccurate theoretical model. Theory creates a model of the reality and one experiment shows ‘just’ one realization of the reality. Test article analysis is mandatory to interpret the test results and for a simulation-based improvement of the design. Only well-understood experiments can verify the design assumptions made!*
- ✓ *Assuming 6 strength quantities seems to violate material symmetry. Cuntze’s SFC set just employs 5 measurable strengths and 2 friction parameters.*

The SFC models of Puck and Cuntze are most probably those SFC models, which are best validated by 2D and the few 3D UD experiments.

28 Technical Terms, Laminate Description, Material Stressing Effort *Eff*

Aim: Bridging mutual understanding between engineering disciplines.

28.1 Terms

Some terms for a better common understanding and for the application of SFCs comparison shall be added. This is the more necessary for composites:

- Analysis: Computation that uses fixed model parameters, such as of the final design
- Fracture body: smoothed surface of the ends of the multi-axial failure stress vectors
- Failure condition: Condition on which a failure becomes effective, meaning $F = 1$ for one limit state
- Failure criterion: Distinctive feature defined as a condition for one of the 3 states $F < = > 1$
- Failure Mode Concept (FMC): invariant, failure mode-based general concept to generate strength failure conditions (SFCs) for single failure modes. It is a ‘modal’ formulation in contrast to ‘global’ concepts where all failure modes are mathematically linked and a concept for materials that can be homogenized (smeared). Applicability of a SFC ends if homogenization as pre-requisite of modeling is violated
- Fracture body: smoothed surface of the ends of the multi-axial failure stress vectors
- First-Ply-Failure (FPF): usually First Inter-Fiber-Failure IFF in a lamina of the laminate. FPF failure envelopes are searched by the SFCs. This means determination of ‘Onset-of-damage’ and includes both Inter Fiber Failure (IFF) and Fiber Failure (FF)
- Lamina: Designation of the single UD ply as computational element of the laminate, used as laminate subset or building block for laminate modeling. It might capture several equal plies.
- Last-Ply-Failure (LPF) in the laminate: usually requires a non-linear laminate analysis, which can be used to save a design
- Material Stressing Effort $\sigma = R \cdot Eff$ (not material utilization in the usual sense of manufacture waste minimization): artificial term, generated in the UD World Wide Failure Exercises in order to get an English term for the meaningful German term Werkstoffanstrengung. The SCF is stress-based and not strain –based. In the linear case it is directly valid $f_{Res} = RF = 1 / Eff$. (in his book Puck originally used the term effort ε and further exposure). $Eff_{max} = 100\% = 1$
- Ply, layer: Physical element from a winding, tape-laying process etc
- Properties: ‘Agreed’ values to achieve a common and comparable design basis. Must be provided with average value and coefficient of variation
- Reserve Factor RF: load-defined value $RF_{ult} = final\ failure\ load / design\ ultimate\ load\ DUL$
- (material Reserve factor f_{Res} : $f_{Res} = strength\ design\ allowable\ R / stress\ at\ design\ load\ DUL$
- R : general strength, strength design allowable for Design Verification;
- \bar{R} : average strength in model validation for mapping tasks, marked by the statistical ‘bar over’
- Simulation: Process, that consists of several analysis loops and lasts until the system is imitated in the Design Dimensioning process. The model parameters are adjusted hereby to the ‘real world’ parameter set
- Strength quantities: $\{R\} = (R'_{//}, R^c_{//}, R'_{\perp}, R^c_{\perp}, R_{//\perp}; R_{23})^T \Leftarrow (X, X', Y, Y', S_{12}; S_{23})^T$ Tsai
- Stress components: They should exactly read stress tensor components or simpler just stresses (*only a shear stress can be composed of a tensile component jointly acting with a compressive stress component*)

- Technical strength, standard-measurable: for clarity symbolically indexed $R_{\perp}^c, R_{\perp//}$;
- General strength quantity: indexed by numbers like $R_{\perp\perp} \rightarrow R_{23}$.

It seems necessary to again cite for the two different composite domains two long-time used terms:
Material composite (Werkstoffverbund): structural-mechanically a composed ‘construction of different materials.

Note: A not smearable ‘conglomerate’ is usually the Fiber-grid-Reinforced-Concrete.

Composite material (Verbundwerkstoff): combination of constituent materials, different in composition.

For the strength properties it is to discriminate in the English language:

Yield stress (unfortunately termed yield strength, despite of the fact that it is not set as a strength property for Design Verification): material property corresponding to the point at which the material begins to deform plastically (in German Streckgrenze R_e), is end of proportionality σ_{prop}

Proof stress: point at which the material exhibits 0.2% of plastic deformation, known as stress at 0.2% strain- offset and set as yield strength property $R_{p0.2}$. (in German Fließgrenze or 0.2% -Dehngrenze).

Repetition: Shear Strength Quantities in Spatial Analysis of isotropic and anisotropic materials

These are an essential input with UD materials however also with isotropic mineral materials.

For 3D-analysis two specific shear strength quantities are applied, see Fig. 28-1:

* An approach-formalistic τ_{23}/R_{23} linked shear fracture stress $\tau_{23}^{fracture} = R_{23} \leq R_{\perp}^t$, used with Tsai-Wu and in the invariant approach of Hashin and further

* A Mohr-based approach linked so-called cohesive strength $R_{23}^r = \tau_n(\sigma_n = 0)$, used within an Hashin approach and in consequence, principally also with Puck. Since Puck formulated a full IFF-SFC $R_{23}^r \Rightarrow R_{23}^A$, defined by all 3 IFFs interacting approach, he could model-associated dedicate his action plane resistance a relation with other model parameters.

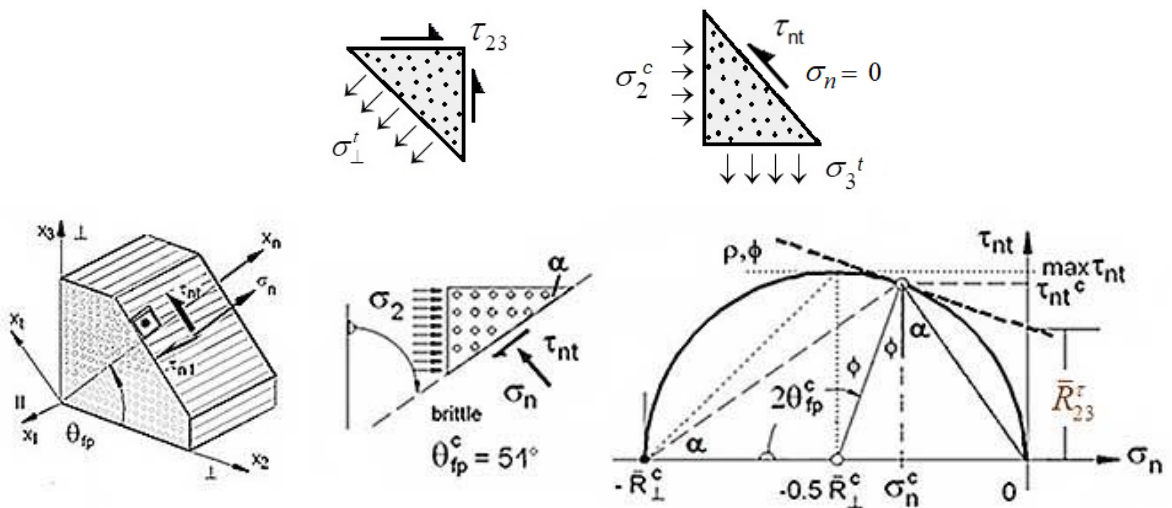


Fig. 28-1: (up) Difference of transversal shear fracture stress and cohesive strength. (below) Mohr-Coulomb curve characteristics

28.2 Indexing and Material Notations

Indexing is a chaos in the disciplines: It seems to be that the author could find (*some years ago for the planned novel ESA –Material Handbook*) a physically clear indexing system for the 3 material family models isotropic, transversely-isotropic UD and orthotropic materials (fabrics etc.). This indexing captures all material properties and allows a switching between.

The author’s Glossar on ‘Technical Terms’ (Springer) hopefully shall be a contribution to a better mutual understanding of 'constructive' engineers from the building industry and engineers from mechanical engineering and further, of engineers from the textile, manufacturing and material discipline as well in order to better manage the more and more interdisciplinary future project tasks.

Notes on designations: As a consequence to isotropic materials (European standardization) the letter R has to be used for strength. US notations for UD material with letters X (*direction 1, ||*) and Y (*direction 2, \perp*) confuse with the structural axes’ descriptions X and Y . $R_m :=$ ‘*resistance maximale*’ (French) = tensile fracture strength (superscript t is usually skipped because in mechanical engineering design runs in the tensile domain, which is opposite to civil engineering, where fiber reinforcement is coming up viewing carbon concrete). See further [*Cuntze Glossar*].

In the following Table, on basis of investigations of the VDI-2014 Working Group and on investigations for above Materials Handbook, Cuntze proposed internationally not confusing terms for strengths and physical properties. These self-explaining symbolic designations read for UD- materials:

| Property type | UD quantities | ‘generic’ number |
|---|---|------------------|
| fracture strength properties + friction properties | $\{R\} = (R_{ }^t, R_{ }^c, R_{\perp}^t, R_{\perp}^c, R_{\perp }^t)^T,$ $\mu_{\perp }, \mu_{\perp\perp}$ | 5 2 |
| elasticity properties | $\{E\} = (E_{ }, E_{\perp}, G_{ \perp}, \nu_{\perp }, \nu_{\perp\perp})$ | 5 |
| hygrothermal properties | $\{\alpha\} = \text{CTE } (\alpha_{ }^T, \alpha_{\perp}^T); \text{CME } (\alpha_{ }^M, \alpha_{\perp}^M)$ | 2 ; 2 |

Notes on composites and matrices:

- (1) The constituents retain their identities in the composite; that is, they do not dissolve or otherwise merge completely into each other although they act in concert. Composite materials provide improved characteristics not obtainable by any of the original constituents acting alone.
- (2) Normally the constituents can be physically identified, and there is an interface between them.
- (3) Composites include fibrous materials, fabrics, laminated (layers of materials), and combinations of any of them.
- (4) Composite materials can be metallic, non-metallic or a hybrid combination thereof. Carbon concrete is one example.
- (5) Approximately homogenizable to a smeared material are short fiber-reinforced FRC, SMC, UD-ply = UD-lamella. The lamella is smearable and therefore it can be modelled as a ‘composite material’.
- (6) Layered materials and foam materials are also forms of composite materials.
- (7) Cement-based mortar is a ‘smearable’ composite material (the construction organization RILEM has a problem here, because they do not discriminate ‘material composite’ from ‘composite material’)

Table 28-1: Notations of material properties

| | | | | | | | | | | |
|---|-------------------------------|------------------|---------------------|---------------------|---------------------|---------------------|---------------------|---------------------|------------------------|------------------------|
| 9 | general orthotropic | R_1^t | R_2^t | R_3^t | R_1^c | R_2^c | R_3^c | R_{12} | R_{23} | R_{13} |
| 5 | UD, \cong non-crimp fabrics | $R_{ }^t$ NF | R_{\perp}^t NF | R_{\perp}^t NF | $R_{ }^c$ SF | R_{\perp}^c SF | R_{\perp}^c SF | $R_{ \perp}$ SF | $R_{\perp\perp}$ NF | $R_{\perp\perp}$ SF |
| 6 | fabrics | R_W^t | R_F^t | R_3^t | R_W^c | R_F^c | R_3^c | R_{WF} | R_{F3} | R_{W3} |
| 9 | fabrics general | R_W^t | R_F^t | R_3^t | R_W^c | R_F^c | R_3^c | R_{WF} | R_{F3} | R_{W3} |
| 5 | mat | R_{1M}^t | R_{2M}^t | R_{3M}^t | R_M^c | R_{1M}^c | R_{2M}^c | R_M^c | R_M^c | R_M^c |
| 2 | ductile isotropic | R_m SF | R_m SF | R_m SF | deformation-limited | | | R_m^c | R_m^c | R_m^c |
| | brittle | R_m NF | R_m NF | R_m NF | R_m^c SF | R_m^c SF | R_m^c SF | R_m^c NF | R_m^c NF | R_m^c NF |

| | | | | | | | | | | | |
|---|-------------------------------|----------|-------------|-------------|---------------|------------------|---------------|-----------------|--------------------|-----------------|--|
| 9 | general orthotr. | E_1 | E_2 | E_3 | G_{12} | G_{23} | G_{13} | ν_{12} | ν_{23} | ν_{13} | comments |
| 5 | UD, \cong non-crimp fabrics | $E_{ }$ | E_{\perp} | E_{\perp} | $G_{ \perp}$ | $G_{\perp\perp}$ | $G_{ \perp}$ | $\nu_{ \perp}$ | $\nu_{\perp\perp}$ | $\nu_{ \perp}$ | $G_{\perp\perp} = E_{\perp} / (2 + 2\nu_{\perp\perp})$ $\nu_{\perp\perp} = \nu_{ \perp} E_{\perp} / E_{ }$ 3 is perpendicular to quasi-isotropic 2-3-plane |
| 6 | fabrics | E_W | E_F | E_3 | G_{WF} | G_{W3} | G_{W3} | ν_{WF} | ν_{W3} | ν_{W3} | Warp = Fill |
| 9 | fabrics general | E_W | E_F | E_3 | G_{WF} | G_{W3} | G_{F3} | ν_{WF} | ν_{F3} | ν_{W3} | Warp \neq Fill |
| 5 | mat | E_M | E_M | E_3 | G_M | G_{M3} | G_{M3} | ν_M | ν_{M3} | ν_{M3} | $G_M = E_M / (2 + 2\nu_M)$ 1 is perpendicular to quasi-isotropic mat plane |
| 2 | isotropic | E | E | E | G | G | G | ν | ν | ν | $G = E / (2 + 2\nu)$ |

| | | | | | | | |
|---|------------------------------|----------------|-------------------|-------------------|----------------|-------------------|-------------------|
| 9 | general orthotropic | α_{T1} | α_{T2} | α_{T3} | α_{M1} | α_{M2} | α_{M3} |
| 5 | UD \cong non-crimp fabrics | $\alpha_{T }$ | $\alpha_{T\perp}$ | $\alpha_{T\perp}$ | $\alpha_{M }$ | $\alpha_{M\perp}$ | $\alpha_{M\perp}$ |
| 6 | fabrics | α_{TW} | α_{TF} | α_{T3} | α_{MW} | α_{MF} | α_{M3} |
| 9 | fabrics general | α_W | α_F | α_F | α_{MW} | α_{MF} | α_{M3} |
| 5 | mat | α_{TM} | α_{TM} | α_{TM3} | α_{MM} | α_{MM} | α_{MM3} |
| 2 | isotropic for comparison | α_T | α_T | α_T | α_M | α_M | α_M |

Table of structural properties

Strength properties: NF:= Normal Fracture, SF:= Shear Fracture, R:= strength, σ, τ := indicate the fracture responsible normal or shear stress acting on the fracture 'plane'.

Hygro-thermal properties: T:= Thermal, M:= Moisture and Mat. λ, c : not listed.

Elasticity properties: E :=Young's modulus, ν :=Poisson's ratio, G :=shear modulus. \parallel := parallel to the fiber, \perp := transversal to the fiber direction; W := Weft, F := Fill, M := Mat . $\nu_{\parallel\perp}$:= (here!) larger Poisson's ratio. 1 := lamina fiber direction, 2 := lamina transverse fiber direction across the width or the plane, 3 := through-thickness direction; x, y := principal in-plane laminate directions, z := thickness direction (interlaminar)

28.3 Upcoming construction standards in Germany with comments

Standards in Germany are finalized, see [Fig.28-2](#):

- polymer matrix: BÜV 10 update
- mineral matrix: novel DAfStb-Richtlinie "Betonbauteile mit nicht-metallischer Bewehrung".

BÜV-Empfehlung

Überarbeitung 2019

Tragende Kunststoffbauteile im Bauwesen [TKB]

- Entwurf, Bemessung und Konstruktion -
Stand 08 / 2010

Zwanzig20 - Verbundvorhaben Carbon Concrete Composite C³
V1.2: Nachweis- und Prüfkonzepte für Normen und Zulassungen

C3 V1.2 PG3
DAfStb D 109

C3 V1.2 PG1/2
DAfStb D 161

Arbeitspapier Bewehrung

Carbonbewehrung; Sorten, Eigenschaften, Kennzeichnung, Prüfkonzepte

Schlussfassung V1.2 (16.07.2019)

Ansprechpartner:

DAfStb
FTA Albstadt
Glassolden GmbH Oschatz
ILK TU Dresden
IMB TU Dresden
ITM TU Dresden

Warnhinweis:

Dieses Dokument stellt keine DIN-Norm oder Richtlinie dar.

Es handelt sich um ein Arbeitsdokument, welches im Rahmen des Projekts C3 V1.2 als möglicher Vorschlag für eine Richtlinie erarbeitet wurde und im Rahmen des Projekts C3 L9 weiterentwickelt wird.

DEUTSCHER AUSSCHUSS FÜR STAHLBETON

D 36 DAfStb UA
Nichtmetallische Bewehrung

DAfStb-Richtlinie

Betonbauteile mit nichtmetallischer Bewehrung

Entwurf 19. August 2019
Cyan – Änderungen und Ergänzungen gegenüber D 29

Teil 1: Bemessung und Konstruktion
Teil 2: Bewehrungsprodukte
Teil 3: Hinweise zur Ausführung
Teil 4: Prüfverfahren

Notifiziert gemäß der Richtlinie (EU) 2015/1535 des Europäischen Parlaments und des Rates vom 9. September 2015 über ein Informationsverfahren auf dem Gebiet der technischen Vorschriften und der Vorschriften für die Dienste der Informationsgesellschaft (kodifizierter Text) (ABL L 241/1 vom 17.09.2015).

Bezüglich der in dieser Richtlinie genannten Normen, anderen Unterlagen und technischen Anforderungen, die sich auf Produkte oder Prüfverfahren beziehen, gilt, dass auch Produkte bzw. Prüfverfahren angewandt werden dürfen, die Normen oder sonstigen Bestimmungen und/oder technischen Vorschriften anderer Mitgliedstaaten der Europäischen Union oder der Türkei oder einem EFTA-Staat, der Vertragspartei des EWR-Abkommens ist, entsprechen, sofern das geforderte Schutzniveau in Bezug auf Sicherheit, Gesundheit und Gebrauchstauglichkeit gleichermaßen dauerhaft erreicht wird.

Herausgeber:
Deutscher Ausschuss für Stahlbeton e. V. – DAfStb
Budapester Straße 31
D-10787 Berlin
Telefon: 030 2693-1320
info@dafstb.de

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Other standard works on fiber-reinforced materials are :
wind energy GL re-work, Deutsche Bundesbahn

Guideline work ahead! In Germany

Verkauf durch den Beuth Verlag GmbH, Berlin, Vertriebs-Nummer 60300X

Fig.28-2: Guideline work ahead in Germany, BÜV10 update and D 36 novel

Comments of the author after a careful investigation of the two standard proposals, about 2020:

The DAfStb guideline "Concrete components with non-metallic reinforcement" is intended for fiber-reinforced components with concrete matrix.

*For engineers it is confusing not to clearly say which fiber material group the guideline is for. The fiber type CF, GF sets the limits of application.

*Originally for the *open* fiber grid the name lamella for a *dense* non-crimp fabric was used. The lamella however was still intensively used in construction repair of corroding steel-concrete ceilings (see [Fig.28-3](#))

*The suffix $_{nm}$ could be replaced by the indices of the polymer matrix world, namely for the pure fiber $_f$ and the cured fiber strand $_{||}$.

*Why sticking further to the old German-originated letter f (strength). Still at the GruSiBau-time (*about 1985, development of the excellent partial safety factor concept*) the author used the international letter R for the resistance entity strength in construction. Using the letter R – internationally and partly nationally still started - makes life of engineers over the technical fences simpler, internationally at least.

The BÜV-recommendation for load-carrying composite parts in construction is intended for fiber-reinforced components with polymer matrix.

*Above two upcoming standards are not harmonized regarding the designations amongst themselves and w.r.t. terms half a century internationally used in timber construction and also with polymer matrices. This is all the sadder for the author, because he edited the VDI 2044, sheet 3 guideline - initiated by civil engineers !- but not used in construction. The European Codes hopefully will improve this unfortunate situation.

LL: *Harmonization of denotations remains an urgent on-going task .*



Fig.28-4 presents a proposal for an ordering scheme.

Production of optimal structural components firstly requires an optimum design which includes the connections / joints and possible materials.

Then the locally best materials are to determine and to sort out - regarding production - to ensure the required optimum component properties considering sustainability.

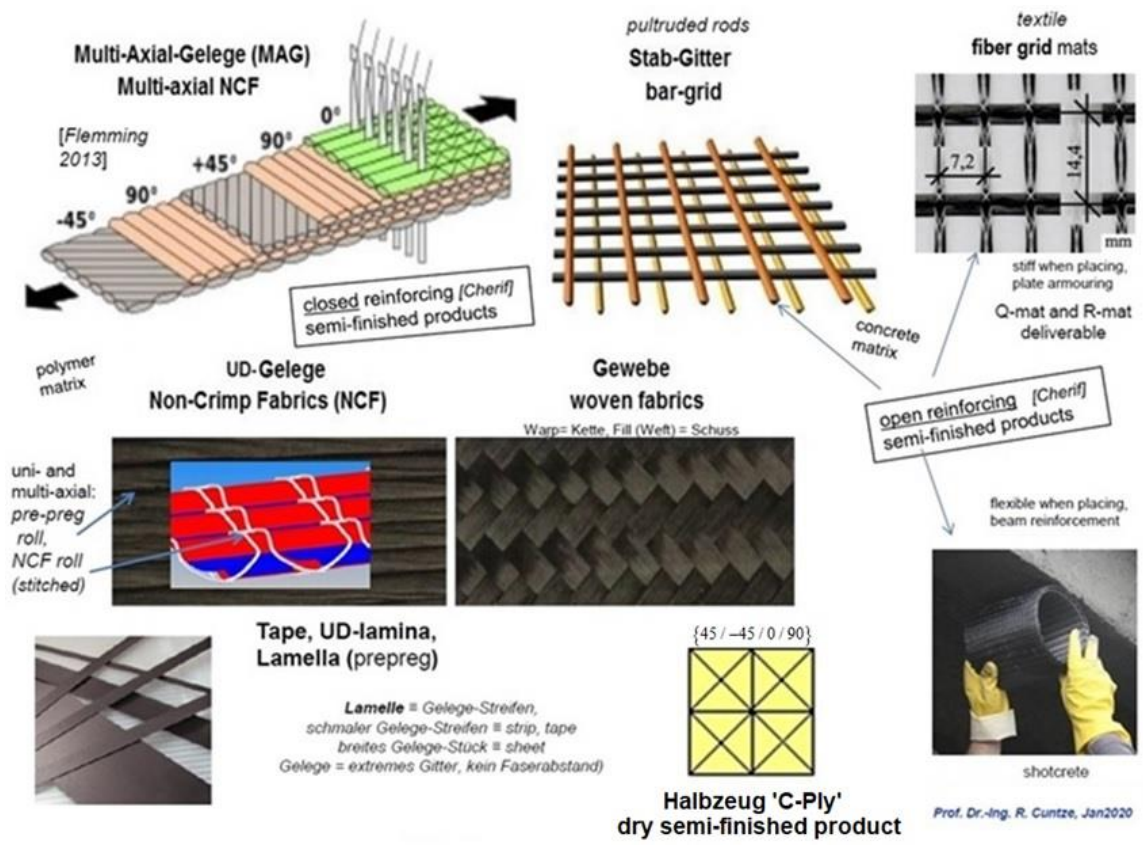


Fig.28-3: Basic fiber-reinforcing products in Engineering

| | | | | | | | | | |
|------------------------------|------------------------------|---|--|---------------------------------|---|--|---|-------------------------------|---------------------------|
| isotropic | Normal-Concrete | Concrete matrix | water + cement (CEM I, CEM III) + aggregate (sand, gravel) + possibly additives, such as super-plasticiser, retarder | | max. grain > 4 mm ¹⁾ | Fiber Reinforced Concrete GFRC CFRC PPFRC PBOFRC | F(R)C | | |
| | Fine ¹⁾ Concrete | | | | max. grain < 4 mm | | | | |
| anisotropic | Reinforcement Form | | grid-type reinforcing structures Fiber-Concrete-Composite FCC | | | | | Fiber Composite Materials FCM | |
| | CRC or CC GFC or GC | FRC | UDRC | Textile-Reinforced Concrete TRC | | | | | |
| | | | rope bar | rebar grid | R-, Q-grid | embroidered sandwich | non-woven (randomly oriented, oriented) | | short fiber long fiber |
| | | | 1D | 2D Reinforcement Alignment | | 2D | 2D - 3D | | 2D - 3D |
| CFRP GFRP AFP BsFRP | FRP | semi-finished products for reinforcements (endless fiber, long fiber) | | | | | for matrix improvement | | |
| UD ply lamella strips | | NCF lamella sheet | fabric | | non-woven (randomly oriented, oriented) | | | | |
| isotropic | Fiber-Polymer-Composite FPC | | closed reinforcing structures | | | | | SMC, BMC | FP |
| | Thermosets Thermoplastics | Polymer matrix | Resin Systems: thermosets, thermoplastics, with catalysts etc. | | | | | | |

Fig.28-4: Ordering scheme proposal for Fiber-Composite Materials FCM, construction-linked such as Fiber-Reinforced Polymer FRP, Fiber-Reinforced Concrete FRC, CFRC:= CarbonFiber-Reinforced Concrete, Bi-Directionally Reinforced Concrete BDRC, UHP-(short)Fiber-Reinforced Concrete, HPFRC. Green coloured are still fixed notions. Matrix types of the Reinforcements FRPm = Fiber-Reinforced Polymer matrix, Fiber-Reinforced-Mineral matrix FRMm. International subscripts f = filament, m = matrix; superscripts t = tension, c = compression

29 Miscellaneous

In this chapter some results of the author's works are collected, which have been discussed in his various working groups.

29.1 Construction-linked Additive Fabrication AF

Classification of fabrication processes: Subtractive processes (waste), Formative processes and Additive processes (automatically digitized fabrication now) [VDI 2403]. *The term manufacture is not accurate: Manus and facere → means made by hand.*

1. In subtractive processes, the geometry to be created is created by defining the removal of individual volume regions. Typical representatives of this group of manufacturing processes are machining processes such as turning, drilling or milling. (timber construction etc.)
2. Formative processes refer to the production of geometries by forming in compliance with volume constancy. Formative manufacturing processes are deep drawing, forging or primary forming.
3. Additive fabrication processes create a geometry by joining together volume elements (so-called "voxels"), such as the standard processes concreting, bracketing, plastering a wall etc.

The engineer's desire is to obtain accurate process names in the additive fabrication point 3, the term 3D-print does not give a clear process information. Therefore some basic definitions

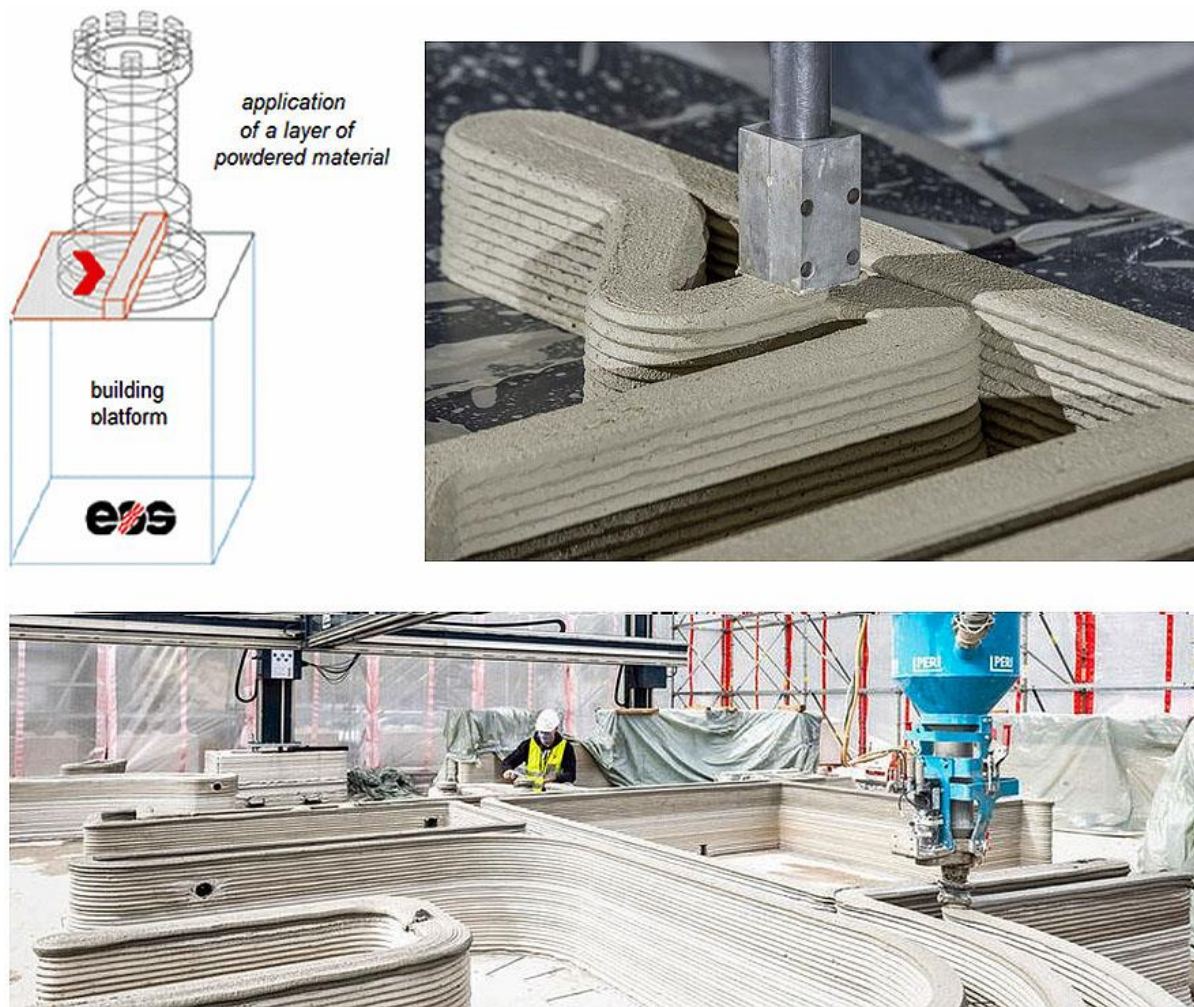


Fig.29-1: Particle-bed technique (up left), Mortar strand depositing (up, right); (below) Peri GmbH 2021, building a two-story house in Beckum

are provided for construction. The two basic digitized additive fabrication processes in construction are to term:

(3a) **Powder bed process:** *true original 3D horizontal slice printing in construction*

Total cross-section including the 'openings' is produced in a powder bed layering process. Layer thickness is usually $\ll 1$ mm. (*for formwork production, usually*). Technique Selective Laser Sintering, does pretty well correspond with the printing definition 'Procedure, to apply something by pressure like printing a book'.

(3b) **Extruded mortar-strand deposit process:** *is no 3D printing in the original sense*

Total cross-section including 'openings' is produced in a 'path tracking operation'. Layer thickness is several mm, depending on the strand thickness. Technique Extruded mortar strand deposit.

LL:

- * Any material that can be glued, welded or melted can be used in AF. For industrial purposes, metals, plastics, sand and ceramics are common materials, but the process is to adapt
- * The extrusion process is for walls and other compressed load-bearing building structures!. That's why there is no fully '3D-printed house' existing.
- * Cost-effective conventional ceiling slabs are still required
- * Any material that can be glued, welded or melted can be used in AF. For industrial purposes, metals, plastics, sand and ceramics are common materials, but the process is to adapt.

29.2 Buckling analysis versus Strength analysis

This chapter provides introductory information about buckling of columns (beams), plates, panels and shells. It shortly addresses just essential features in stability analysis (*speaking stability is more positive than buckling*).

This chapter is just dealing with static stability problems. It covers a very basic background in order to guide the practicing designer to better understand the manuals of commercial analysis software.

The following contents basically stems from the creation of the ESA Buckling Handbook, ECSS-E-HB-32-24A. Cuntze was first convenor and founder of the team as well as a co-author of the later prepared HSB 40100-04] from R. Cuntze and J. Broede. Noteworthy: In the HSB, section 40000, for a wide spectrum of columns, rods, rings and deep beams design sheets are found. The same is given in the chapter plates where the available design sheets on anisotropic plates provided by J. Broede and colleagues are outstanding sheets.

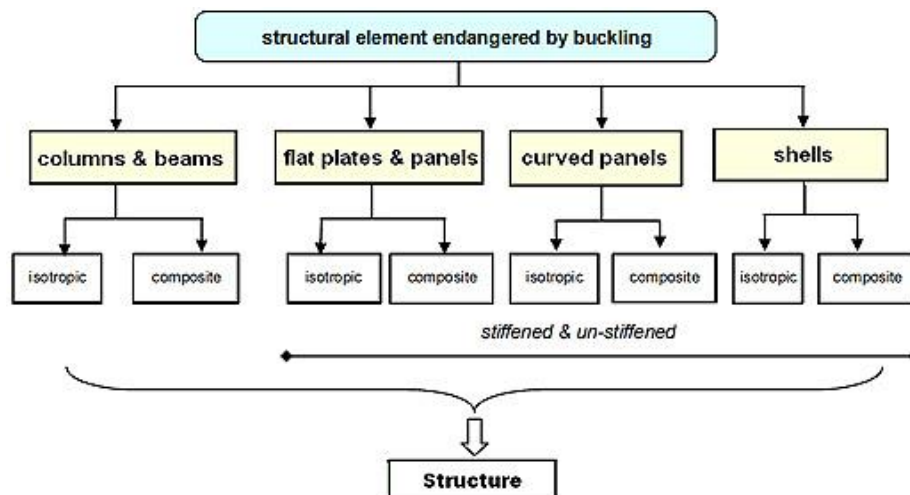


Fig.29-2: Breakdown of buckling of endangered structural elements [Cuntze, ECSS]

Different levels of analysis complexity are treated in the literature above. Going in steps from the lower level of complexity to the higher level of complexity (which will be denoted as a “hierarchical approach”) the structural analyst is able to carry out and finally to successfully interpret analyses at the highest level of complexity, typically finite element computations (see [CUN22]).

In structural design the following subjects must be demonstrated: *Material Strength*, applying SFCs, strength criteria, and *Structural Strength*, applying buckling resistance conditions. Fig.23-3 compiles these subjects.

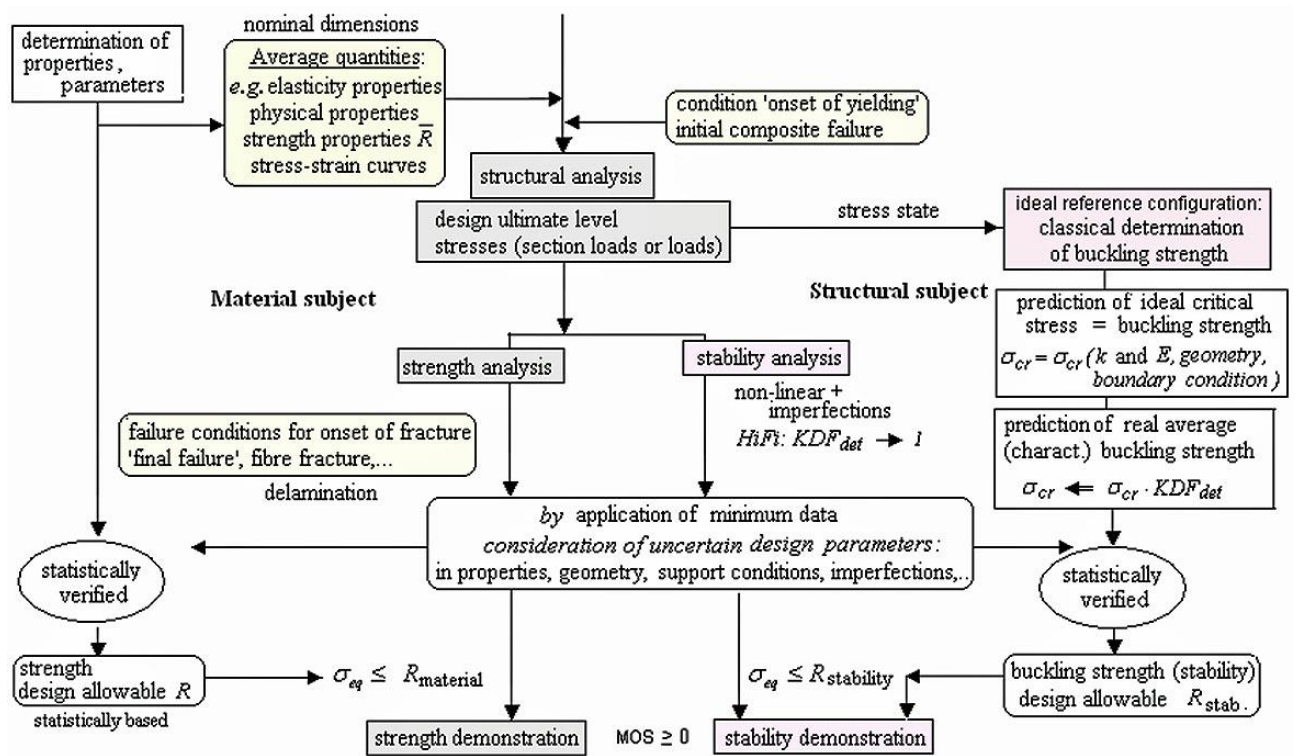


Fig. 29-3: Visualization of the (actually) required deterministic input demands. *KDF* usually corrects difference of calculation model to experiment (50% expectance value), k : =buckling factor (from handbook tables), *MoS*: =Margin of Safety

Using such an engineering procedure the engineer is able to analyze the stability of (large) structures composed of structural elements, also referred to in literature as structural components or structural items. The term structural element includes typical elements such as columns and beams, plates, panels, and shells. In practice these structural elements often contain structural details, e.g. shells containing openings or reinforcements. The associated “basic” structural elements, the elements without structural details are denoted in the ECSS as “Typical Structural Elements”. Non-axial symmetric shells and truss systems are not addressed in the book.

29.3 Annex on other Works of the author

If one might be interested one can find information on the following subjects in the author's compilation [*CUN22*] especially on the various projects at MAN the author was involved.

- Influence of low Cross-sectional Shear Rigidity and Rotatory Inertia on the Critical Speeds of Shafts with Uniformly Distributed Mass (1984 for centrifuges)
- The Influence of Cross-sectional Shear Flexibility and Rotatory Inertia on the Natural Frequencies of Beams with Uniformly Distributed Mass (1983)
- Natural Frequencies of a Cracked Beam for Production Quality-testing of Rotor blades
- Design of the Metallic Energy storage Flywheel for the floating crane ship Swartow (1982)
- Design of Fiber-reinforced Gas-Ultra-Centrifuges, GUZ (1971-1986)
- A New concept of a Composite Flywheel due to novel fiber-reinforced materials (1988)
- Increasing the Limit of Usability of CFRP Tubes by Built-in-Stresses (1993)
- Structural Reliability, Factors of Safety and Design Values, §12
- Some Winding Theory of Filament Wound Pressure Vessels, §20.
-

The author's annual books piled up over the decades.



The author's
Annual booklets
with their
technical project-records
from
1970 up to 2023

29.4 First Discussion of the applicability of the FMC (VDI 2014 working group, 1994-1995)

Gespräch in Klausur am 23.1.95
 Herr Prof. Pache 20.1.95
 Dr. Hügbrockts 25.1.95

ZIEL: Beschreibbare Materialcharakterisierung für mehrachsige Beanspruchung

INFO: v. Mises - Versagens typ. "Fließbeginn"
 • 1 Festigkeitswert bestimmt die Versagensoberfläche.
 Fließbeginn

$$F_{\text{Fl}}^2 = c_1 \sqrt{J_2} / R_{p0.2} = 1 \quad J_2 \sim \epsilon_{\text{pl}}^2$$

 aus dem Druckfestigkeitspunkt σ_{c1} -Bestimmung
 $(\sigma_{c1})^2 = (\sigma_c^d = -R_{p0.2}^d, 0, 0)^T$
 • Taut [Bismar / Malvernalt] Gleitlinien = Versagens ebenen bestimmbar

IDEA:
 1. Mit Invarianten für isotrope und transversal-anisotrope Werkstoffe vorhandene Zustände abbilden für die Versagensformen "Schubbruch" (verwandelt dem Fließbeginn-Versagen) = elastoplastisches Verhalten) und "Zugbruch". Das bedeutet die Abbildung mittels eines Ansatzes mit 1 Freiwert (analog zu v. Mises) und 2 weiteren v. Mises im Fließversagenfall, Bauschinger Effekt), so daß

$$F_{\text{Fl}} = c_1 \sqrt{I_1} / R_m^c + b \sqrt{J_2} / R_m^{d2} = 1 \dots \text{Schubbruch}$$

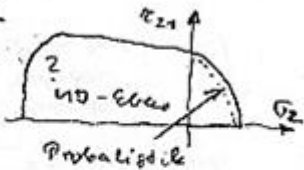
$$F_{\text{Z}} = (c_1 I_1 + c_2 \sqrt{J_2}) / R_0^2 = 1 \dots \text{Zugbruch}$$

 mit
 $R_0 = R_m^2$ bei spröden Verhalten
 $R_0 \neq R_m^2$ bei zähen Verhalten
 (letztens komplizierter zu bestimmen)

kein globales Versagenskriterium, sondern Ermittlung einer stückweise glatten Versagensoberfläche = benötigte Abbildung eines zinschen, Versagens typ.

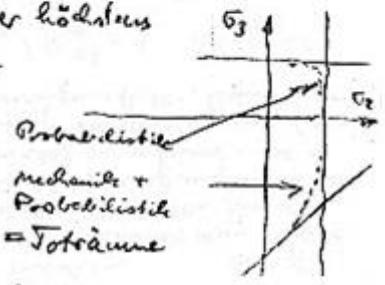
WIRKEBENE KRITERIUM

Das Kriterium stiftet die Mechanik
 begl.: F_{\perp} mit $F_{\perp\perp}$
 F_{\parallel} mit $F_{\parallel\parallel}$
 F_{\perp} mit $F_{\perp\perp}$?



Das Kriterium stiftet nicht oder höchstens Integral als Probabilistik

Frage ist: Inwiefern ausreichend ?



INVARIANTEN FORMULIERUNG DER ZWISCHENFASER BRUCHKRITERIEN

$$F_{\perp} = [a I_2^{\perp} + b \sqrt{I_3^{\perp}}] / R_{\perp}^{\perp} = 1$$

$$F_{\perp\perp} = [a I_2^{\perp} + b I_4^{\perp}] / R_{\perp\perp}^2 = 1$$

$$F_{\parallel\parallel} = [a I_2^{\parallel} I_3^{\parallel} + b I_5^{\parallel}] / R_{\parallel\parallel}^3 + c I_3^{\parallel} / R_{\parallel\parallel}^2 = 1$$

$$I_1 = \sigma_1$$

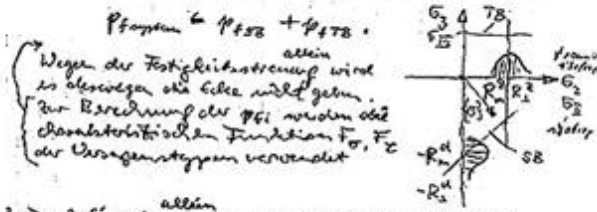
$$I_2 = \sigma_2 + \sigma_3$$

$$I_3 = \tau_{13}^2 + \tau_{12}^2 = \tau_{31}^2 + \tau_{21}^2$$

$$I_4 = (\sigma_2 - \sigma_3)^2 + 4 \tau_{23}^2$$

$$I_5 = (\sigma_2 - \sigma_3) (\tau_{13}^2 - \tau_{12}^2) + 4 \tau_{23} \tau_{13} \tau_{12}$$

2. Interaktion wird anschließend vorgenommen
 Diese berücksichtigt das mechanische Zusammenwirken der Versagensarten
 (= sog. mechan. oder physikalisches Modell) und somit probabilistisches Zusammenwirken (= sog. prob. Modell).
 darf im Interaktionsgebiet = Mischversagensgebiet die Versagenwahrscheinlichkeit nicht nach dem Serienmodell für Systemversagen betrachten. Es ist



3. Durch Einsetzen der versagensaktiven Wirtelbeanspannungen (die Invarianten sind damit kleiner) in die von Wirtelbein-KOS formulierten Quasianten eine Aussage für den Versagenswinkel zu finden.
 Die Oberflächen vom 1FT-Ansatz und Wirtelbein sind in seiner Bruchtyp-Bereich ähnlich

oder $F = F(\sigma_1, \sigma_2, \sigma_3, \tau_{12}, \tau_{13})$

Hv. Hügbrockts
 Grup R. Cuntze

FAX 0241-8888316

ZUR DURCHFÜHRUNG

1. Freiwertbestimmung

$F_{II} : \{G\} = (0, 0, 0, 0, 0, R_{II})^T$
 und in diesem Punkt
 $\tan \alpha_{\perp II} = d\sigma_{\perp II} / d\sigma_2$ mit
 $\alpha_{\perp II}$ = negativer Neigungswinkel

$\{G\} = (0, R_{II}^2, 0, 0, 0, R_{II})^T$

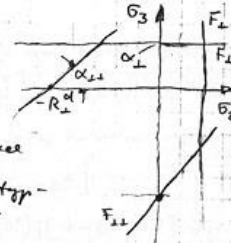
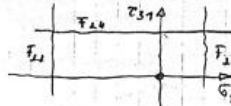
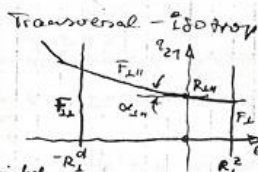
$F_{\perp} : \{G\} = (0, R_{\perp}^2, 0, 0, 0, 0)^T$
 $\tan \alpha_{\perp} = d\sigma_{\perp} / d\sigma_2 = 0$

$F_{II} : \{G\} = (0, -R_{II}^2, 0, 0, 0, 0)^T$
 $\tan \alpha_{\perp II} = d\sigma_{\perp} / d\sigma_2$ mit
 $\alpha_{\perp II}$ = positiver Winkel
 $= 45^\circ + \text{Reibungswinkel}$

Hand → Abbildung der reinen Versagensstypen scheint gelungen zu sein

2. Interaktion im Mischgebiet

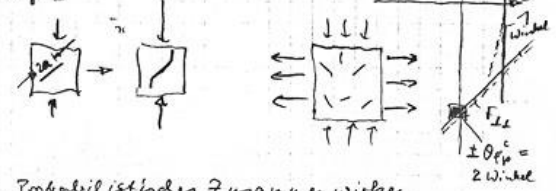
Aufgrund der Ähnlichkeit des Verlaufes infolge mechanischer und probabilistischer Zusammenwirken besteht Hoffnung, daß die Testergebnisse als Auswirkung beider Aspekte durch eine probabilistisch herleitbare Interaktionsfunktion dargestellt werden können. Eine solche ingenieurmäßige Betrachtung ist schon deswegen geboten, weil σ_c Größeneinfluß, Verschieben bzgl. der Festigkeitsnormung und bisherige Nichtberücksichtigung mehrerer Versagensstufen (im Ausfallpunkt σ_c , im Zug-Zug-Punkt alle Richtungen) nicht erfaßt sind (z.B. Gespräch mit RCP am 19. 1. 95)



Die Deutung der Testdaten erfordert hier aber eine genauere Betrachtung.
 → nichtig für ZFB-Kriterium

Hand → im Sinne einer ingenieurmäßigen, die Interaktion abbildenden Funktion werden bezüglich einiger Testdaten in der Diplomarbeit Pizer unternommen. Nach meinen nachfolgenden Überlegungen die Probabilistike heranzuziehen wurde hier nicht weiter nachgedacht

2.1 Mechanisches Zusammenwirken
 Hierin wurden vom Verfasser (Studienarbeit Pohl) und Radvitz/Plica bruchmechanische Überlegungen angestellt, wie sich Defekte und Druckspannung stabil vergrößern, aber damit den σ_2 -Querschnitt verringern



2.2. Probabilistisches Zusammenwirken
 • Quasisotrope Ebene 4. oder 2. Quadrant
 z Winkel sind möglich

• Quasisotrope Ebene 1. Quadrant
 Die Hauptspannung in $\{G\} = (0, R_{II}^2, R_{II}^2, 0, 0, 0)^T$ ist in allen Richtungen gleich hoch wirkend und kann sich dem entsprechend liegenden Defekt ausbreiten oder die kritische Spannungsintensität

$K_{IC} = \sigma_c \sqrt{a}$ Geometriefaktor

führt in allen Richtungen zum Versagen.
 Nach Wöhler kann dieses berücksichtigt werden gemäß
 $F_{II}^{22} = F_{II} [1 + (\sigma_3/\sigma_2)^m]$ für $\sigma_3^2 > \sigma_2^2$

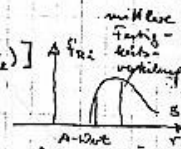
σ_c ist aus R^{22} ermittelbar [A-werte].

Für gemeinsamen Versagen [siehe Radvitz; Drost-Christensen (Kapitel 7)] sei hier noch angegeben.
 Die Wahrscheinlichkeit des Versagens eines Systems aus 2 Elementen = 2 Versagensstypen ist berechenbar für eine vorgegebene Lastspannung

$$F_R(s) = P(R \leq s) = 1 - P(R_1 > s_1)$$

$$= 1 - [1 - F_{R_1}(s_1)] [1 - F_{R_2}(s_2)]$$

$$F_{RC}(s) = 1 - [1 - F_G] [1 - F_C]$$



Annahme: die Versagensstufen sind voneinander unabhängig
 F_{RC} = Verteilungsfunktion für die "kombinierte Bruchspannung" aufgestellt für den mittlere Erwartungswert, interessant für den Konstrukteur als A- oder B-Wertkurve.

Übergang IFT-A → Winkelmess-Ansatz

- Da die Bruchoberfläche für den Übergang herangezogen wird, wurde in erster Annäherung angenommen (z. Bsp. 10°, keine Reibung), daß einfach die linke Seite maximiert werden muß, d.h. die Maxima von $d(\text{red } F) / d\theta = 0$ gesucht werden müssen.
 Näherung funktioniert nur, falls $F = 1 \propto \text{red } F$ (also Kontrolle)
- Sofolge des Verlassens der Invarianten Bruchbedingung $F = 1 \rightarrow \text{red } F \neq 1$ ist der Streckungsfaktor zu minimieren

29.3 Some Final Notes from Personal Experience

- ✓ Mechanics remains one very essential basis when developing light-weight structural components and Artificial Intelligence (AI) with its algorithms is a helpful supporting tool
- ✓ Only System Engineering with experienced engineers using mechanics and the necessary other disciplines - together with AI - enables to produce qualified products
- ✓ At the end someone has to sign that the developed structure will work and by that will take over responsibility. This experienced person is the absolutely necessary ‘plausibility checker’ for the obtained analysis and test results including generic AI-supported results.
- ✓ Bridge disciplines and materials by showing up similarities to simplify engineering life!
- ✓ In the present multi-physics applications product development is the work of several experienced engineers. Otherwise one does not deliver qualified ‘Multiple function structural products’.
- ✓ In the case of bending of FRPlastic- and FRConcrete-parts carbon fibers can be only exploited using pre-tensioning and thereby compressing the tension-sensitive matrices Plastic or Concrete
- ✓ Viewing SFCs, one must be careful with conclusions reported in literature (unpleasant personal experience): SFC model *modifications* - created by another author - are used under the name of the originator and then poorly rated, however, the modification was not reported!
- ✓ Experienced engineers know: “Check your test together with test data evaluation. Check your analysis including assumptions.”
- ✓ There is a rationale to take a distinct $\sigma - \varepsilon$ curve: From risk analyses and decision theory the best prediction will be achieved by applying the mean(σ, ε)-curve = 50% probability !
- ✓ Certification by Analysis, only: Here, simulation can optimize the output of the usually only permitted minimum number of physical tests, and enabling to better manage risk and improve prediction.

30 Glossary Book, Contributions to Handbooks, Guidelines etc.

30.1 “Technical terms for composite components in civil engineering and mechanical engineering”

The construction industry is an industry in which the topic of high-performance fibre composites is not yet established on the one hand, but where there is enormous application potential on the other. Against this backdrop, Carbon Composites e.V. (CCeV) in Augsburg has founded a specialist department "CC Bau (construction)".

For CC Bau, this repositioning meant that Fibre-Reinforced Plastic (FRP) - and the various fibre-reinforced concrete matrices had to be covered. i.e. 'fibre-reinforced concrete'. The latter involves endless '(roving)-Reinforced Concrete' (RC) as well as '(short) Fibre Reinforced Concrete' (FRC).

The following matrix applications must therefore be captured: (1) *Polymer matrix-related*, such as Glass fibre plastic pipes and containers, wind rotor blades and pedestrian bridges in GlassFRP and CarbonFRP as well, and (2) *Concrete matrix-related*, such as textile 'fibre grid' (mat) - reinforced concrete bridges and machine foundations, overhead line masts, industrial floors, multi-storey car parks, silos, prefabricated garages, transformer houses, offshore applications, tubbings, sandwich façade panels, un-tensioned and tensioned bending panels, FRP shells and bridges. Further, a big topic is the rehabilitation with FRP-'lamellas' (tapes, strips) such as the reinforcement of a ceiling plate (slab), because of increased moments, using CFRP-lamellas applied by surface bonding,

Carbon Fibers in the construction industry reduce the concrete amount, which is positive for the CO₂ footprint due to the reduced clinker production required and are sustainable due to their non-corroding behaviour in contrast to steel.”

Purpose of this Glossary:

Borders between engineering disciplines are disappearing, more and more. It can also be noticed that in the different fields of the fiber-using industry there are different "speeches" and that technical terms are sometimes used very differently. Several groups of engineers would therefore have to be connected conceptually so that they understand each other correctly when making decisions. These are 'constructive' engineers from building industry and mechanical engineering and further, engineers from the textile and material range as well as from manufacturing.

This glossary focuses especially on carbon fibers CF and concrete matrices. At the beginning it presents a first scheme of order for the different, interconnected disciplines. At the end, a picture gallery illustrates technological details and applications. This gallery just includes carbon applications from construction industry because these are less known.



Mathematisches Buch Körberstiftung

30.2 “Handbook for the German Ministry of Defense on a Safety Concept for Fiber Reinforced Plastic Structures". Koblenz, 1992 (in German)

Cuntze R., Rackwitz R., Gollwitzer S., Plica S. and Stoeffler G.

30.3 “ VDI 2014 Sheet 1-3” (1980 - 2006)

*Editor of the VDI 2014, sheet 3, (2006) and contributor to sheets 1 and 2

*Presenter of a draft on “Reliability & Safety”



Working Group VDI 2014, Meeting members: Moser, Cuntze, Lutz



VDI 2014-dedicated Meeting at Dresden University (September 1990)

b.24. EG ✓ → EG5

Protokoll der 1. Sitzung des FKV-Diskussionskreises am 6.12.83
bei der Fa. MAN-Neue Technologie, München

1. Anwesende Diskussionskreis-Mitglieder

Herren Prof. Ahrendts
Dr. Bansemir
Bessler
Buchs
Dr. Cuntze
Prof. Einsfeld
Dr. Geier
Prof. Giencke
Lutz
Prof. Moser
Prof. Puck
Dr. Rother
Dr. D. Schütz
Dr. Schulte
Dr. Stellbrink
Dr. Wurtinger

Die Anwesenden danken Herrn Krawinkel für die Einladung und das Nikolausgeschenk des VDI.

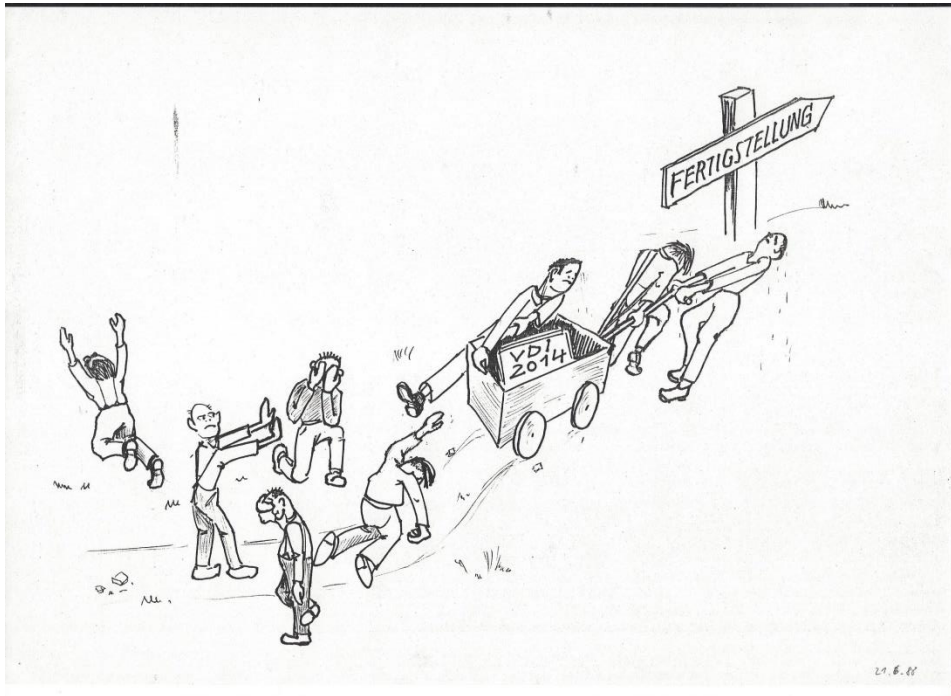
2. Kurzvorträge

Die Vorträge wurden bis auf den Beitrag Lutz/Moser "Sicherheitskonzept" gehalten. Der aus Zeitgründen entfallende Beitrag liegt als Anlage dem Protokoll bei.

3. Vorstellungen zum Thema Diskussionskreis

Die Nützlichkeit eines derartigen Diskussionspodiums wurde allgemein betont.

Als zeitlichen Abstand zwischen Zusammenkünften könnte ein Jahr vorgesehen werden.



Data –driven engineering

Sondersitzung "Schwingfestigkeit", VDI Richtlinie 2014, Teil 3

Ort: Institut für konstruktiven Leichtbau und Bauweisen, TU Darmstadt
Datum: 21.6.00, 9:15 bis 16:30
Thema: **Wie wird der rechnerische Schwingfestigkeitsnachweis für Faser-Kunststoff-Verbund-Bauteile in der Praxis geführt?**

Teilnehmer:

| | |
|-----------------|--|
| Prof. Cuntze | MAN-Technologie |
| Prof. Schürmann | Institut für konstruktiven Leichtbau und Bauweisen, TU Darmstadt |
| Herr Gerharz | LBF Darmstadt |
| Dr. Huth | Fairchild Dornier, Oberpfaffenhofen |
| Herr Osthorst | Aerodyn, ... |
| Herr Garbe | ehemals TUD und LBF |
| Herr Denzin | Institut für konstruktiven Leichtbau und Bauweisen, TUD |
| Herr Scholz | Rolladen-Schneider Flugzeugbau, |
| Dr. Huybrechts | ABB Forschung, Heidelberg |
| Prof. Puck | ehemals GH Kassel |
| Dr. Gaedke | DLR, Braunschweig |
| Herr Kleinhansl | Aero Dynamik Consult, ... |
| Dr. Wacker | Germanischer Lloyd, HV Hamburg |
| Herr Kenschke | DLR, Stuttgart |
| Herr Lutz | IMS, Innsbruck |

entschuldigt:

| | |
|----------------|----------------|
| Dr. Hachenberg | DASA, Hamburg |
| H. Emmerling | ECD, Ottobrunn |

Anlass der Sondersitzung:

Beim Endbearbeiten des Kapitels 'Schwingfestigkeit' kam der Ausschuß überein, nachzuprüfen, ob nicht doch ein paar Vordimensionierungsformeln dem Leser bereitgestellt werden können. Dazu sollte ein Expertenmeeting stattfinden, zu dem vornehmlich Industrielleute eingeladen werden, die solche Nachweise zu führen haben.

Bemerkung:

Die stattfindenden 'Fatigue-Conferences' (s. Anlage) geben höchstens Hilfe, das Thema Ermüdung (Schwingfestigkeit) zu verstehen, bieten aber keine Unterlagen für das Aufstellen von 'Auslegungsformeln'.

1. Ausführungen der einzelnen Teilnehmer

Wacker

Der GL gibt in seiner Windflügelrichtlinie Hinweise zur rechnerischen Durchführung des Schwingfestigkeitsnachweises. Wacker verteilt das diesbezügliche Kapitel der GL-Richtlinie.

Im wesentlichen basiert der rechnerische Schwingfestigkeitsnachweis auf folgenden Grundlagen:

- Die Zeitfestigkeit des FKV wird durch eine synthetische Wöhlerlinie (sog. Typ 2, d.i. Gerade im doppelt-logarithmischen Maßstab ausgehend von der Kurzzeitfestigkeit bzw. deren charakteristischem Wert) beschrieben für $R = -1$
- Annahme: Gültigkeit des Goodman-Diagramms vorausgesetzt zur Umrechnung auf andere Spannungsverhältnisse R
- Annahme: Gültigkeit der Miner Regel

Die Neigungen k der Wöhlerlinie $N^{1/k}$ sind in der GL-Richtlinie angegeben mit:

- $k = 9$ für GFK mit UP-Harzen
- $k = 10$ für GFK mit EP-Harzen
- $k = 14$ für CFK.

VEREIN
DEUTSCHER
INGENIEURE

Entwicklung von Bauteilen
aus Faser-Kunststoff-Verbund
Berechnungen
Development of FRP components
(fibre reinforced plastics)
Analysis

VDI 2014

Blatt 3 / Part 3

Ausg. deutsch/englisch
Issue German/English

Frühere Ausgabe: 04/97 Entwurf, deutsch
Former edition: 04/97 Draft, in German only

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Die deutsche Version dieser Richtlinie ist verbindlich.

No guarantee can be given with respect to the English translation. The German version of this guideline shall be taken as authoritative.

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VDI-Gesellschaft Kunststofftechnik

VDI-Handbuch Kunststofftechnik
VDI-Handbuch Konstruktion


Vervielfältigung – auch für innerbetriebliche Zwecke – nicht gestattet / Reproduction – even for internal use – not permitted

| | | |
|--|---|---|
| <p>VEREIN DEUTSCHER INGENIEURE</p> | <p>Entwicklung von Bauteilen aus Faser-Kunststoff-Verbund Zuverlässigkeit und Sicherheit</p> | <p>VDI 2014 Blatt 4 (4. Entwurf) Zusammengestellt R. Cuntze, MAN</p> |
| <p>Development of FRP components (fibre reinforced plastics) Reliability and Safety</p> | | <p><i>Einsprüche bis ??,??,1995 an Verein Deutscher Ingenieure VDI-Gesellschaft Kunststofftechnik Postfach 10 11 39 40002 Düsseldorf</i></p> |
| <p>Inhalt Seite</p> | | |
| <p>Vorbemerkung</p> <p>1. Anwendungsbereich, Nutzen</p> <p>2. Abkürzungen, Begriffe, Symbole</p> <p>3. Nachweis</p> <p>3.1 Allgemeines zu Rechnerischem und Experimentellem Nachweis</p> <p>3.2 Mechanische Modellierung (Grenzzustände)</p> <p>3.3 Experimenteller Nachweis</p> <p>4. Traditionelles Sicherheitskonzept (TSK)</p> <p>4.1 Allgemeines, charakteristische Werte</p> <p>4.2 Unsichere Eingangsgrößen beim Rechnerischen Nachweis</p> <p>4.3 Sicherheitsfaktoren (Auslegung) und Sicherheitsmargen (rechn. und experim. Nachweis)</p> <p>4.4 Kritik am TSK</p> <p>5. Probabilistisches Sicherheitskonzept (PSK)</p> <p>5.1 Allgemeines</p> <p>5.2 Einwirkungsgrößen, Widerstandsgrößen und stochastische Konstruktionsparameter</p> <p>5.3 Teilsicherheitsfaktoren (semiprobabilistische Vorgehensweise)</p> <p>5.4 Stochastische Modellierung und Logische Modellierung des Versagenssystems</p> <p>5.5 Berechnung von Versagenswahrscheinlichkeit oder Zuverlässigkeit</p> | | <p>6. Bestimmung von Kennwerten</p> <p>6.1 Definition und Verwendung von Dimensionierungskennwerten</p> <p>6.2 Ermittlung der statistischen Parameter aus Meßdaten</p> <p>6.3 Ermittlung Charakteristischer Werte</p> <p>7. Produktsicherung</p> <p>7.1 RAMS-orientierte Entwicklung</p> <p>7.2 Qualitätssicherungskonzept und Integritätskontrolle mit Prüfmethoden</p> <p>7.3 Versuche zur Erzielung der Strukturzuverlässigkeit des FKV-Bauteils</p> <p>Literatur</p> <p>Anhang 1 Schätzverfahren, Statistische Tests, Updating Verfahren, Bayes</p> |
| <p>Vorbemerkung</p> <p>Die Richtlinie enthält Empfehlungen für das Entwickeln von Bauteilen aus Faser-Kunststoff-Verbunden (FKV), die aus einzelnen faserverstärkten Schichten bestehen, bei denen Fasern in einer Kunststoff-Matrix eingebettet sind. Die Bauteilentwicklung wird dargestellt, wobei die Berechnungen eingehender behandelt werden. Die Richtlinie will durch Systematisieren und Vereinheitlichen der Bauteildimensionierung auch die Zulassungsverfahren und die</p> | | <p>Qualitätssicherung erleichtern. Sie ist in vier Teile gegliedert:</p> <ul style="list-style-type: none"> • Blatt 1 Grundlagen • Blatt 2 Konzeption und Gestaltung • Blatt 3 Berechnung • Blatt 4 Zuverlässigkeit und Sicherheit. <p>Beispielsammlung: s. VDI Report??</p> |
| <p style="text-align: center;">VDI-Gesellschaft Kunststofftechnik</p> <p style="text-align: center;">VDI-Handbuch Kunststofftechnik</p> | | |





30.4 “German Aeroautical Handbook” (‘Airbus design folder’)

| | | |
|--|--------------------------|--------------------------------|
| HSB HANDBUCH STRUKTUR BERECHNUNG | General description | Issue E Year 2018 |
| | | Page 1 of 21 |
| <p>Luftfahrt-Technisches Handbuch (LTH)</p> <h1>HSB</h1> <h2>HANDBUCH STRUKTUR BERECHNUNG</h2> <p>Fundamentals and Methods for Aeronautical Design and Analyses</p> <p>issued by</p>  <p>INDUSTRIE- AUSSCHUSS- STRUKTUR- BERECHNUNGSUNTERLAGEN</p> | | |
| Prepared: Dr. M. Schagerl | Checked: Dr. M. Magin | Date: 22.11.2018 IASB / IVW |

- Design sheet contributor 1972-2015
- Co- reworker and Co- translator of the 'Airbus' structural handbook into English (2004)

IASB 1972-2024

First participation:

July 1972,

Session 12.

Last participation:

July 2024,

Session 185

at Ottobrunn.

22.11.2018, 13:24, page 1 of 21
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Digitally signed by Michael Magin
 Date: 2020.10.20 00:13:05 CEST



30.5 “ Weekly course: Development of Fibre Reinforced Plastic Components” (1986, 1989)



SHORT COURSE
DEVELOPMENT OF FIBRE REINFORCED PLASTIC
COMPONENTS

Pretoria

16th to 18th April 1986

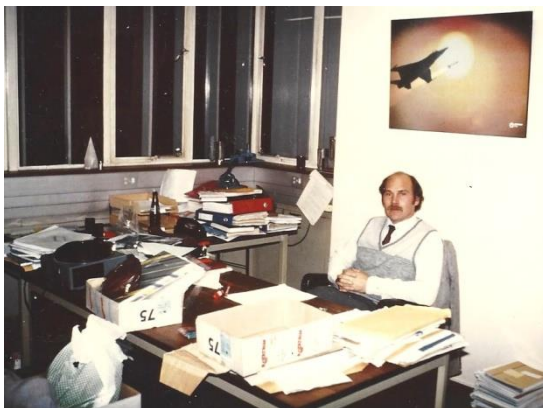
repeated 1989

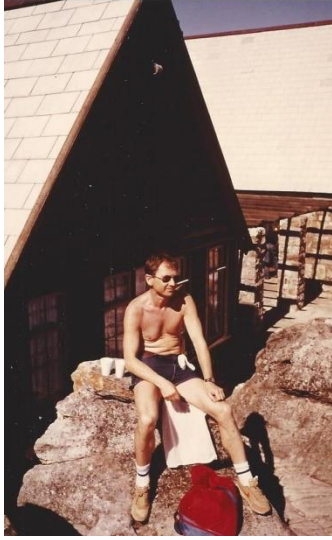
Presented by : Dr R G Cuntze
Course Organizer : P A Coetzer



Inviting person Prof. P. Haarhoff, right

Organizer P. A. Coetzer





After tackling the Table Mountain alone. Warning: climb through the area of very poisonous snakes



ein zebra einer herde von weit über 1000



30.6 ESA/ESTEC Contributions: Since 1970 for the ESA/ESTEC Material Handbook and down Organizer of below Working Group (WG), convenor and contributor”

ECSS-E-HB-32-24A
24 March 2010



Space engineering

Buckling of structures

ECSS Secretariat
ESA-ESTEC
Requirements & Standards Division
Noordwijk, The Netherlands


This Handbook has been authored and agreed upon by:

J. Arbocz, TU Delft
C. Bisagni, Politecnico di Milano
A. Calvi, ESA-ESTEC (Convenor)
E. Carrera, Politecnico di Torino
R. Cuntze, formerly MAN-Technologie
R. Degenhardt, DLR Braunschweig and PFH Göttingen
N. Gualtieri, Thales Alenia Space
H. Haller, Intales
N. Impollonia, Università di Catania
M. Jacquesson, CNES
E. Jansen, TU Delft
H.R. Meyer-Piening, ETH Zuerich
H. Oery, RWTH Aachen
A. Rittweger, Astrium EADS
R. Rolfes, Leibniz Universitaet Hannover
G. Schullerer, MT Aerospace
G. Turzo, CNES
T. Weller, Technion, Haifa
J. Wijker, Dutch Space

The valuable contributions of the following persons are acknowledged:

C. Huehne, DLR Braunschweig; D. Petry, Astrium EADS;
H. G. Reimerdes, RWTH Aachen; K. Rohwer, DLR Braunschweig;
R. Zimmermann, DLR Braunschweig.

The ECSS-E-HB-32-24 has been prepared by merging the volunteer contributions of the authors. Comments concerning the technical content of this handbook will be welcomed by the European Cooperation for Space Standardization, Noordwijk, the Netherlands, www.ecss.nl.

| | | |
|---|---|--------------------|
|  | BUREAU OF LIGHTWEIGHT STRUCTURES | Date : |
| | MINUTES OF MEETING | Feb. 02/03 2006 |

Location : INTALES at Technical University Innsbruck (A)

Object : ECSS-E-30-24 Buckling Handbook (BH): 4th meeting

Meeting Place and Time:

- First day: Start at 9.15 , End at 18.00
- Second day: Start at 9.00 , End at 15.30.

List of Participants:

| Name | Acronym | Company | Remark |
|--|----------------|-----------------------|----------------|
| Full members and supporting members | | | |
| Johan Arboez | JA | TU Delft | - |
| Paul Arendsen | PA | NLR, Holland | - |
| Gerd Bertrand | GB | AIRBUS-Germany, Hbg. | no response. |
| Vincent Billey | VB | EADS-ST LM | 02/03- |
| Chiara Bisagni | CB | Politec Milano | 02/03 |
| Adriano Calvi | AC | ESTEC | 02/03 |
| Erasmus Carrera | EC | Politec Torino | - |
| Ralf Cuntze | RC (convenor) | Bureau Cuntze | 02/03 |
| Richard Degenhardt | RD | DLR, BS | 02/03 |
| Bodo Geier | BG | DLR, BS | - |
| Nunziatino Gualtieri | NG | ALENIA Spazio | 02/03 |
| Herbert Haller | HH (secretary) | INTALES, Innsbruck | 02/03 |
| Felco L. Jansen | EJ | TU Delft | 02/03 |
| Marie Jacquesson | MJ | CNES DLA (EVRY) | 02/03 |
| Ali Limam | AL | INSA Lyon | - |
| H. R. Meyer-Piening | HRMP | ETH Zuerich | 02/03 |
| Huba Oery | HO | RWTH Aachen | - |
| Dirk Petry | DP | ASTRIUM EADS | 02/03 |
| Hans G. Reimerdes | HGR | RWTH Aachen | - |
| Michael Rotter | MR | Univ. Edinburgh | 02/03 |
| Andreas Rittweger | AR | EADS Bremen | never response |
| Raimund Rolfes | RR | Uni Hannover | 02/03 |
| Günther Schuller | GS | MT-Aerospace Augsburg | 02/03 |
| Guy Turzo | GT | CNES Toulouse | 02/03 |
| Jaap Wijker | JW | Dutch Space | - |
| Rolf Zimmermann | RZ | DLR, BS | retired |
| New and visiting members | | | |
| | | | |

Diffusion : participants + non-attending WG members + T. Henriksen + Athena Tilson.

Thanks to Herbert for providing us with such a nice weather.

Again, my thanks for the fruitful and effective discussions, for taking over work for the preparation of the BH draft, and to Michael for helping to substantiate 'our' ECSS handbook by inputting 'things' from 'his' civil engineering ECSS standard/handbook (of mixed type)!

Further Meetings planned:

- 5th Meeting: End of April, Bremen, EADS (Dr. Rittweger) + AIRBUS (Mr. H. Wellen)
Discussion of *Large Structure Examples*, permitted for being published in our Buckl. Hdbk.
To be fixed by RC with Rittweger and Wellen! Desired: April, 18-20
- 6th Meeting: planned: June 26-27, Edinburgh (Prof. M. Rotter)
- 7th Meeting: planned: Sept. 21-22, Milano (Prof. C. Bisagni)
- 8th Meeting: planned: first advent week (In 2006 is the latest possible begin!), Dec. 4-8 1, Munich, DP
- 9th Meeting:

DISCUSSION POINTS AND RESULTS

- 1) **Welcome, Introduction of new WG Member** Prof. Michael Rotter
- 2) **Discussion of MoM, 3rd Meeting, Hannover** No comment.
- 3) **Discussion on next meetings** Results see above.
- 4) **Discussion of Action Items List (AIL), 3rd Meeting, Hannover** Status of Actions
 - A19: Linked to A46 are linked concerning strategy. A common agreement is suggested. Impact from later presentation by Michael Rotter to be included.
 - A49: The tracking of doc: Marking for example by letter "X" and date for the version (whole doc). Result of discussion:
Name of doc.: (Sub)Chapter Title.(acronym date)
Example §1.3 Analysis Methods (NG11oct2005).
 - A62: CB tries further to get agreement from system engineer an input for §3.2
 - A64: CB did some analyses and the action is closed for CB. HH tries to do some analyses till next meeting
 - A70 RC provides MR and PA with info till 15.2.06
 - A77 will be taken over by EJ. RC will inform TW until the next meeting
- 5) **Discussion of contents of Website Folders of the Buckling Handbook.**
 - All folders of the BH Website have been looked at and actual uploaded contents discussed.
 - Direct actions are directed to the AIL
 - The Hdbk draft will be uploaded as .pdf file.
- 6) **References**
 - The reference list will be attached to each chapter
 - The references will be handled by full authors name for preparation
 - The citation will be changed to numbers after checking procedure of WG, the see A91, for final draft to be delivered before Public Review
 - Equations, Tables, Figures: follow the referencing system of the other ECSS-handbooks
 - Paragraphs: As it is.

- 7) **Checking/Reviewing of a Working Document = DRR folder** (proposition from NG)
 Presentation by RC
 - The DRR folder within a chapter folder is the area for submission of review results.
 - In order to not forget any correction proposal, comment etc. the official Document Review Record (DRR) form sheet for ECSS Standards shall be taken, see Website: DRR for E-30-24, template and for one example(of RC), see *DRRs folder for corresponding ECSS Standards*.
 Procedure if you wish to write a DRR, on chapter 2.3 e.g.,
- Download the template from the *Template Folder*
 - Update the completed file named, e.g. *DRR2_3 FlatPanels (EJ11oct2005). RC date*, as seen above, where RC here means the initials of the author of the DRR
 - Upload the document to the folder Working Documents
 §2.3 Flat Panels
 Chapter DRRs.
- 8) **+ How to get on BH Website?** Repetition for PA.
 GOOGLE→ECSS→Log in→My Team
- 9) **Presentation of Company INTALES** by HH
- 10) **Presentation about Shell Structures** (Analysis, Failure modes, ECSS Handbook) by MR.
 Prof. Rotter was invited to present in Innsbruck his institute + past ECSS activities + how ECSS may contribute to ECSS BH.
 There are valuable parts/ideas/figures/tables. However, the (legal) possibility of implementation of which will have to be considered by us when taking over something.
- 11) **Example for BEAM buckling** presented by HRMP
 Variable cross section values – Finite Diff. on Excel Sheet
 Example transferable to a solid propellant launcher on launch pad
- 12) **Commitment of RC and RD as compilers for §1.7**
- 13) **Full TOC urgently needed.**
 Therefore, RC needs the not yet structured SUB-TOCs of the respective compilers in order to complete the Handbook TOC
- 14) **Paul Andresen may take over §2.4 Flat Panels** (information from EJ)
- 15) **Drafting of §1.8**
 To A92: RR encourages JW to begin drafting.
 Also ask the contributors to give their statements by 1st march 2006.
- 16) **Proposal for § 1.7 „Manufacturing Aspects“** (see Website §1.7). Presentation by RC
 The proposal, necessary for the upcoming discussions of RC + RG with manufacturing engineers, has been discussed, simplified and uploaded:
 New: §1.7 Buckling implications of manufacturing and assembling processes
 1.7.1 Sensitive parameters for buckling
 1.7.2 Metallic materials

3 of 7

1.7.3 Composite materials.

- 17) **GT shall provide a presentation on the development of a launcher** for Bremen
- 18) **Discussion: Should software be annexed to the handbook** for design reasons, only?
 * Software: - not certified 'little' programs, no maintenance (author may be addressed)
 - Teaching software and checking tools for the more sophisticated FE codes
 - Not foreseen for production aspects
 * Description of Software: Scope, Theory, Limitations
 * Simple Program Structure.
 Final Conclusion of Discussion: Decision will be taken at 5th meeting in Bremen. Majority voted for an annex of little programs on EXCEL sheets etc. ..
- 19) **Lack of support by ECSS Secretariat** (reported by AC)
 Due to this lack the WG members are asked to produce the Figures and Tables as far as possible on their own.
 Discussion on a support, again, when the preliminary Hdbk draft will be finalized for Public Review.
- 20) **Which are the 'design limit states' to be applied in the non-linear buckling analysis?**
 Load reached at stop of a numerical computation; deformation or strain as a usability limit state; initiation of global yielding or of local yielding; initiation of inter-fibre fracture in a lamina of the laminate; delamination; or??
 To be discussed at Bremen.
- 21) **TOC Structure of Large Examples**
 A structure for the body text of the Large Examples was presented by RC (see Website §3.1) and discussed. GT recommends to give Test-Analysis Correlation an own chapter.
 This structure may be improved by WG members in order to find a more general structure. Of course, the structure can be tailored by each chapter compiler, if necessary.
- 22) **Virtual Testing shall be mentioned in the handbook**
- 23) **TOC Structure of Structural Element examples**
 A structure for the body text of these examples was presented by RC (see Website § 2.5, shared area) and discussed. The application was for a CFRP cylinder at DLR Braunschweig.
 This structure may be improved in order to probably find a more generally applicable structure. Of course, the structure can be tailored by each chapter compiler if necessary.
- 24) **Subgroup fixations**
 - Ali Limam, Paul Arendsen and Michael Rotter will take over work.
 - Rolf Zimmermann still contributes to the sub chapters he addressed before his retirement.

Best regards, Ralf & Herbert. 07/02/06

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ECSS-E-30.24 BUCKLING-HANDBOOK (BH)
 Status: November, 08/09, 2006 at MT Aerospace, Augsburg

ACTION ITEMS LIST
 for next meeting

| Action | Description | Actionee | Deadline | Status |
|--------------|--|----------|----------|--------------------------|
| 1. 27/04/05 | use of verbs | EG | 30/05/05 | closed |
| 2. 27/04/05 | sketch on validation/verification to Work Docs | RR | 15/05/05 | closed |
| 3. 28/04/05 | putting MoM attachments on website | AT | asap | ongoing |
| 4. 28/04/05 | WG BH member list | RC | 15/05/05 | closed |
| 5. 28/04/05 | review of NWI (basis for Work Plan discussion) | all | 20/06/05 | closed |
| 6. 28/04/05 | plausibility check list for analyses | RC | 08/07/05 | closed |
| 7. 28/04/05 | input glossary/definitions from STRUCTURAL | NG | 15/07/05 | closed |
| 8. 28/04/05 | clarification of 'consideration of operational aspects' | AC, MK | 20/06/05 | closed |
| 9. 28/04/05 | clarification of the meaning 'Documentation of lessons learned during design, manufacturing, verification and operations | NG, MK | 20/06/05 | closed |
| 10. 28/04/05 | copy right problem (we use source data) | EG, AC | 20/06/05 | response text required |
| 11. 28/04/05 | recommend. to fill in the date on 1 st page of Hdbk ? | RC | 20/06/05 | closed |
| 12. 28/04/05 | asking INSA colleagues for their vacant time | RC | 20/06/05 | closed |
| 13. 28/05/05 | provision of ASTM website on 'council on codes & standards, committee 60' | AC | 20/06/05 | closed, on website |
| 14. 28/04/05 | fill in the Report Form for EP | RC | July 05 | closed |
| 15. 28/04/05 | volunteers for contributions, part 1 through 3 | all | 20/06/05 | closed |
| 16. 28/04/05 | collection: contributions to sub-chapters | all | 20/06/05 | closed |
| 17. 28/04/05 | downloading possibility from our website: for members, only | Athena | asap | closed |
| 18. 28/04/05 | template for handbook from secretariat | EG, AC | 20/06/05 | open |
| 19. 28/04/05 | flow chart How to run buckling | HH | 20/06/05 | closed |
| 20. 28/04/05 | search for further volunteers for checking (especially those which cannot attend the meetings) | all | 20/07/05 | ongoing |
| 21. 23/06/05 | asking HRMP for a Part 3 structure example or a Part 2 structural element example | RC | 01/09/05 | closed |
| 22. 23/06/05 | contributions from EADS-ST France (Aerospatiale) required for §3.1 | VB | 15/09/05 | closed |
| 23. 23/06/05 | issue of an address list incl. acronyms (add to next MOM) | RC | 15/09/05 | closed |
| 24. 23/06/05 | does ESTEC perform Structure buckling tests? | AC | 15/09/05 | closed |
| 25. 23/06/05 | interviewing Dr. Vollath, VDI, for taking VDI2014 Guideline as a Reference Doc and its §2.2. Stability, as a Work Doc | RC | 01/09/05 | closed |
| 26. 23/06/05 | example Part 3: from Columbus receivable? NO! | NG | 15/09/05 | closed |
| 27. 23/06/05 | giving V. Billey and B. Geier full access to website | Athena | asap | closed |
| 28. 23/06/05 | processing and distributing of a Work Plan draft | RC | 01/08/05 | closed |
| 29. 23/06/05 | adding NASA docs to website AC provides link | EJ, AC | 21/07/05 | closed |
| 30. 23/06/05 | interviewing Dr. Clormann, MAN Technologie, for examples Part 3 (Lox-Tank, Front Skirt, Booster...). | RC | 15/09/05 | Closed and linked to A32 |
| 31. 24/06/05 | interviewing Hr. Wellen, (fuselage shells, composite bulkhead...), §3.3 | RC, GV | 15/09/05 | pending |
| 32. 24/06/05 | interviewing Dr. Rittweger, EADS-ST Bremen for examples, Part 3 (link to A 30 1) | RC | 15/09/05 | closed (AR agreed) |
| 33. 24/06/05 | interviewing Prof. Schwarmann, Airbus, for examples at Bremen and Hamburg | RC | 15/09/05 | closed |

All. Ootobnum 04/07/06

1 of 8

16.11.2006

| | | | | | |
|----|----------|--|-----------------|-----------------------|------------------------|
| 34 | 24/06/05 | attempting to put GARTEUR results on website: BG checks potential contributions (Nov. Hannover) | BG, RD | 15/02/06 | closed |
| 35 | 24/06/05 | upload Koiters article on curvature parameter | EJ | 01/08/05 | closed |
| 36 | 24/06/05 | PDF draft of BH to website before each meeting | RC | 10/09/05 | ongoing |
| 37 | 24/06/05 | presentation on COCOMAT (design verification, glossary, philosophy...) | AK | 2 nd meet. | closed |
| 38 | 24/06/05 | wrt. definitions adding BG article on Structural Behaviour to Work Doc | BG | 01/08/05 | closed |
| 39 | 24/06/05 | setting up and distributing a list of the Work Docs (although still on website) | HH | | closed |
| 40 | 24/06/05 | setting up and distributing a list of the Reference Docs (although on website) | HH | | closed |
| 41 | 24/06/05 | finding a specialist incl. compiler for § Manufacturing Aspects (see SMH group) | GT, RC, RD, all | asap 09/2006 | pending |
| 42 | 24/06/05 | informing RMP wrt Zurich on meeting requirements | RC | 01/08/05 | closed |
| 43 | 24/06/05 | implementing DRD Buckling as Work Doc | NG | 01/08/05 | closed |
| 44 | 24/06/05 | wrt "Engin. language": Can we get a PDF of Bush-Almroth?? If yes, add to Reference Docs | EJ | 01/08/05 | closed |
| 45 | 24/06/05 | wrt accurate property input for all materials (ductile aerospace metals, usually brittle composites, ductile GLARE hybrid composite, etc.): generate a table for the elasticity properties for the strength properties for the hydrothermal properties | RC RC RC | 01/09/05 | closed |
| 46 | 15/09/05 | Zuerich handbook flow chart at beginning is required | HH | asap | closed |
| 47 | 15/09/05 | checking whether the ECSS ordering terms for the Handbook (not a standard!); paragraph, section, chapter are correctly used ??? | EG | asap | closed |
| 48 | 15/09/05 | DRD on Buckling is added to website | RC | 01/08/05 | closed |
| 49 | 15/09/05 | Tracking of Buckling HB draft changes | ECSS + WG, AC | asap 15/01/06 | Still under discussion |
| 50 | 15/09/05 | Preparation of general list of symbols (collecting from members) as basis for discussions, this list will be enhanced if necessary | RC | 1/11/05 | closed |
| 51 | 15/09/05 | Input for action 50, Send to RC (nothing received) | all | 23/09/05 | closed |
| 52 | 15/09/05 | Discussion between NG + RR about §1.1 to finalize | NG, RR | 30/09/05 | closed |
| 53 | 15/09/05 | Ask HGR and HO to make a TOC §1.4 to be discussed at 3 rd meeting | RC | 15/10/05 | closed |
| 54 | 15/09/05 | §1.5.1 (new), §1.5.3 proposal for draft | RC | 4/11/05 | closed |
| 55 | 15/09/05 | §1.5.2 proposal for draft | CB | 4/11/05 | closed |
| 56 | 15/09/05 | §1.6.3 proposal for draft (see A100) | JB, RD | 15/10/05 | closed |
| 57 | 15/09/05 | Ask JB for contribution to §1.7 (JB cancelled membership) | GT | 15/10/05 | closed |
| 58 | 15/09/05 | Comments about §1.9 to CB | MI, GT | 15/10/05 | closed |
| 59 | 15/09/05 | Information from RC to compilers: Tanchum Weller for §2.3, Bodo Geier for §2.4, Jerome Buffe for several chapters in PARTs 1 to 3. | RC | 23/09/05 | closed |

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30.6 Author-founded Germany-wide group BetriebsfestigkeitsNachweis BeNa (in 2010) Büchlein

Founder of the German (beside all β, β, β , Schürmann, Gude,, β , Horst,?, ? and) Considering the high-performance UD lamina-composed laminates the classical fatigue tests are performed on laminates. The idea of the author-founded Germany-wide group BeNa (in 2010) with his colleagues however was to base fatigue life prediction 'embedded lamina-wise' in order to be more general in fatigue life design in the future and to save test CoSts and time. Associated test specimens shall capture the interface effect of the lamina embedded in the laminate.



Names:

30.7 “MAN-Project leader of the funded Research on UD Fracture Criteria”, SFC Puck

R. Cuntze, R. Deska und B. Szelinski, Karlsfeld
 R. Jeltsch-Fricker und S. Meckbach, Kassel
 D. Huybrechts und J. Kopp, Aachen
 L. Kroll, Dresden
 S. Gollwitzer und R. Rackwitz, München

Neue Bruchkriterien und Festigkeitsnachweise für unidirektionalen Faserkunststoffverbund unter mehrachsiger Beanspruchung – Modellbildung und Experimente –

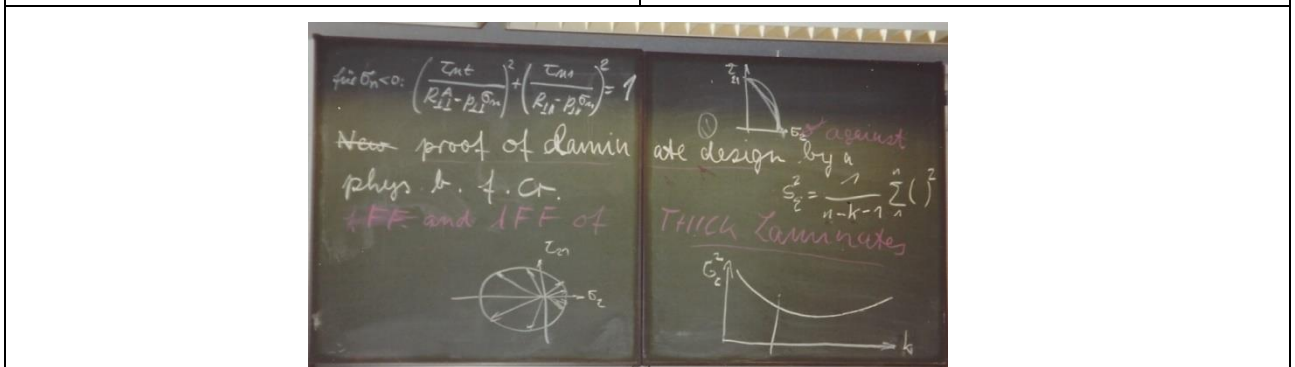
Reihe **5**: Grund- und Werkstoffe Nr. **506**

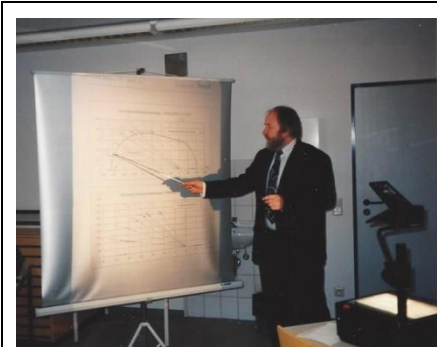


Meckbach, Jeltsch-Fricker, Rackwitz



Lothar Kroll, Alfred Puck





Bernd Szelinski,
MAN Part project
leader, informing the
team.

(right) Our
mathematician lady
honoured





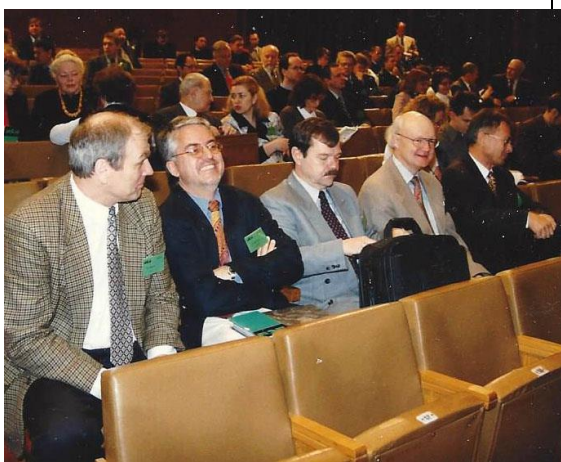
30.8 European Projecton



Jose Luis & wife
 P. J. C.
 Nigel Knobel
 & Polina
 Ralf Cuntze
 Rüdiger Ruchwald
 Gern Gollmann
 Mr. Gollmann
 Ralf J. Cuntze



30.9 Various Conferences on Composites

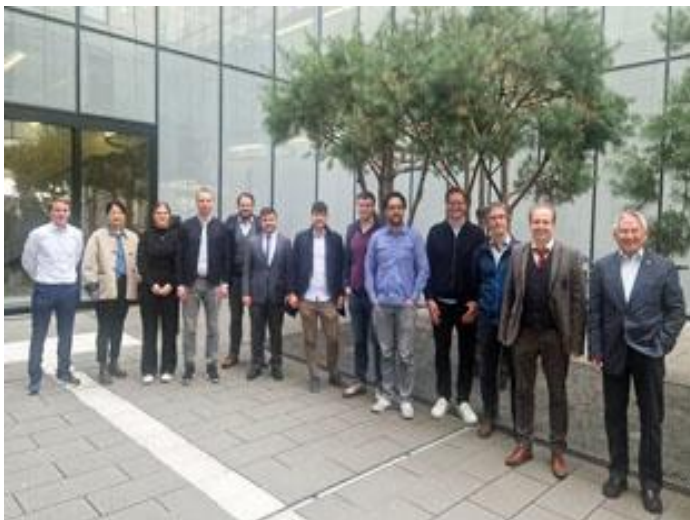


Rolands Rikards, Holm Altenbach, Holger Theilig, ??, Ralf Cuntze





Last Presentations 2023-2025



One of my last presentations:

*Static 3D-Strength Failure Criteria for the Structural Material Families Isotropic, Transversely–isotropic UD-Lamina and Orthotropic Fabrics.
And? Achievement of a Novel, Simpler Structural Mechanics Building.
on basis of Cuntze’s successfully Test Data-linked Failure-Mode-Concept (FMC)*

*DGM-Fachausschuss Hybride Werkstoffe,
21.2. 2024*





| | | |
|--|---|--|
| <p>TAGUNGSPROGRAMM: DIENSTAG, 26. FEBRUAR 1985</p> <p>9.00 BEGRÜßUNG: <i>Konferenzrat/Ing. M. Huter, Präsident der Vereinigung Österreichischer Industrieller, Sektion Tirol</i> <i>Prof. Dr. F. Ziegler, Vorsitzender des Nationalkomitees für Theoretische und Angewandte Mechanik</i></p> <p>KONSTRUKTIVER INGENIEURBAU <i>Vorsitz: Prof. Dr. G. F. Schuller, Universität Innsbruck</i> <i>Dr. F. Nahler, VOEST Alpine, Linz</i></p> <p>9.15 <i>Prof. Dr. W. B. Krätzig, Ruhr-Universität Bochum</i> Nichtlineares seismisches Traggerhalten von Stahlbetonsäulen</p> <p>9.45 <i>Prof. Dr. F. Rammerstorfer, Technische Universität Wien</i> Abheben und Beulen von zylindrischen Behältern bei Windbelastung</p> <p>10.15 <i>Prof. Dr. F. Fischer, Montanuniversität Leoben</i> Erdbebenbeanspruchte Tankbauwerke — gelöste und ungelöste Probleme</p> <p>10.45 KAFFEE</p> <p>11.15 <i>Prof. Dr. F. Ziegler, Technische Universität Wien</i> Ein praktisches Verfahren zur Wärmespannungsanalyse mit Erweiterung auf stofflich nichtlineare Probleme</p> <p>11.45 <i>Prof. Dr. H. Grunemann, Technische Universität München</i> Zum Schutz vor Erschütterungen und Körperschall</p> <p>12.15 <i>Dr. H. Nußbaumer, Siemens AG, Linz</i> Einige baodynamische Fragen des Kraftwerksbaus</p> <p>12.45 MITTAGSPAUSE</p> <p>ANLAGENBAU <i>Vorsitz: Prof. Dr. F. Ziegler, Technische Universität Wien</i> <i>Dr. M. Prodan, Motor Columbus, Baden, Schweiz</i></p> <p>14.00 <i>Prof. Dr. G. J. Schuller, Universität Innsbruck</i> Dynamische und bruchmechanische Probleme bei der Analyse der Rohrleitungen von Anlagen</p> <p>14.30 <i>Dipl.-Ing. N. J. Krutzik, KWU Offenbach</i> Dynamische Auslegung von Reaktorstrukturen bei äußeren und inneren Störfällen</p> <p>15.00 <i>Dr. M. Prodan, Motor Columbus, Baden, Schweiz</i> Anwendung bruchmechanischer und spannungsanalytischer Konzepte auf druckführende Komponenten</p> <p>15.30 <i>Dr. F. Nahler, VOEST Alpine, Linz</i> Probleme der Schalenstatik im Behälterbau</p> <p>16.00 KAFFEE</p> <p>NICHTLINEARE PROBLEME <i>Vorsitz: Prof. Dr. H. Troger, Technische Universität Wien</i> <i>Dr. J. Affenzeller, AVL, Graz</i></p> <p>16.30 <i>Prof. Dr. H. Mang, Technische Universität Wien</i> Finite Elementeberechnungen im Stahlbetonbau</p> <p>17.00 <i>Prof. Dr. W. Weitz, Universität Karlsruhe</i> Parameterschätzung und Identifikation dynamischer Systeme und Prozesse</p> <p>17.30 <i>Prof. Dr. K. Dreyer, Technische Universität Wien</i> Industrieroboter — Einige spezielle Probleme</p> <p>18.00 <i>Dr. W. Mitter, VEW Kapfenberg</i> Plastische Anomalien zufolge Änderung der Mikrostruktur</p> <p>18.30 Ende</p> <p>19.00 Empfang der Stadt Innsbruck und des Landes Tirol</p> <p>19.30 Bankett</p> | <p>MITTWOCH, 27. FEBRUAR 1985</p> <p>PROBLEME DES MASCHINENBAUS UND INDUSTRIELLE VERFAHREN <i>Vorsitz: Prof. Dr. H. Lippmann, Technische Universität München</i> <i>Dr. K. Schwaha, VOEST Alpine, Linz</i></p> <p>9.00 <i>Dr. R. Cuntze, MAN München</i> Strukturberechnungen heute — Auf was muß ich achten? Was brauche ich?</p> <p>9.30 <i>Prof. Dr. W. Scheible, Universität Stuttgart</i> Rechnergestützte technische Dynamik</p> <p>10.00 <i>Prof. Dr. H. Troger, Technische Universität Wien</i> Nichtlineare Stabilitätstheorie und ihre technischen Anwendungen</p> <p>10.30 KAFFEE</p> <p>11.00 <i>Dr. J. Affenzeller, AVL, Graz</i> Dynamische Probleme im Kraftfahrzeugbau</p> <p>11.30 <i>Prof. Dr. K. Wohlhardt, Technische Universität Graz</i> Zur Dynamik des Turbulenz-Mischers</p> <p>12.00 <i>Prof. Dr. G. Schweizer, ETH Zürich</i> Magnetaeder — Ein Beispiel für die Verbindung Mechanik und Elektronik</p> <p>12.30 MITTAGSPAUSE</p> <p><i>Vorsitz: Prof. Dr. F. Rammerstorfer, Technische Universität Wien</i> <i>Dipl.-Ing. N. J. Krutzik, KWU Offenbach</i></p> <p>14.00 <i>Prof. Dr. W. Schneider, Technische Universität Wien</i> Strömungs- und Erstarungsvorgänge in Metallen und Kunststoffen</p> <p>14.30 <i>Dr. K. Schwaha, VOEST Alpine, Linz</i> Möglichkeiten und Grenzen der theoretischen Analyse des Strangießprozesses</p> <p>15.00 <i>Dr. F. Strahmer, VOEST Alpine, Linz</i> Anwendung numerischer Methoden zur Strömungsberechnung bei hydraulischen Maschinen</p> <p>15.30 PODIUMSDISKUSSION <i>Vorsitz: Prof. Dr. D. F. Fischer, Montanuniversität Leoben</i> <i>Dr. G. Gsell, VOEST Alpine, Linz</i></p> <p>— <i>Dr. R. Cuntze, MAN, München</i> — <i>Dr. M. Prodan, Motor Columbus, Baden, Schweiz</i> — <i>Prof. Dr. H. Lippmann, Technische Universität München</i> — <i>Prof. Dr. G. Schweizer, ETH Zürich</i></p> <p>16.45 Ende der Tagung</p> | <p>EINLADUNG ZUR TAGUNG</p> <p>MECHANIK UND INDUSTRIE</p> <p>Kongreßzentrum Igls, Tirol 26./27. Februar 1985</p> <p style="text-align: center;"><small>MODRUGA KRATSBACH</small></p> |
|--|---|--|

- 8.30 Hajela : Expert Systems in Structural Modelling and Design
- Hoeltzel : An Interactive Hybrid System Approach to Near Optimal Design of Mechanical Components
- 9.15 Hartmann : Application of AI-Tools for Re-Analysis Within Structural Optimization
- Reitman : Optimization Problems on the Boundaries of Structural Engineering
- 10.30 Cuntze : Reliability - Oriented Optimization With Application
- Mlejnek : Some Contributions to Optimal Design Using Approximate Explicit Behaviour Models
- 11.15 Fonseca : Elastic Limit Design of Shells
- Kafkanoglu : Computer Aided Design of Power Shafts

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JNICT: Junta Nacional de Investigação Científica e Tecnológica
Instituto Superior Técnico
Universidade Técnica de Lisboa
MAGUE: Construções Metalomecânicas, S.A.R.L.
PECHINEY
European Research Office, United States Army
SOREFAME: Sociedades Reunidas de Fabricações Metálicas, S.A.R.L.
Escola Náutica Infante D. Henrique
BPA: Banco Português do Atlântico
TAP: AIR PORTUGAL
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Computer Aided Optimal Design

Structural and Mechanical Systems

Mota Soares

Volume 3

TRÓIA, PORTUGAL
June 29 - July 11
1986

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Dachauer Straße 667
8000 MÜNCHEN 50
Telefon (089) 1403279
München, 22.07.1983

Dr.-Ing. habil. RALF CUNTZE VDI

Hr. Prof. Dr. Schieleff, TU-Stuttgart
Hr. Dr.-Ing. Sperling, TU-München
Hr. Dipl.-Ing. G. Hirsch, RWTH-Aachen
Hr. Dipl.-Ing. Kießling, DFVLR-Göttingen
Hr. Dr.-Ing. Rother, Dornier
Hr. Dipl.-Ing. Stollie, VFW
Hr. Dr.-Ing. Bansemir, MBB-UD
Hr. Dipl.-Ing. Bauer, ERNO

Mitarbeit am HSB-Blatt "Gedämpfte Schwingungen"

Sehr geehrte Herren,

der Industrieausschuß für Strukturberechnung (IASB) bemüht sich für sein luftfahrttechnisches "Handbuch für Strukturberechnung" (HSB) um die Aufnahme eines Handbuchsblattes "Gedämpfte Schwingungen". Dieses soll einerseits dem Anwender eine schnelle Arbeit erlauben und andererseits auch den physikalischen Hintergrund nebst diversen Dämpfungsarten aufzeigen, damit eine schnelle Kontrolle der, bei Rechnungen getroffenen Dämpfungsmaßnahmen, möglich wird.

Dieser Blattentwurf enthält eine Ausarbeitung, die die verschiedenen, an mich herangetragenen Wünsche berücksichtigt. Leider fehlen noch die 2 Seiten Beispiele (S. 33, 34), da die Zahlenrechnung im Rahmen einer noch fertigzustellenden Semesterarbeit an der TUM erst geliefert wird. Im übrigen konnten aus Zeitgründen nicht alle Korrekturen seitens der DFVLR aber auch der Verfasser bei diesem Entwurf mit eingearbeitet werden.

In Anbetracht der Wichtigkeit dieses Blattes würden der IASB und speziell ich sich freuen, von Ihnen oder Ihren Mitarbeitern Anmerkungen und Verbesserungsvorschläge zu bekommen. Dieses betrifft auch Kürzungen. Beim vorliegenden Entwurf, gegenüber dem letzten, wurden bezüglich des Gesamtinhaltes schon einige Kürzungen durchgeführt.

Herzlichen Dank für Ihre Mühe im voraus

R. Cuntze

Kopie: Hr. Dr. E. Schreiner, MBB-UF
Vorsitzender des IASB

Groups/Groups -> Engineering). For this desired cross-linking, a sub-working group "Evaluation of NDI" is planned with the topics of maintenance, repair, inspectability, damage tolerance, and later recycling.

Another important sub-working group on the "Evaluation of new engineering software products" has already started its work. For this purpose, members are asked to list their software tools according to a fixed scheme, to submit evaluations and to formulate wishes. Some of the criteria to be applied here are: result quality, robustness, reliability, user-friendliness, cost, etc. The software companies are already involved in order to then specifically address the problems of the users.

For the next event, which will take place on 22.10.2010, the topic "Material characteristics and material characterization" was requested. For the members of the Engineering Working Group, the lack of reliable material characteristics (elasticity variables, strength parameters, etc.) for design and verification is also a major problem. In addition, validated strength conditions for the representation of multiaxial fracture stress states of the unidirectional (UD) layer are only available to a limited extent.

In this context, it is regrettable that the huge expenditures and valuable validation results of participants and organizers of QinetiQ (UK) in the implementation of the World Wide Failure Exercises-I and -II are unknown to many designers, but also to materials characterizers and test center employees in industry and institutes. WWFE-I had as its topic the validation of UD strength conditions with biaxial fracture stress states, while the WWFE-II, which is currently being phased out, is trying to do the same with provided 3D fracture stress states. The author was successfully involved in WWFE-I, and has experienced the difficulties of providing reliable test data - especially in WWFE-II: Test specimens, test benches, measurements and evaluation offer many possibilities for error. A test result is always experimental evidence, but almost never the complete experimental proof. A sensible balance of theory and experiment, in addition to good cooperation in both areas, is therefore imperative.

The CCeV strives to pass on experience so that some wheels are not reinvented by expensive experiments, but gaps are closed. For this purpose, contributions are also uploaded to the CCeV website, which help, for example, to determine the necessary multiaxial strength data.




ANHANG C**Mitgliederverzeichnis der für die Merkblatterstellung verantwortlichen DVM-Arbeitsgruppe "Kennwertermittlung und Anwendung der Bruchmechanik"**


Das vorliegende DVM-Merkblatt wurde in einer Gemeinschaftsarbeit von der Arbeitsgruppe "Kennwertermittlung und Anwendung der Bruchmechanik" innerhalb des "Arbeitskreises Bruchvorgänge", integriert im "Deutscher Verband für Materialprüfung e. V. (DVM)", unter der Federführung der beiden Leiter der Arbeitsgruppe, A. Cornec, J. Heerens, beide GKSS-Forschungszentrum Geesthacht, ausgearbeitet. An diesem DVM-Merkblatt waren nachfolgend aufgeführte Institute und Firmen und deren Mitarbeiter maßgeblich beteiligt:

| | |
|---|------------------------------|
| Bundesanstalt für Materialforschung und -prüfung (BAM), Berlin | K. Wobst |
| Fraunhofer-Institut für Werkstoffmechanik (IWM), Freiburg | B. Voß |
| GKSS-Forschungszentrum Geesthacht GmbH | A. Cornec J. Heerens |
| Institut für Eisenhüttenkunde (IEHK), Aachen | A. Heuser D. Dormagen |
| Interatom, Bergisch-Gladbach | H. Huthmann |
| Kernforschungszentrum Karlsruhe (KfK) | A. Brückner |
| Kraftwerk Union AG, Erlangen | A. Gerscha |
| Kraftwerk Union AG, Mülheim | F. Vahle |
| MAN-Technologie, München | R. Cuntze |
| Messerschmitt-Bölkow-Blohm (MBB), Bremen | K. Blomeier L. Schwarmann |
| Rheinisch-Westfälischer TÜV, Essen | B. Reimann M. Remke |
| Staatliche Materialprüfungsanstalt (MPA), Universität Stuttgart | U. Eisele E. Roos |
| TÜV-Hannover | E. Boroske |
| Universität Karlsruhe | W. Ehl |

Meetings of his Civil Engineering Working Group



universität
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„Automatisierte Fertigung im Bauwesen von Bauteilen mit Polymermatrix und Betonmatrix“ - Was gibt es Neues? -

2. Thementag 'Automatisierte Fertigung'

DACH-Veranstaltung von allen CC Bau-AG's, speziell AG 'Robotergestützte Fertigung'

Zielgruppen: Bauwirtschaft, Architekten, Planer, Handwerker, Auftraggeber, Zulassungsteilnehmer


am 13. Juni 2019 an der Uni Innsbruck, Technikerstrasse 21, A 6020 Innsbruck
Institut für Konstruktion und Materialwissenschaften, Arbeitsbereich für Massivbau und Brückenbau:
Univ.-Prof. Dr.-Ing. Jürgen Felix

Institut für Experimentelle Architektur, Arbeitsbereich Hochbau: Univ.-Prof. Dr. M. Colletti,
Dl. G. Grasser 0043 650 441 9086
CCeV-Kontakt Bernhard Jahn (Bernhard.Jahn@carbon-composites.eu)
Thementag: 95 € für Nicht-Mitglieder, frei für Vortragende. Presse.
Folien: englisch, deutsch. Vorträge: 25 + 5 min Diskussion.

| | |
|-------|--|
| 8:30 | Registrierung der Teilnehmer mit Kaffee, Brezeln, Getränke |
| 9:00 | Einführende Worte, R. Cuntze |
| 9:15 | Vorstellung des Instituts, M. Colletti, J. Felix |
| 9:30 | 3D Concrete Printing - from Lab to Practice, F. Bos, Uni Eindhoven, Holland |
| 10:00 | Eine mögliche zukünftige Bauweise für den Hoch- und Brückenbau unter den Gesichtspunkten von Lebensdauerkosten, Qualität, Erhaltung der Flexibilität des Ortsbetonbaus und BIM, J. Kurath, ZHAW Winterthur |
| 10:30 | Kaffeepause |
| 11:00 | Entwurfs- und Bemessungsstrategien für den Faserverbundpavillon auf der BUGA in Heilbronn 2019, V. Koslowski, ITKE (Knippers) Uni Stuttgart |
| 11:30 | Integration der Bewehrung in die additive Betonfertigung, V. Mechtcherine, Institut für Baustoffe, TU Dresden |
| 12:00 | Material und Prozess - adaptive Strategien in der Baurobotik mithilfe von Assistenzsystemen, S. Brell-Cokcan, Lehrstuhl für Individualisierte Bauproduktion, RWTH Aachen |
| 12:30 | Mittagspause |
| 13:30 | Technische Umsetzung des 3D-Betondrucks in praxisgerechte Fertigung, E. Athner, O Balog, Baumit GmbH/Textile |
| 14:00 | Modulares bauen bei Max Bögl, J. Schaffrath, Max Bögl, Neumarkt in der Oberpfalz |
| 14:30 | COEBRO - Additive Fabrication of Concrete Elements by Robots - Technologie und Anwendungsbeispiele, A. Trummer, TU Graz |
| 15:00 | Kaffeepause |
| 15:30 | Textile Bewehrung für den 3D Druck mit Beton, J. Konzalla, G. Grasser, Uni Innsbruck |
| 16:00 | Resümee, Abschlussdiskussion zur Inhaltsbelegung der AG "Robotergestützte Fertigung" |
| 16:15 | Institutsbesichtigung mit "Techn. Umsetzung des 3D Betondrucks in praxisgerechte Fertigung" |
| | Get Together |

Ihre Ralf Cuntze und G. Grasser

Hinweise: CC Bau, Fachabteilung des CCeV, besteht aus 4 AGs: Bemessung und Nachweis (Prof. Dr.-Ing. habil. Ralf Cuntze, Augsburg), Faserverbundarmierter Beton (Dr.-Ing. Ingelore Galtzsch, Leoben Dresden), Faserverstärkte Kunststoffe (Prof. Dr.-Ing. Jens Ritzewski, IMA Dresden), Robotergestützte Fertigung (Dipl.-Ing. Otto Kellenberger, AlphaTec) Nächste Termine der CC-Bau-AGs:
13. November 2018: CC Bau-Abteilungsverammlung mit 4 Fachvorträgen bei der HWTK Leipzig, März 2019: AG 'Faserverstärkter Beton' bei Neviges, Oktober 2019 AG 'Bemessung und Nachweis - vom Werkstoff bis zur Zulassung für Bauteile mit Polymermatrix und Betonmatrix' bei der Uni Weimar
2. Thementag 'Automatisierte Fertigung im Bauwesen', RC, 13.juni19_final



COMPOSITES
UNITED

CU-Thementag

„Vom Werkstoff bis zur Zulassung von Bauteilen aus Polymermatrix und Betonmatrix“

22. Oktober 2019 beim Institut für Angewandte Bauforschung Weimar gGmbH, Über der Nonnenwiese 1, 99428 Weimar

Zielgruppen: Bauwirtschaft, Architekten, Planer, Handwerker, Auftraggeber, Zulassungsteilnehmer
Veranstaltung der CC Bau-AGs 'Bemessung und Nachweis' und 'Faserverstärkter Kunststoff'

CCeV-Kontakt: Bernhard Jahn (Bernhard.Jahn@carbon-composites.eu)
Technikangehörige: 95 € für Nicht-Mitglieder, frei für CCeV-Mitglieder, Vortragende und Presse.
Folien: englisch, deutsch. Vorträge: 25 + 5 min Diskussion.

| | |
|-------|---|
| 8:30 | Registrierung der Teilnehmer, Begrüßungskaffee |
| 8:45 | Einführende Worte mit Ausrichtung CC Bau, R. Cuntze / J. Ritzewski |
| 9:00 | Vorstellung Institut: Dr.-Ing. Barbara Leydolph, IAB Weimar |
| 9:15 | Verwendete Baustoffe mit Schwerpunkt Carbonbeton: M. Butler, Institut für Baustoffe, TU Dresden |
| 9:30 | Fertigungsverfahren: Bauteile mit Polymermatrix, J. Ritzewski, IMA Dresden |
| 10:00 | Fertigungsverfahren: Bauteile mit Betonmatrix, G. Eisele, BCS Netz- und Spezialbaustoffe GmbH Dresden |
| 10:30 | Kaffeepause |
| 11:00 | Festigkeitsmodellierung der Baustoffe Beton und Lamelle, R. Cuntze, CC Bau Augsburg |
| 11:30 | Lasten, Sicherheitskonzept und Zulassungsspekte, S. Rempel, Solidon Albstadt |
| 12:00 | Dimensionierungsbeispiel, GFK-Behälter (nach BauV 2010), M. Oppé, Büro Knippers+Heibig Stuttgart |
| 12:30 | Mittagspause |
| 13:30 | Dimensionierungsbeispiel, Faserbetonbauteil, B. Wielek, Sistrans, Trol |
| 14:00 | Dimensionierungsbeispiel, CF-Textilbeton-Bauteile, S. May, Carbon-GmbH Dresden |
| 14:30 | Dimensionierungsbeispiel, CLT-Modellierung und Bemessung einer Textilbeton-Schale, D. Franitz, FE-Union GbR, Chemnitz |
| 15:00 | Kaffeepause |
| 15:30 | Abteilungsverammlung - CC Bau |
| 16:30 | Get Together |

Wir freuen uns auf Ihre Beiträge Ihre Ralf Cuntze und der Leiter CC Bau-Vorstand

Hinweise: CC Bau, ehemals CC TUDAU, Fachabteilung des CCeV, besteht ab 1.1.2018 aus den 4 AGs: Bemessung und Nachweis (Prof. Ralf Cuntze), Faserverbundarmierte Betone (Dr. Ingelore Galtzsch), Faserverstärkte Kunststoffe (Prof. Jens Ritzewski), Automatisierte Fertigung im Bauwesen (Ralf Cuntze) Termine 2020: Workshop am 19. Februar, Ulmer Betonstage bzgl. 'Wie bekomme ich Carbonbeton-Anwendungen auf die Baustelle?', CC Bau-AGs-Sitzung 17. März bei der Universität Siegen.

Thementag 'Bemessung und Nachweis', 22okt19_Cuntze 25jul19

CC Bau-Forum

“Carbonbeton-Anwendungen in der Praxis - auf der Baustelle und im Fertigteilwerk“

Transformation von praktischem Anwendungswissen.

Zielgruppe: *Hersteller, Sanierer, Vertreter der Betonbranche*

am 25. Februar, Donnerstag, als Podium integriert bei den

Ulmer Betontagen vom 23. - 26.02.2021

25 min Vortrag + 5 min Diskussion. Folien: deutsch oder englisch. Vortrag: falls möglich deutsch

| | |
|-------|--|
| 08:30 | Kaffeepause – Networking |
| 09:00 | Einführende Worte Prof. Dr.-Ing. habil. Ralf Cuntze, CU Bau, Augsburg, Composites United e.V. CUEV (formerly CCoV) |
| 09:15 | Digitale Entwicklung – von der Faser zum textilen Gelege. Roy Thyroff, rothycan – Roy Thyroff Consulting, Maila |
| 09:30 | Carbonbeton: Zwischen Theorie & Praxis. Dipl.-Ing. Oliver Hoppas, GOLDBECK Bauelemente Bielefeld SE |
| 12:10 | Größer, stärker, wirtschaftlicher - Carbonbewehrungen in neuen Dimensionen. Dipl.-Ing. Stephan Gießler, Solidan, Albstadt |
| 12:40 | Mittagspause und Besuch der Ausstellung |
| 14:00 | Tragwerksplanung des C³-Ergebnishauses „CUBE“ – Bemessung, Nachweiseführung und Zulassung im Einzelfall Dipl.-Ing. Hendrik Ritter, Assmann Beraten und Bauen, Dresden |
| 14:30 | Modulare Brückenbauwerke aus Carbonbeton M.Sc. Sven Boobach, Lehrstuhl und Institut für Massivbau, RWTH Aachen |
| 15:00 | Bauen mit CPC-Carbonbetonplatten, einer komplett neuen Bauweise in Beton – Verfahren, konstruieren, Bemessen. Prof. Joseph Kurath, ZHAW Winterthur, Schweiz |
| 15:30 | Stadtbahnbrücke Stuttgart: CFK-Zuglieder finden Akzeptanz. Prof. Dr. Urs Meier, EMPA, Dübendorf, Schweiz |
| 16:00 | Ausgewählte Ingenieurbauwerke - nachträgliche Bauteilverstärkung mit Carbon-Faser-Kunststoffen „CFK“ (geklebte Lamellen). Dr.-Ing. Horst Peters, HPTL Carbon GmbH, Ditzingen |

Ihr Ralf Cuntze und der weitere Vorstand des Netzwerks CU Bau

Weitere Termine 2020/2021 des Netzwerks CU Bau:

- 19.11. 2020: CU Thementag „Richtlinien, bauaufsichtliche Zulassungen und Bauangehmigungen für die potenziellen Anwender: Architekten, Tragwerksplaner und Bauherren“. Zoom-Konferenz
- 17.03.2021: Thementag aller Arbeitsgruppen des Netzwerks CU Bau mit TU Chemnitz. Zoom-Konferenz?
- 30.03.2021: 4. Thementag der AG „Automatisierte Fertigung im Bauwesen inkl. Serielles Bauen“. Zoom-Konferenz?

Das Netzwerk CU Bau besteht aus 4 Arbeitsgruppen: Bemessung und Nachweis (Prof. Ralf Cuntze), Faserverbundarmierter Beton (Dr. Ingelore Gatzsch), Faserverstärkte Kunststoffe (Prof. Jens Ritzewski), Automatisierte Fertigung im Bauwesen inkl. Serielles Bauen (Cuntze)

Agenda Carbonbeton-Forum Ulmer Betontage Cuntze 23Feb20

CC Bau-Forum

“Carbonbeton-Anwendungen in der Praxis - auf der Baustelle und im Fertigteilwerk“

Transformation von praktischem Anwendungswissen.

Zielgruppe: *Hersteller, Sanierer, Vertreter der Betonbranche*

am 19. Februar, integriert bei den

Ulmer Betontage vom 18. - 21. 2. 2020

| | |
|-------|--|
| 10:30 | Kaffeepause – Networking |
| 10:55 | Einführende Worte Prof. Dr.-Ing. habil. Ralf Cuntze – Composites United e.V (former Carbon Composites e.V.) |
| 11:00 | Verwendete Baustoffe Dr.-Ing. Marco Butler – Institut für Baustoffe, TU Dresden |
| 11:20 | Mehrfach gekrümmte Schalentragwerke aus Carbonbeton : Entwicklung wirtschaftlicher Betontechnologien zur Herstellung von Fertigteilen Dr.-Ing. Gerald Eisenwirth – BCS Natur- und Spezialbaustoffe GmbH Dresden Entwicklung einer Spritzbetontechnologie am Beispiel der „TWIST“-Schale des Ergebnishauses CUBE M.A. Dipl.-Wirtsch.-Ing. (FH) Mathias Teitze – Institut für Massivbau, TU Dresden |
| 12:00 | Einsatzbereiche von C³ – Carbon Concrete Composite aus Sicht einer mittelständischen Baunternehmung Dr.-Ing. Frank Jesse – Heitsche Bau GmbH, Bautzen |
| 12:30 | Mittagspause und Besuch der Ausstellung |
| 14:00 | Carbon-bewehrte Parkhausdeckenplatten: Von der Idee zur industriellen Fertigung Dipl.-Ing. Oliver Hoppas – GOLDBECK Bauelemente Bielefeld SE |
| 14:30 | Ökobilanz der Carbonbeton-Brücke in Albstadt-Ebingen Dr.-Ing. Sergej Rempel – solidan GmbH, Albstadt |
| 15:00 | Carbonbetontechnologie in der Sanierung Diplom-Ingenieur Annmar Al-Jamouh – Carbonbetontechnologie und Tragwerksplanung, Dresden |
| 15:30 | Kaffeepause |
| 16:00 | Die wirtschaftliche Carbonbeton-Wertschöpfungskette - am Beispiel der Vorzugsanwendung Carbonbeton-Fertigteilwand M.A. Dipl.-Wirtsch.-Ing. (FH) Mathias Teitze – Institut für Massivbau, TU Dresden |
| 16:25 | Projektkostenabschätzung – eine realisierbare Vision? Dr.-Ing. Arne Ostermann – 4 cost, Hamburg |
| 16:45 | Praxisbericht: Wann rechnet sich Carbonbeton – und wann ist seine Verwendung nachhaltig? Dr.-Ing. Frank Jesse – Heitsche Bau GmbH, Bautzen |
| 17:15 | Der besondere Beitrag der Ulmer Tagesschleuse – Info. wurden angefordert (Frau Blank) |

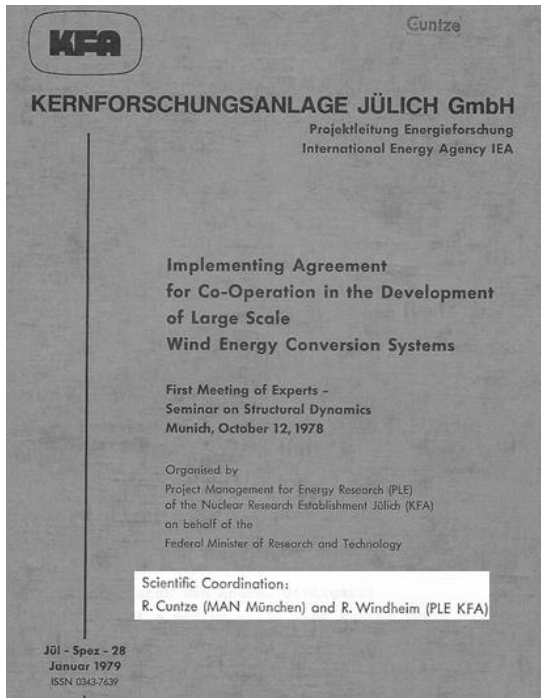
Ihr Ralf Cuntze und der weitere Vorstand CC Bau

Weitere Termine 2020 der CU Carbonbeton CC Bau:

- 17.03.2020 AG-Fachseminar Beton bei der Uni Siegen
- 17.03.2020 AG-Workshop zur automatisierten Fertigung im Bauwesen, Ort noch offen
- 10.09.2020 AG-Stellung bzw. Thementag aller AGs / TU-Chemnitz

Agenda Carbonbeton-Forum Ulmer Betontage Cuntze 19Feb20

Probably the first World-Wide Conference on Wind Energy



Bau, Test und Entwicklung großer Windenergieanlagen

**Seminar über Arbeiten im Rahmen des Programmes
Energieforschung und -technologien
des Bundesministers für Forschung und Technologie
23./24. März 1981**

Durchgeführt von der
Projektleitung Energieforschung PLE
in der Kernforschungsanlage Jülich GmbH KFA
im Auftrag des
Bundesministers für Forschung und Technologie BMFT

Leitung: R. Windheim (PLE) in Zusammenarbeit mit
R. Cuntze (MAN-Neue Technologie, München)

(left) International wind energy conference at MAN (1978),
(right) Seminar Research project from KFA Jülich GmbH (1981)
(below) contributors

C O N T E N T S

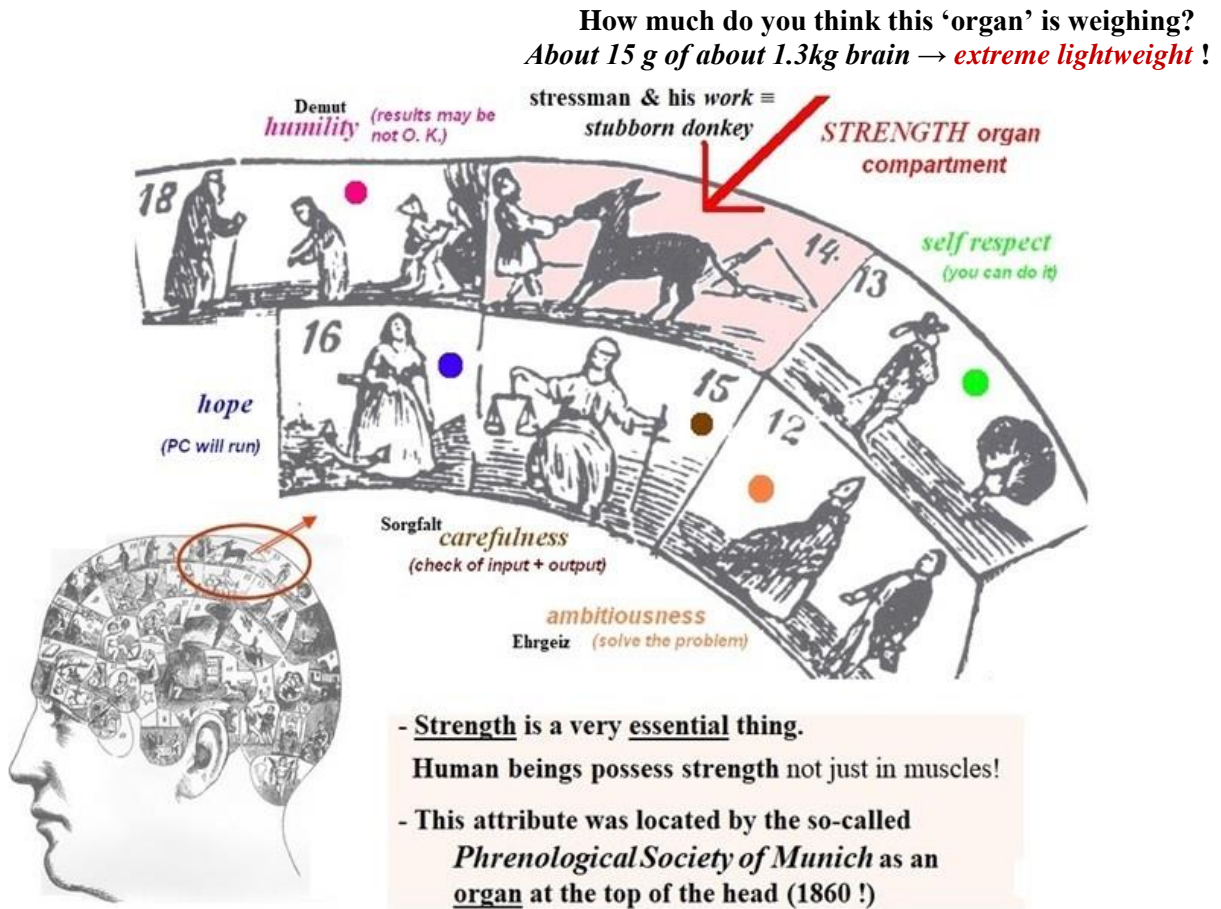
| | <u>Page</u> |
|---|-------------|
| C.J. CHRISTENSEN, P. LUNSAGER (Research Est. Risø) Investigations of structural dynamics on the Gedser WTG and new Danish wind turbines | 1 |
| P. LUNSAGER (Research Est. Risø) Measurements of performance and structural response of the Danish 200 kW Gedser windmill | 51 |
| O. FABIAN (Dept. for Fluid Mechanics, Techn. Univ. DK) A strategy for aeroelastic analysis of WECS | 95 |
| D. PETERSEN (DFVLR Braunschweig) Modes and frequencies of GROWIAN rotor blades | 105 |
| D. LUDWIG (DFVLR Göttingen) Dynamics of wind turbine rotor blades | 139 |
| F. KIESSLING (DFVLR Göttingen) Aeroelastic modelling of the coupled wind turbine rotor tower system | 161 |
| I. KOVACS (Leonhardt und Andrä, Stuttgart) Dynamic design of a medium stiff wind energy tower .. | 181 |
| R. CUNTZE, I. ZAUN (MAN - NT, München) Calculation of the natural frequencies of the braced GROWIAN tower with dead head mass | 197 |
| S.E. THOR (FFA, Stockholm) Computer methods for structural weight optimization of fiber reinforced plastics | 215 |
| D.A. SPERA (NASA, USA) Structural dynamics analysis of large wind turbines in the USA | 239 |

Two typical Bavarian peculiarities addressing Strength and Composites

From a present to the working group members of the Kurt A. Körber Stiftung on “How presenting mathematics to industry people”.

Members:

Standing in the medieval tradition of pictorial expression, the so-called phrenologists created a map of the brain by inscribed the abundance of mental qualities and tendencies of the human being – called organs – in the form of air bubbles in the skull. Phrenology is the study of the size and shape of people's heads in the belief that you can find out about their characters and abilities from this.



(For southern Germans: This has nothing to do with the well beloved ‘white-blue’ soccer club 1860 Munich, but it might stimulate more club success in future)

The phrenological theory of localization places the Strength organ’ in an excellent position of the skull,

- as a Stubborn donkey, which the strength man (14) has to tame, namely, between the 'organs'
- Self-respect (13), that one dares to approach a thing
- Ambitiousness (12), to solve the problem
- Carefulness (15), to check the computational output
- Hope (16), that hardware and software work
- Humility (18) when something was wrong.

Looking at the combination of skills:

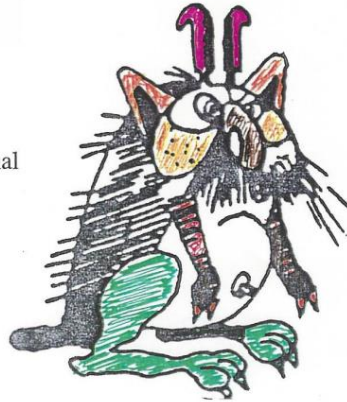
The Wolperdinger, the most famous Bavarian Composite. Animals like the Platypus have already coped very well with this. Our Bavarian Wolperdinger, on the other hand, is a typical Bavarian wish animal.

By the way, Wolperdinger and platypus have in common that they are very shy and nocturnal.

This does not predestine them as models for multifunctional strength criteria, because these are used unrestrainedly during the day without checking the special applicability limit.

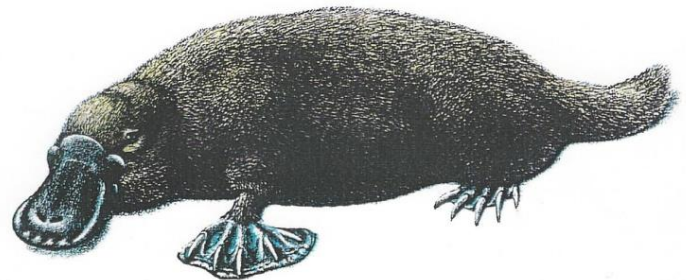


From Bavaria's
famous composite animal
called
WOLPERDINGER



Conference Greetings ICCM 11

to PLATYPUS
Australia's best tailored composite



Some other affiliations

- Member of DVM-WG "Characteristic Value Determination and Application of Fracture Mechanics". *Headed by K.-H. Schwalbe (1980)*
- Member of the Probability Working group of *G. Schueller (1986)*
- Founder and organizer of 4 Working Groups at Composites United e.V.
 - Engineering, *aerospace, mechanical engineering*
 - Composites Fatigue, *aerospace, mechanical engineering*
 - Design Dimensioning and Design Verification, *civil engineering*
 - Automated fabrication in construction including serial production
Including '3D-Printing', *civil engineering.*)

31 References, Cuntze since 2000 and Acknowledgement

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31.2 Thanks and Acknowledgement

Many Thanks to: My parents Helmy and Werner for my *inherited tenacity* to endure this hobby alongside the job, my first wife Jutta and to my second wife Maria for tolerating my private, very time-consuming scientific hobby and work.

The professores

- Hans Kauderer, E. Doeinck, Theo. Lehmann, Frederick-Henry Schroeder and Kurt Magnus for my knowledge of mechanics, which of course is still imperfect. Theo Lehmann and F-H Schroeder were my doctoral supervisors.
- Theo Lehmann, as well as the purple-robed jaguar driver Zerna, especially for the tensor analysis knowledge, which I was later able to use in the fiber laying winding technique
- Bodo Heimeshoff and Huba Öry for the self-critical way of working, which is later called 'self-testing' in ISO 9000. Bodo Heimeshoff and Harry Grundmann, who were my habilitation fathers in 1978
- Alfred Puck for many valuable discussions and his indirect initiation of my Failure Mode Concept (FMC) idea
- Rüdiger Rackwitz for the knowledge of structural reliability, applied in my FMC idea
- Karl-Heinz Schwalbe for my fracture mechanics knowledge
- Ms. Rita Jeltsch-Fricker for "Getting on the Right Mathematical Path" in a large MAN research project on A. Puck's Action Plane Stress criterion
- The honorary professor-reviewers Werner Hufenbach, Hans Eschenauer and finally Mr. Fahlbusch of the Bundeswehr University for the realization of the honorary professorship at the UniBw at Neubiberg

Thanks to all those who are not named, especially MAN colleagues and the members of the Aerospace-Technical Industry Committee for Structural Calculation (IASB), who contributed to both my professional and my scientific work.

Without the encouragement of Wilfried (Prof. Becker, TU Darmstadt) I would not have made it through the laborious work in the last two decades.

Yes, and where wouldn't I have gotten stuck everywhere in Mathcad-code calculations if it there would not have been Dipl.-Ing. Bernd Szelinski, who was usually able to eliminate Mathcad-attributable difficulties, such as changed letters, characters or that vectors were no longer read. It's a pity that such a great calculation program has such shortcomings and errors are incorrectly noted or it doesn't calculate and doesn't give a suspicion of error. Sometimes, another solution way could be gone. Not to forget
Dr.-Ing. Andreas Freund, with whose Mathcad skills I was only able to make the numerical entry into WWFE-I, culminating in a joint paper on WWFE-I A. Thank you Bernd and Andreas.

And, to my scientific friends and partners:

Bodo Heimeshoff, Huba Öry, Alfred Puck, Steve Tsai, Wilfried Becker, Holm Altenbach, Werner Hufenbach, Volker Ulbrich, Rolands Rikards, Lothar Kroll, Klaus Rohwer, Siegfried Schmauder, Gottfried Ehrenstein, Walter Wunderlich, Urs Meier, Frank Schladitz.

Looking at my Lessons Learned **LL**:

Much Experience is required in Design! But what is experience?
Experience is not what happens to you; it's what you do with what happens to you.

Albus Fluxley

“Theory is the Quintessence of all Practical Experience” (A. Föppl)

”Quid quid agis, prudenter agas, et respice finem” (a slogan of my school class)
Whatever you do, do it wisely and remember where it leads. (from *Gesta Romanorum*)

I hope that I followed this saying.

31.3 Resume

Life philosophy, experience

A lifelong close friend of Konrad Zuse and my father-friend once told me Zuse's retrospective:

"Looking back on my career, I can only hope that in the future there will be room not only for the specialist in his field, but also for the universally gifted. I believe that versatility in particular is the prescriptive for ideas that are out of the ordinary. Such an idea, an 'infidelity' of technology, if you will, was ultimately the computer".

In this context what I learned:

“Without versatility, I would never have been able to see the invisible bridges between the materials and their disciplines. Test results of related materials provide valuable information for the pre-dimensioning of the new intended material”. This saves time and money.

As far as new ideas are concerned, T.A. Edison is right: 1 % inspiration, 99% transpiration.

Never stop if people want you to give up or my mathematical code Mathcac.

With my FMC it looks more than 25 years like in short, unfortunately similar to the outstanding scientist Max Planck experiences, who said:

"It is one of the most painful experiences of my life that I have never succeeded in bringing a new assertion, for the correctness of which could provide a compelling theoretical proof, to general recognition. New scientific work prevails after the extinction of the opponents".

Where are the discussions in the contact-simple computer time?

Karl Poppers has summarized what is still very close to my heart in his 12 principles for a new professional ethic:

"It is impossible to avoid all mistakes or even all mistakes that are avoidable in themselves. Since we must learn from our mistakes, we must also learn to accept gratefully when others draw our attention to our mistakes."

I liked to have gratitude for that.

Much Experience is required in Design! But what is experience?

Experience is not what happens to you; it's what you do with what happens to you.

Aldous Huxley

Final note:

Above non-funded work mainly addresses the author's working life in industry with the intension to transfer theory into engineering application. This 'single authored' work includes: *Idea finding, *idea exploitation to generate the FMC-theory, *text writing of this document, *extensive numerical analyses with rendering of results like fracture bodies using the programs Mathcad 13 and 15, *typing of formulas, *difficult visualizations of the calculation results, sketches, diagrams etc. And, facing the program crashes of Word (twice, per day at least) presumably due to the large document and Mathcad 15 due to complicate computations. And that word corrections were indicated to heave been executed but in reality not performed.

All these works have been performed by the author himself.

Many thanks are given to those who believed in my work [*W. Becker*] and had some discussions with me on several topics [*Wolfgang Brocks with Fracture Mechanics, Andreas Freund with Mathcad 13 and UD mapping etc, Martin Grimmelt with Statistics, Probabilistics.*]. This thankyou includes the WWFE organizers within the performance of WWFE-I and –II from 1999 through 2013 especially *Sam (Kaddour)* and those that peered my WWFE-contributions. Not to forget are all those which delivered a basis for some body texts. *B. Szelinski helped not to despair of Mathcad.*

To my wife Maria: “*My heartfelt thanks for allowing me to devote so much time of ‘our time together’ to my time-consuming hobby*”.

Dedication to all the people who have guided me to the wide variety of technical fields during my 55-years working life. This was challenging but also difficult and at the same time beautiful. Never giving up was the motto.

Desire of the author:

“It were good for both the disciplines mechanical and civil engineering to act side-by-side such as croco and hippo document below.
Surprising picture, Sambia 2011.



”*Quid quid agis, prudenter agas, et respice finem*” (*a slogan of my school class*)
Whatever you do, do it wisely and remember where it leads. (*from Gesta Romanorum*).

In this context:

If I had known beforehand how much time this work would take, I probably would not have done it.

“*Si enim alicui placet mea devotio, gaudebo. Si autem nulli placet, memet ipsam tamen iuvat quod feci*”
(*A letter from Hrotsvit, nun of Gandersheim, 950 anno domini*)

If my work pleases anyone, I will be happy,
but if it pleases no one, it will nevertheless be good that I did it.

I am grateful that I was able to do all this.


Multi noscunt multa, omnes aliquid, nemo sati.
Many know a lot, everyone knows something, no one knows enough.

The above Latin saying is for me the basis for hopefully coming valuable discussions.

32 Affiliations, professionally and privately

32.1 German Society of Aeronautical Engineers (DGLR), for more than 40 years

32.2 IASB 1972-2024



**INDUSTRIE-
AUSSCHUSS-
STRUKTUR-
BERECHNUNGSUNTERLAGEN**

München den 20.7.72

PROF. DR.-ING. KH. NASITTA In Firma
Motoren- und Turbinen-Union München GmbH
8 München 50 : Postfach 50 06 40 : Tel. 0811/1 48 99 85

Protokoll über die 13. Sitzung des IASB
am 4. und 5. Juli 1972 bei Messerschmitt-Bölkow-
Blohm GmbH: Ottobrunn

Anwesend waren folgende Damen und Herren:

| | |
|---|---|
| Frau | Amann, MBB-UFE, Ottobrunn (5.7.) |
| Herren | Adolf, VFW-Fokker, Bremen |
| | Ahrendorf, IABG, Ottobrunn |
| | Bauer, ERNO, Bremen |
| | Berninghaus, MBB-UF, Ottobrunn (ABM) (4.7.) |
| | Birrenbach, Airbus, München (4.7.) |
| | Dr. Braun, MBL, München (4.7.) |
| | Dr. Cuntze, MAN, München |
| | Fonk, Fokker-VFW, Amsterdam |
| | Gaertner, MTU, München |
| | Dr. Geier, DFVLR, Braunschweig |
| | Gökgöl, MBB-UH, Hamburg (4.7.) |
| | Goße, Dornier, Friedrichshafen (4.7.) |
| | Grieser, IABG, Ottobrunn (ABM)(4.7.) |
| <i>later President of Indonesia</i> | Dr. Habel, MBL, München |
| ⇒ | Dr. Habibie, MBB-UH, Hamburg |
| | Hoffer, VFW-Fokker, Bremen (4.7.) |
| | Huth, VFW-Fokker, Bremen (4.7.) |
| | Jarosch, MBB-UD, Ottobrunn (4.7.) |
| | Johst, Dornier, Friedrichshafen |
| | Prof. Nasitta, MTU, München |
| | Oberparleiter, IABG, Ottobrunn (4.7.) |
| | Och, MBB-UD, Ottobrunn |
| | Pawaletz, MBB-UF/SB, München |
| | Römer, MTU, München (4.7.) |
| | Röper, Airbus, München (5.7.) |
| | Dr. Schreiner, MBB-UFE, Ottobrunn |
| | Dr. Schütz, IABG, Ottobrunn (4.7.) |
| | Schütz, D., LBF, Darmstadt (4.7.) |
| | Siegel, Airmetal, München |
| | Stolle, VFW-Fokker, Bremen |

32.3 Former private affiliations

10 years 'L'Ordre des Coteaux de Champagne'



Inthronisation, Champagner Empfang und Gala Diner
Hotel "Crown Plaza" Salzburg 6.Juni 2002

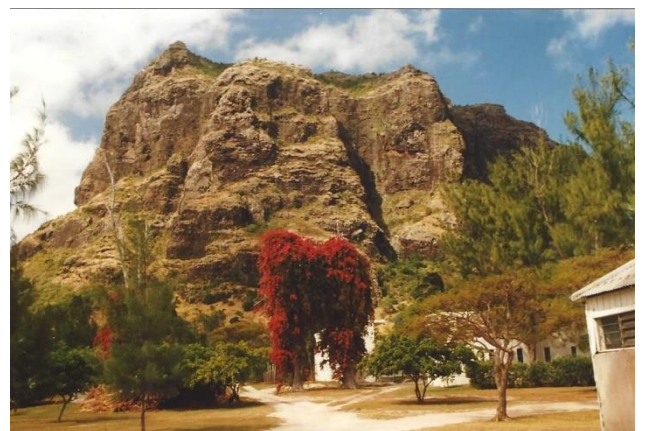
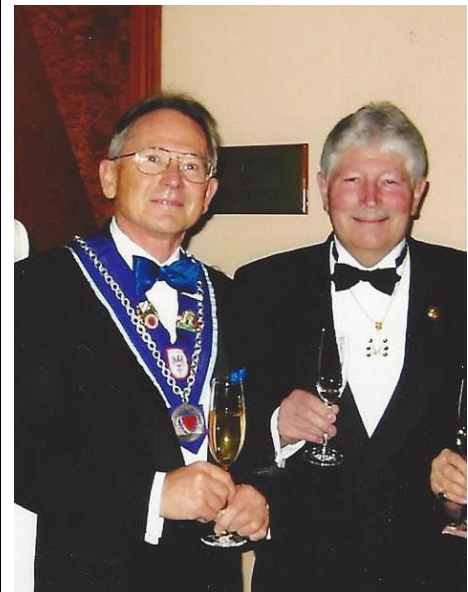
Internationales Grand Chapitre
d'Autriche 2002 Salzburg



Inthronisation, Champagner Empfang und Gala Diner
Hotel "Crown Plaza" Salzburg 6.Juni 2002

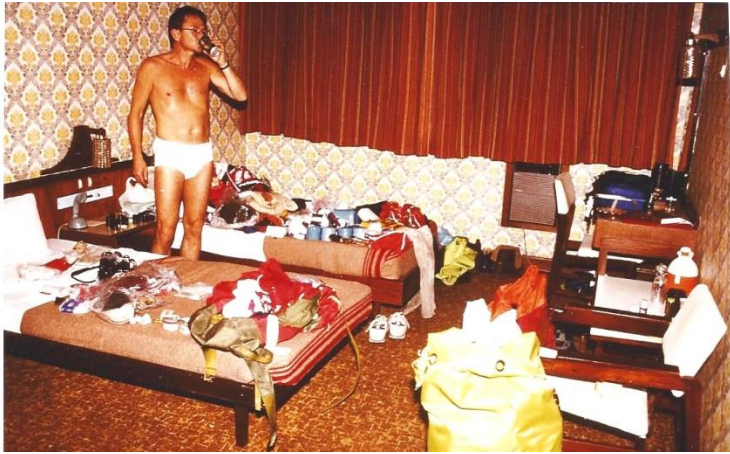
Internationales Grand Chapitre
d'Autriche 2002 Salzburg

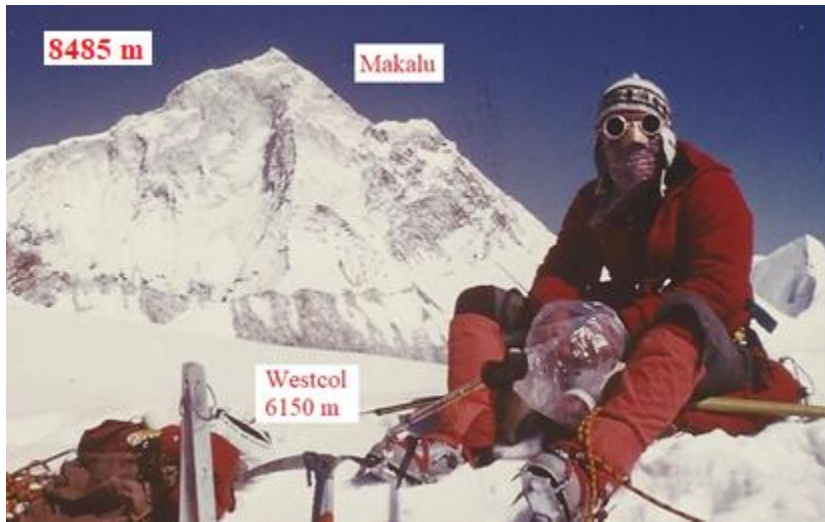
30 years 'Chaîne de Rotisseurs'



(left) Final Meeting of the German Mauritian Association.
founded by my friend Consul General Prof. Dr. Dr. Johannes Kneifel (left)
(right) Famous hill 'Le Morne', which I could still climb in 1986 with the hill-owning farmer

32.4 DAV = German Alpine Club

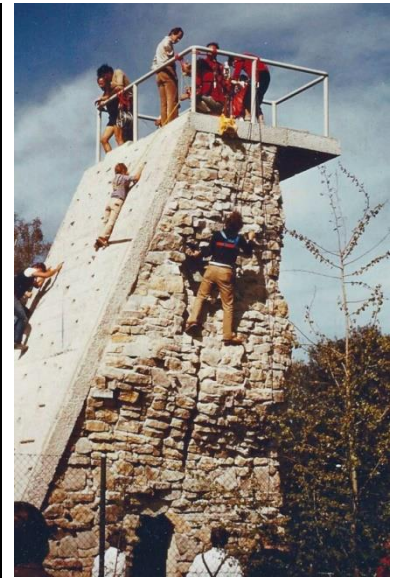




Since the porters with the equipment were missing, we had to take spare clothes.

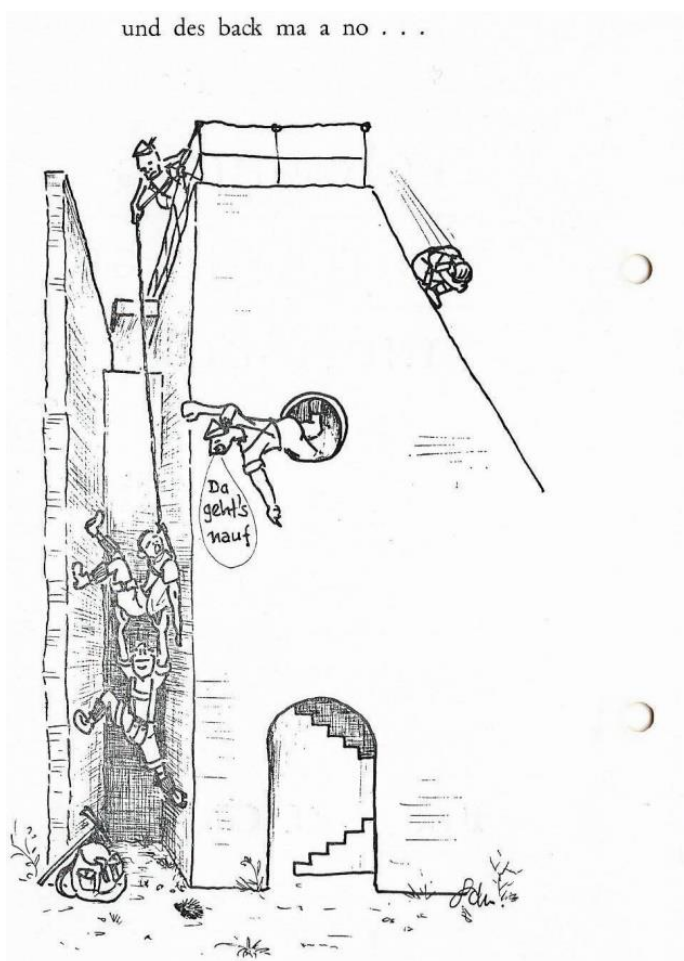
Consequence: Take what you have. Much is possible if a goal is pursued and this was also the case with having some fruitful ideas in this paper.

32.5 Hiking & Mountain friends Club Glonntal-Indersdorf e.V.



1980, inauguration of 'my' climbing garden

With the administrative officer of the County Dachau and world well-known climbers



Programm

Veranstaltungsort: Kletteranlage-Indersdorf
am Sportplatz in Markt-
Indersdorf

Beginn: 10.00 Uhr

Begrüßung der Gäste

Ansprachen von Landrat H.-J. Christmann
und vom Schirmherrn der Veranstaltung
Hans Strizner

Turmweihe

Klettervorführungen durch:

| | |
|-------------------|---|
| Michael Dacher | (4 Achttausender) |
| Otto Wiedemann | (einer der schnellsten Kletterer der Welt) |
| Sepp Gschwendtner | (einer der extremsten Deut- schen Freikletterer) |

Bergvorführungen durch:

die Bergwacht München

Den Stand der neuesten Ausrüstungen zeigen die Firmen
Scheck, Hanwag und Salewa.

Für Ihr leibliches Wohl sorgt ein Festzelt mit diver-
sen Schmankerln,
Bierpreis DM 3,90 je Maß



Birthday 66 years



Catholic Mass on a mountain



Indoor sports during winter time



Cleaning up at the Climbing garden. 2023



32.6 Good-Old-Boys and a –Girl of MAN



One of our yearly trips, 2010 and 2015



32.7 Some specific Conferences

Wissenschaftler erforschen, was ist. Ingenieure schaffen, was es nie gab. *Theodore von Kármán*

Lightweight Progress at least in conference material weight:

Transport of symposium documents: ICCM Hawai 1985 (books, 11kg, post office, received after 5 months),
NAFEMS (0.7 kg, direct home)

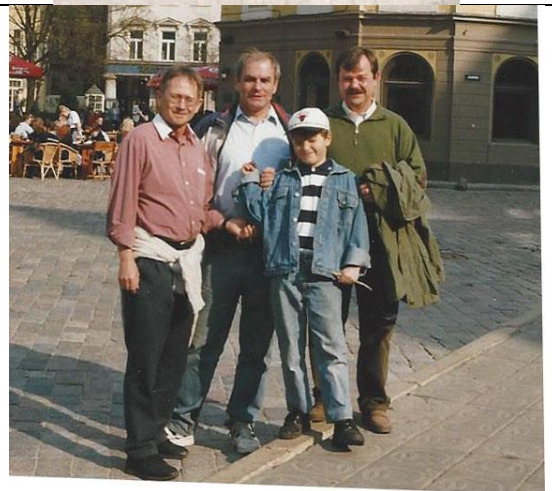


Conference Leeuven



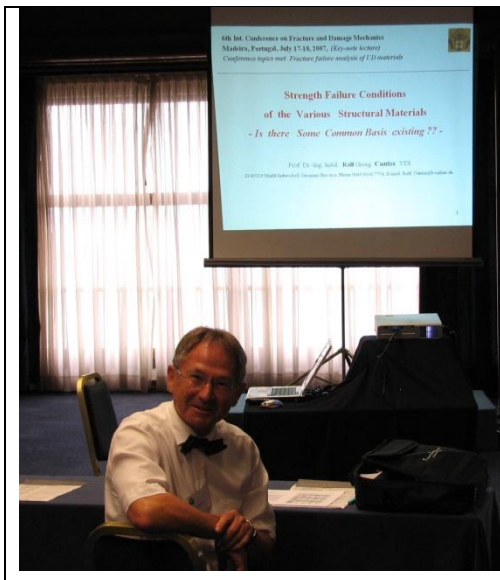


Conferences Paderborn





Madeira



Yakutsk Conference: On the Lena Rocks

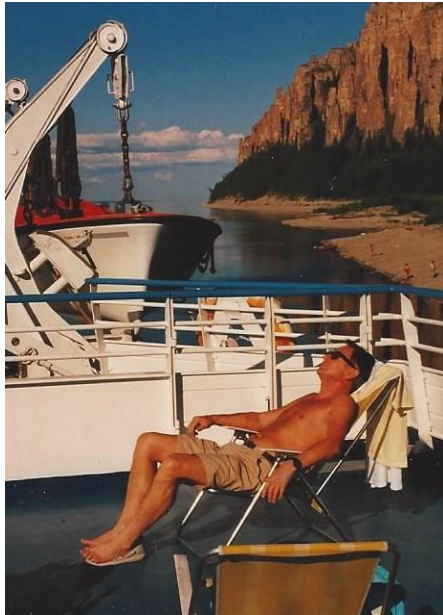
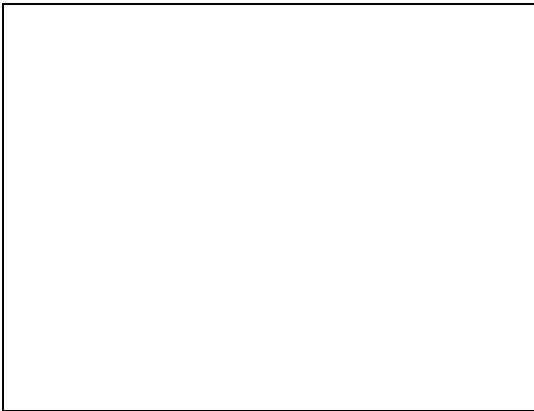
Свидетельство

РАЛФ

СUNTZE

является почетным гостем,
посетившим "9" июля 2001 года Долину Тейзеров,
один из уникальнейших уголков планеты Земля
Почетный гость располагает приятными воспоминаниями
и незабываемыми впечатлениями.





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Meetings at the house of Walter and Rotraud Wunderlich



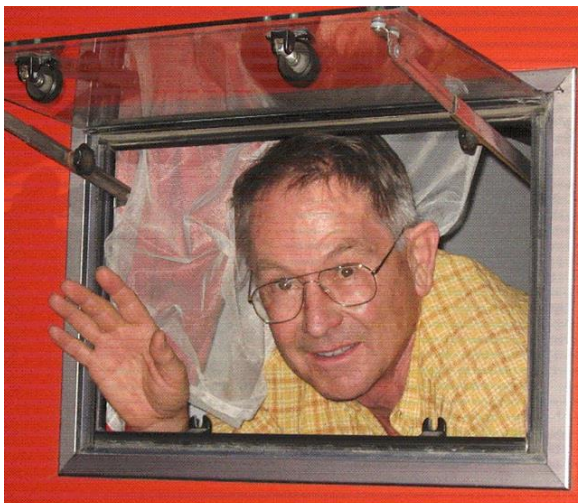
Knittel, Obermeyer, Ramm, Pfeiffer, Heimeshoff



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32.8 Rotel Tours



Cabin: 80cm x 80cm x 210cm

The relative narrowness of a ROTEL sleeping cabin helped the author to explore the wide world of nature and his good basic engineering education helped him to widely explore the world of engineering.



32.9 New engagement, 2015

Der Herbst ist ein zweiter Frühling,
 wo jedes Blatt zur Blüte wird.
 Albert Camus
 (1913 - 1960)



Einen Menschen lieben
 heißt einwilligen,
 mit ihm alt zu werden.

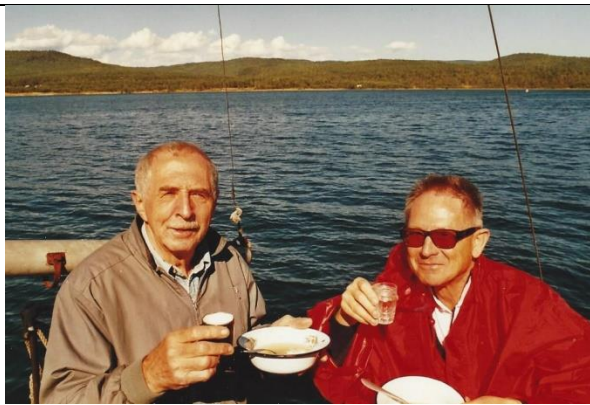
Wir haben uns vertraut,
 wir sind vertraut.
 Wir haben uns getraut,
 wir sind getraut.
 Die Mausi hat's erlaubt ?



Ralf & Maria.

On the Way with my Friends

Alois

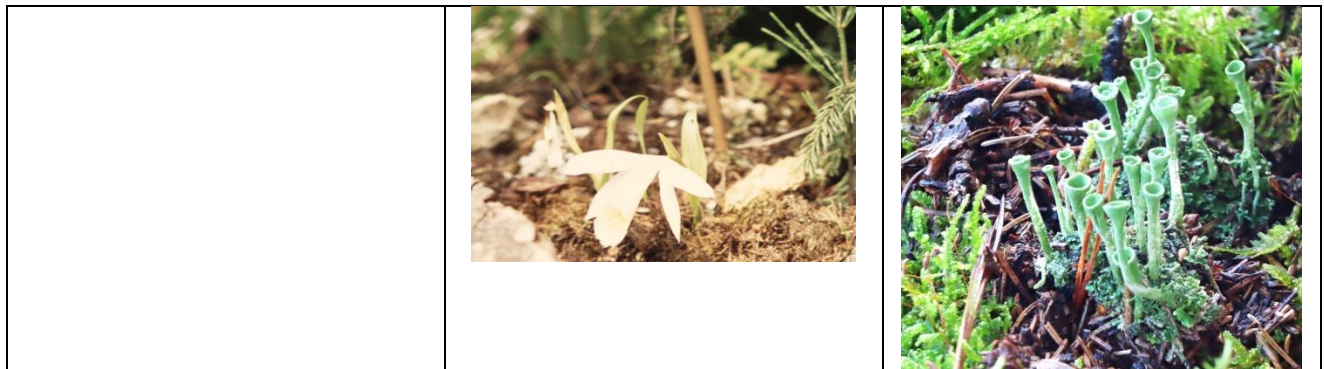


With my friend Anatol. Enjoying the Baikalsea, just before an extreme gust of wind put the sailing ship to the water



33 Documenting Hobbies: Globetrotter, Hiking, Photography, sparrows & alpine-cyclamen 'breeding'

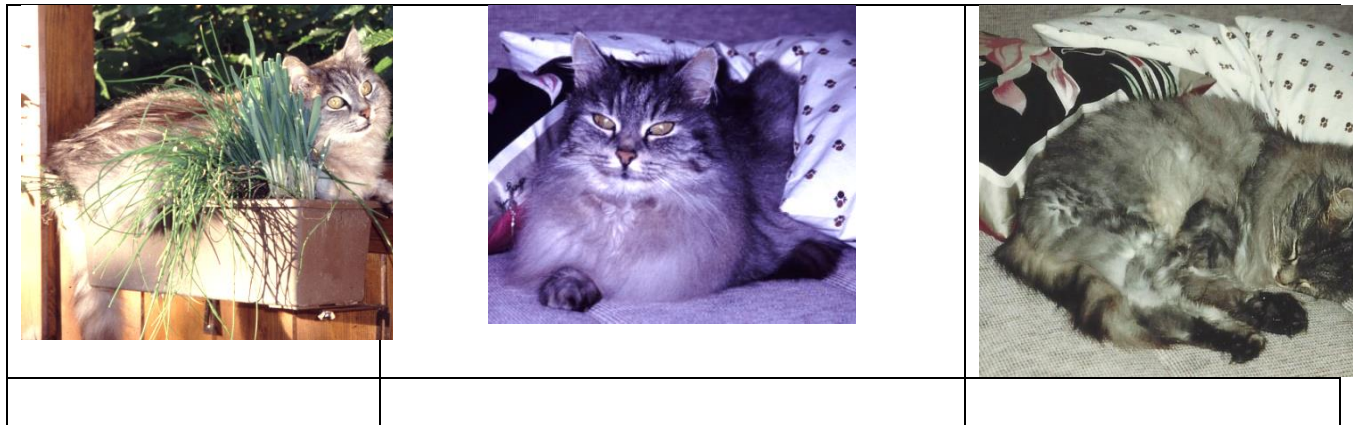
Flowers and Flower Curiosities





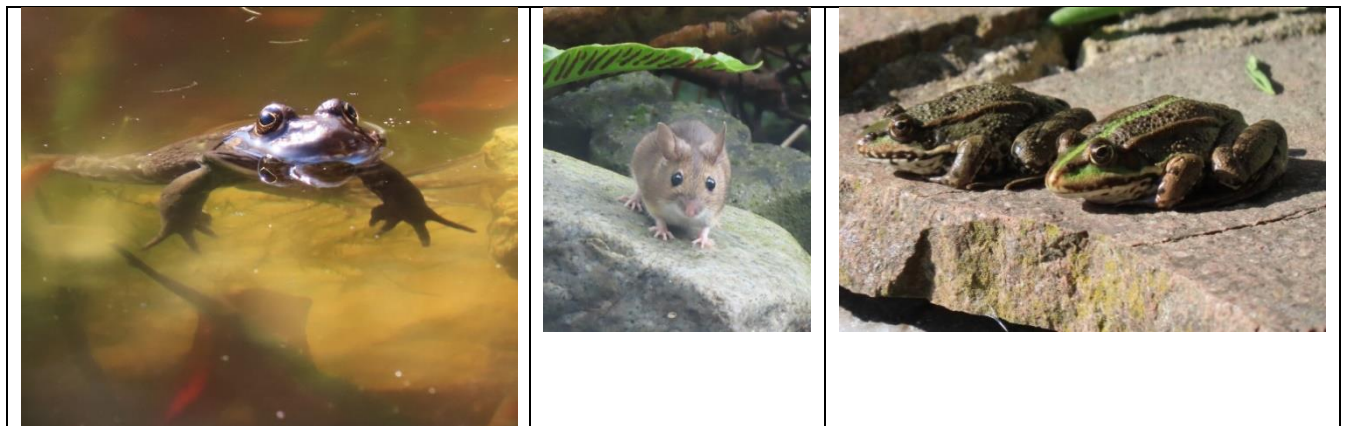
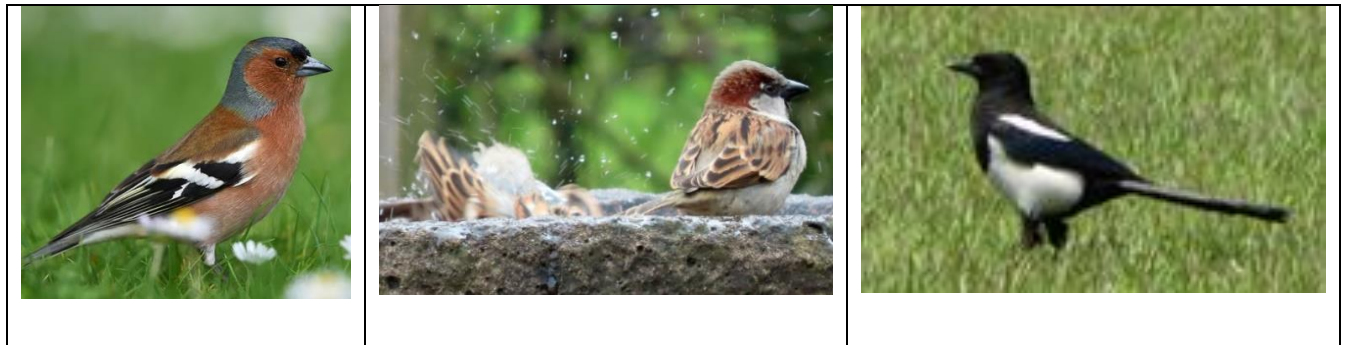
*

Pets and Animals in the Garden





Birds in the Garden

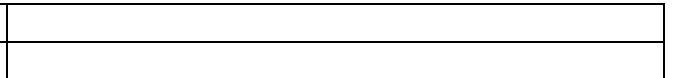
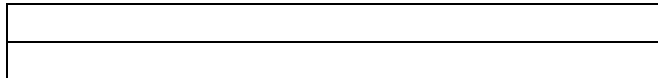
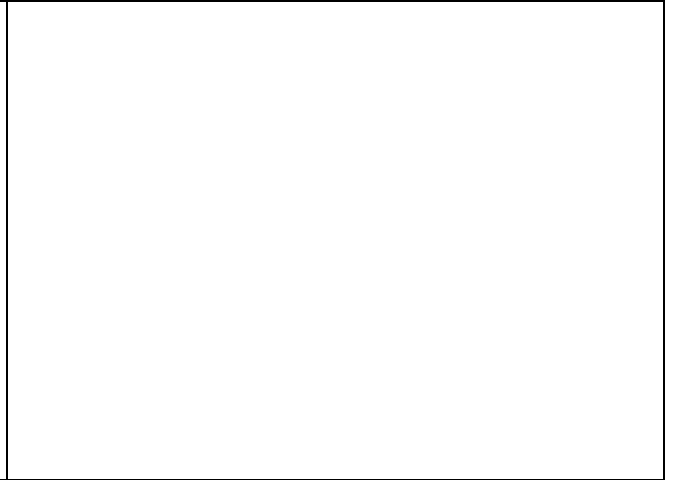
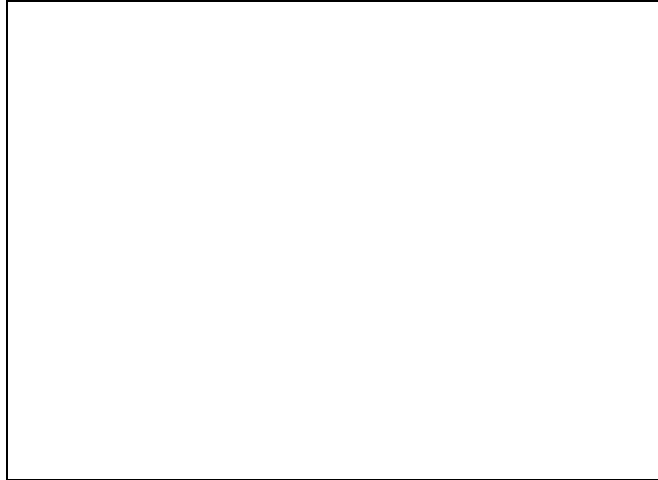


Flower pictures from around the world



80 Years Celebration





The Car Driver

Ford M15



Opel Manta with my mum



Mercedes 230



BMW 230



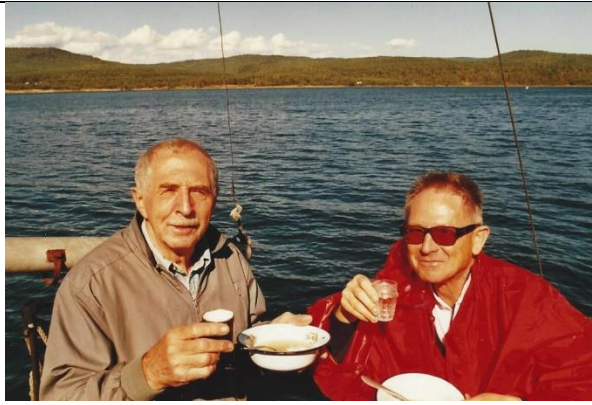
BMW ???



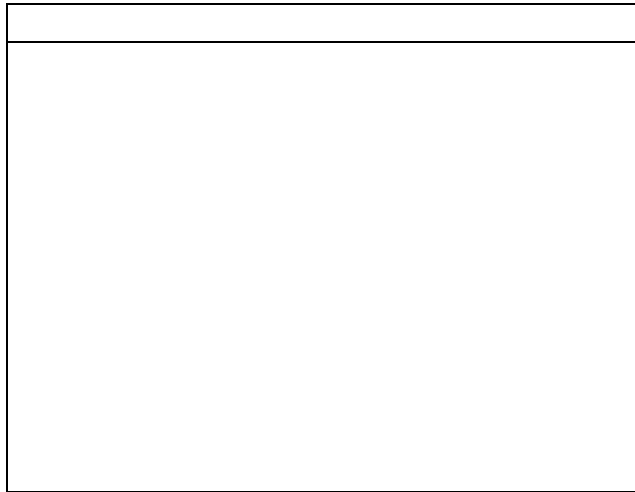
BMW 125

Sportmanship of the author

After I had to say goodbye to soccer at the age of 55 because I couldn't see the football on the ground quickly enough due to a weakness of the upper eyelid muscle, mountain hiking, cycling and jogging remained for the time being. From the age of 75 on, there are 2x circle training, cycling and a special type of 'jogging', protecting the knees, on Sunday morning, plus swimming in summer and gymnastics in winter. 10 more or less big hikes are added per year.



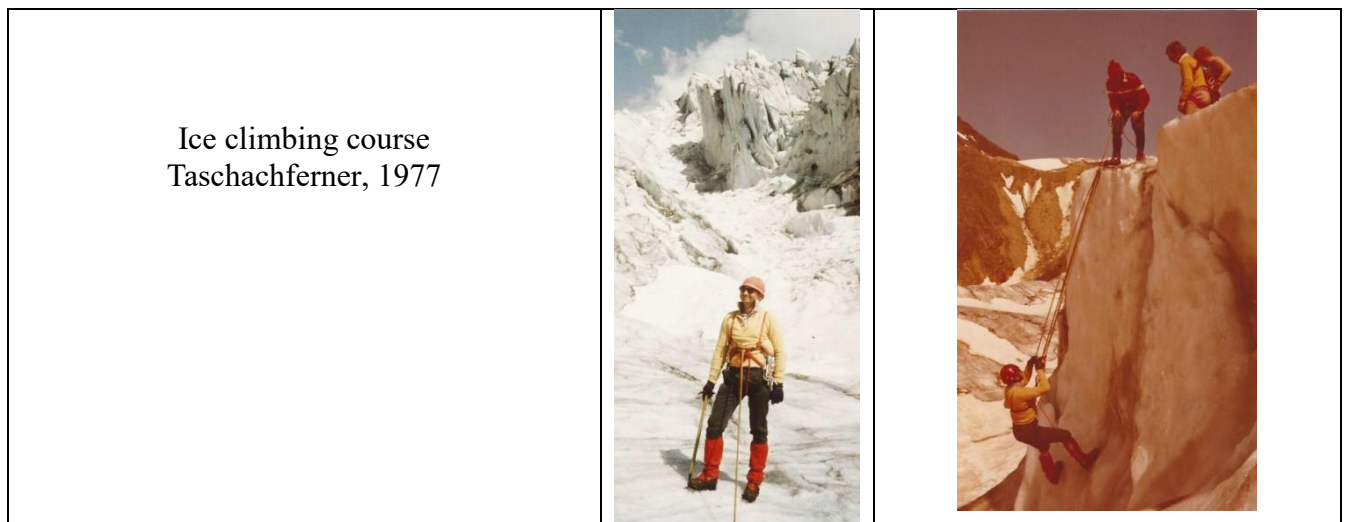
With my friend Anatol.
Enjoying the Baikalsea
just before an extreme gust of wind put the
sailing ship to the water



Dreaming of Visiting the World



Climbing Exercises

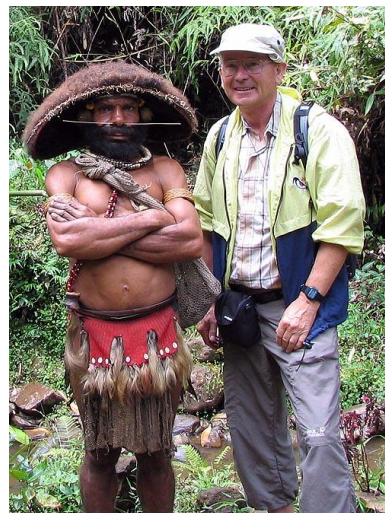
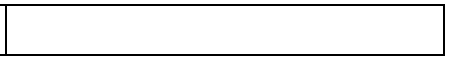
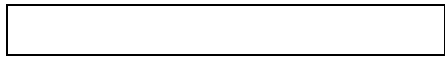
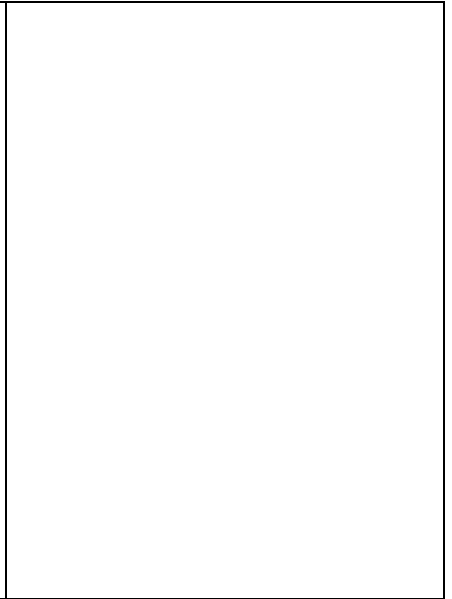
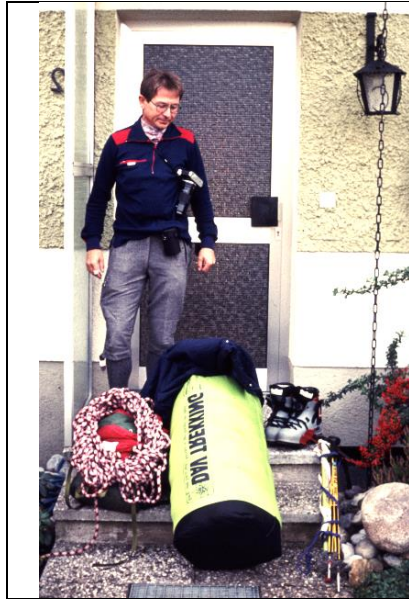


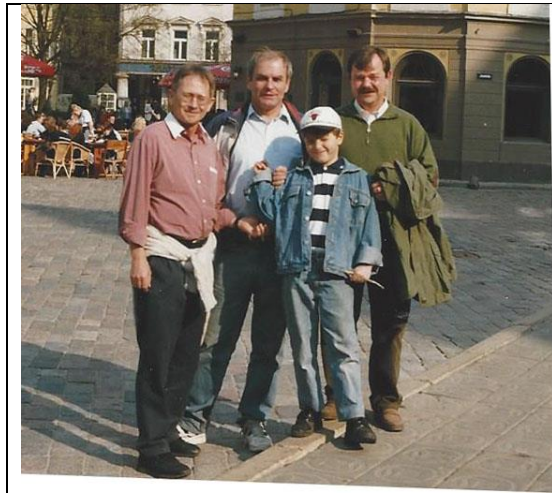
Saas Fee



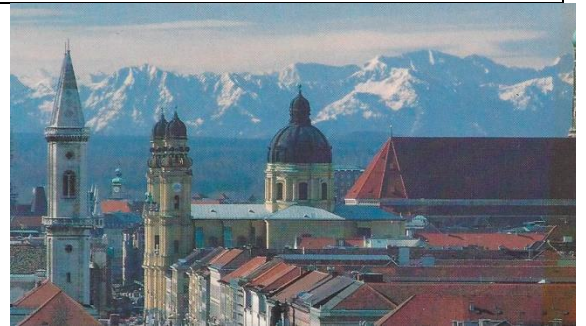
Dom







Hans Richard

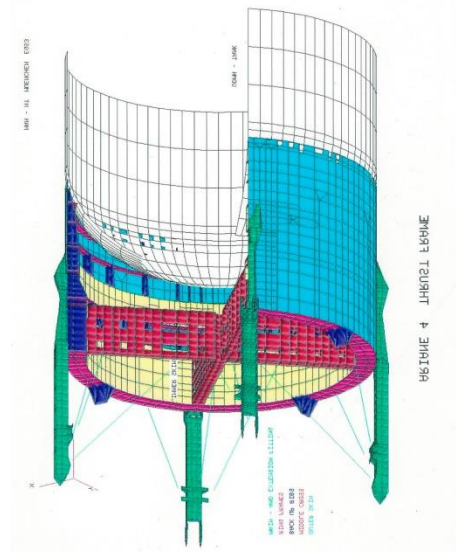


Yakutsk





Honorary Professor Presentation



Used Software Codes

Static, Dynamic and Thermal Structural Analysis Codes

* FEM: NASTRAN, MARC, NONSAP, SAPIV

* Finite differences: BOSOR, ROTOPT

* Pre- & postprocessor: PATRAN/II
with links to NASTRAN, MARC and NONSAP

. Thermal analysis, heat radiation, heat convection: MARC

. Composite laminates computation: MSV 2

. Viscoelastic, -plastic behaviour: MARC

. Fracture Mechanics: MARC

. Component Life, Crack Propagation: ESA-CRACK, NASGROW

. Special programs for fibre wound vessels,
bolt connection, ...

. Structure reliability analysis: FORM, SORM
with links to MSV 2 → FRP Rel

. Structure optimization: SAPOP

. Test data evaluation and monitoring: BASMAN
and rain flow counting code. Statistic codes, ...
EV CREEP (θ- projection concept) for fatigue loading

. Computer graphics package: DISSPLA

. Material databank composites

. Geometrical nonlinear beam analysis: PROFI + GROBI
for wind energy converter (GROWIAN, WKA 60, ...) statics

HARDWARE

Link-up MT Computer VAX 8550

4 TEKTRONIX 4115 Colour Graphics Display Terminal

1 Evans & Sutherland PS390 Colour Graphics Display Terminal

2 TEKTRONIX 4692 Colour Graphics Copier (Inkjet), DIN A 4

Personal Computers. Workstation DEC 2000



In the Alps and at home



(left) **Ralf & Maria.**

(right) The relative *narrowness* of a **ROTEL sleeping cabin**, 80cm x 80cm x 210cm) helped the author to explore the *wide* world of nature and his good basic engineering education helped him to widely explore the *wide* world of engineering



Mountain luggage

Ras Dashen, East Africa, 4550 m

(left) Hiking in the Alps. (right) West Col 6150m



Venezuela
Me and the guide

Venezuela

“Challenging
tasks
strengthen!”



Leopard, hyacinth, elephant

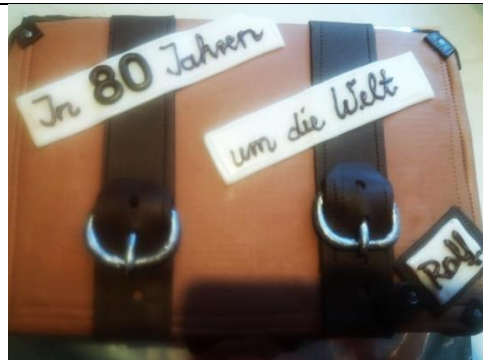
Deamavend Iran, 1975 (the 'half-'person left)

Fitz Roy hiking, Patagonia, 1987

Largest moth in the world

Smallest ape, Makaki

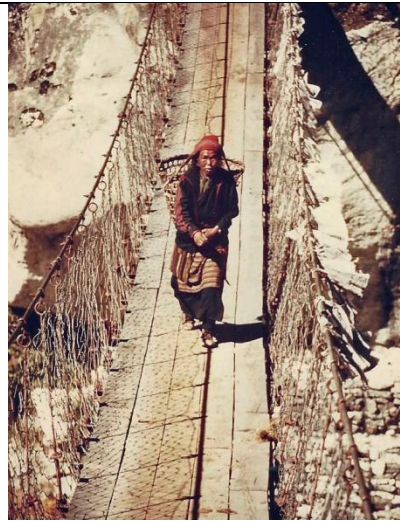












Albtraum einer Brückenüberfahrt

Cuntze: Sikkim, 2012

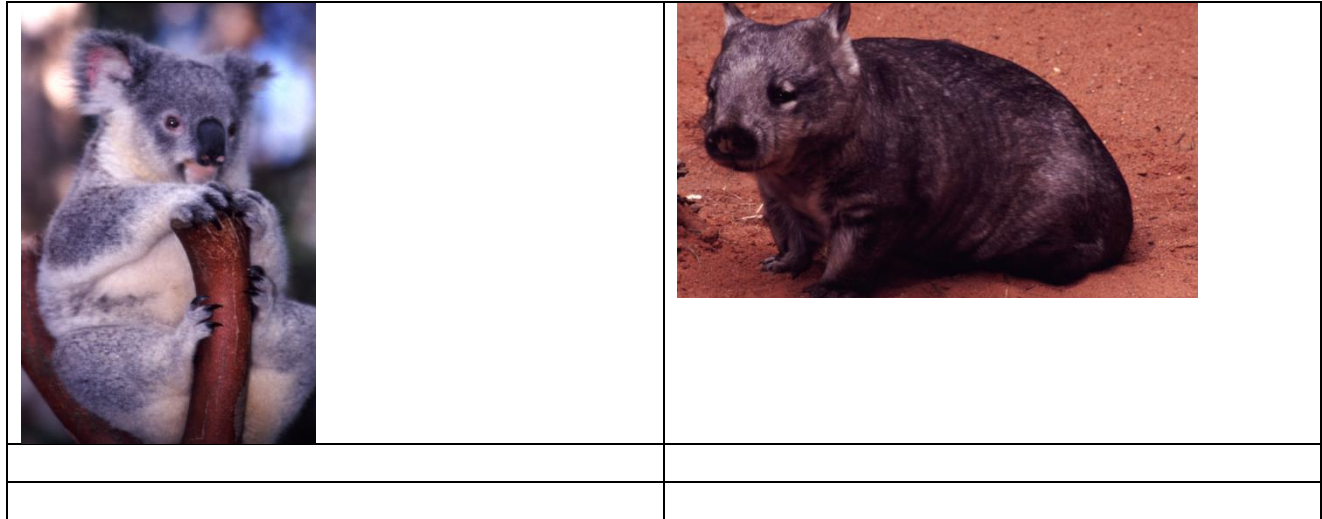


Nightmare of a bridge crossing

**Pinatubo, Renatured after about 15 years.
Nature repairs**

Sportmanship of the author

After I had to say goodbye to soccer at the age of 55 because I couldn't see the football on the ground quickly enough due to a weakness of the upper eyelid muscle, mountain hiking, cycling and jogging remained for the time being. From the age of 75 on, there are 2x circle training, cycling and a special type of 'jogging', protecting the knees, on Sunday morning, plus swimming in summer and gymnastics in winter. 10 more or less big hikes are added per year.





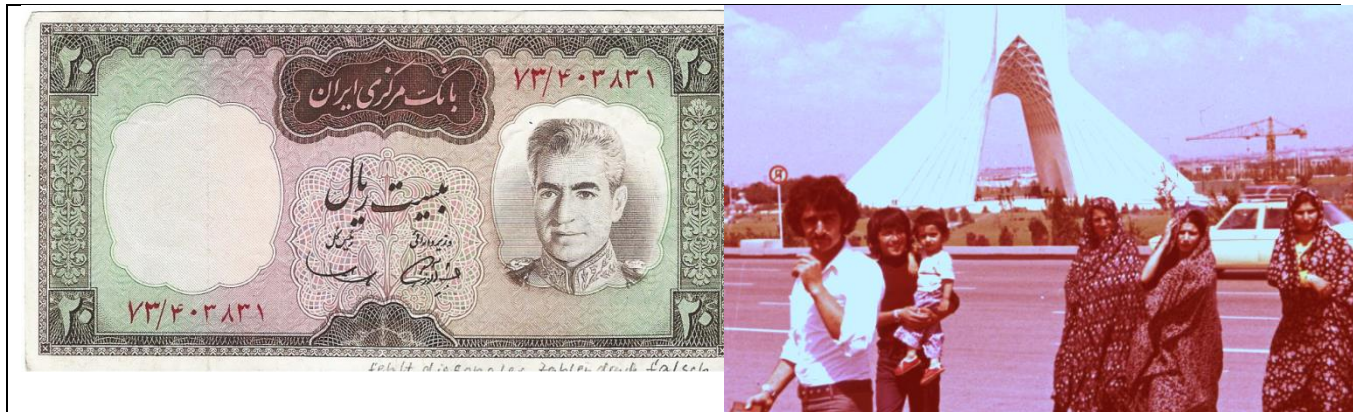
Europe: UK, Portugal, Spain, Denmark
1972 Tunisia
1975 Iran
1979 Peru
1980 Ladakh
1983 Himalayan Tour Nepal
1986 Alaska
1991?? Myanmar, Thailand, Laos, Cambodia, Saigon
Russia: Moscow 2007, Irkutsk 200?, Yakutsk 2001 Kamptchatka 200?
1997 Australia
1997 New Zealand
???? Pacific Islands
2002 Emirates, Cuba
2005 Crossing China from Kazakhstan (Ikarus)

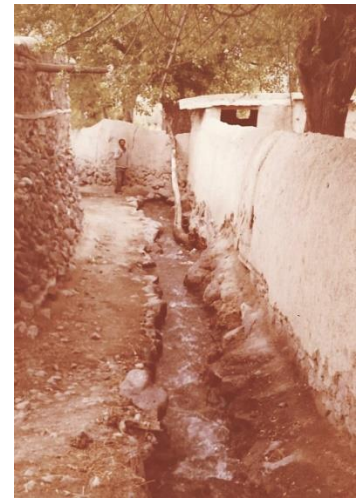
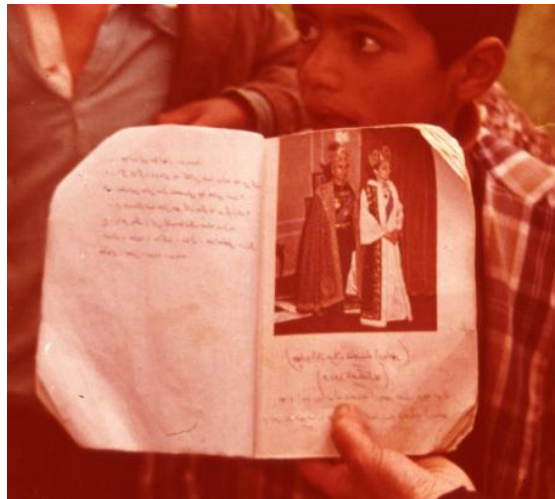
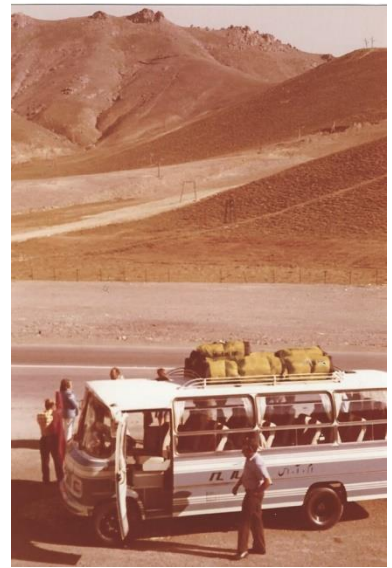
2005 Syria, Lebanon, Jordan
2007 Uzbekistan (Ikarus)
2008 Panama→Mexico (ROTEL)
2009 Yemen
2009 Paraguay
2011 Crossing Africa (ROTEL)
2012 Crossing India (ROTEL)
2012 Kolkatta, (Ikarus)
2013 Venezuela
2013 Marokko
201? Namibia
Scandinavia
2020 Island
2023 Oman
Bolivia
Philippines
Indonesia
Papua-Newguinee
North-Corea
1987 Argentina, Patagonia
Italy: Apulia, Sicilia, ..
South India-Sri Lanka
South European Countries
Azores, Madeira, Canary Islands

Most interesting Private Journeys Tunisia 1972

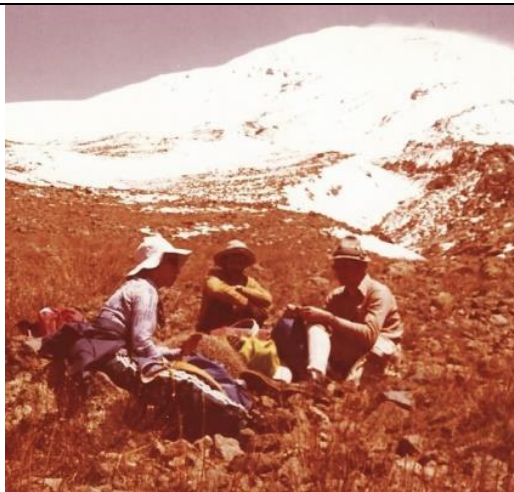
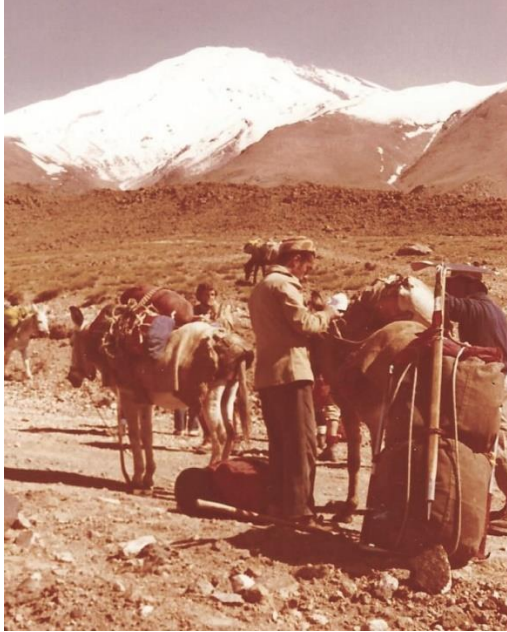


Iran 1975, mountain tour Demavend (4667m) , Teheran, Caspic sea, Isfahan, GOM





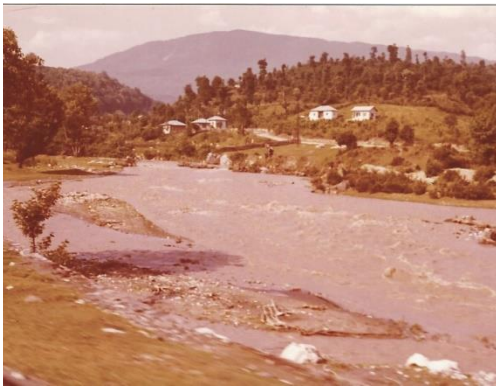
**Before Khomeini, not to forget:
The “Shaw” sent busses to the mountains to get children to school !**



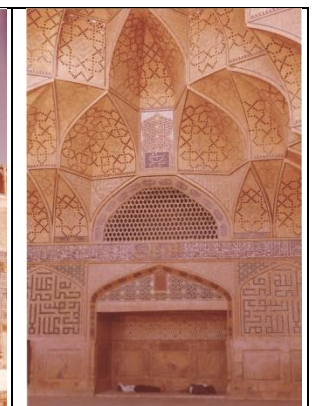
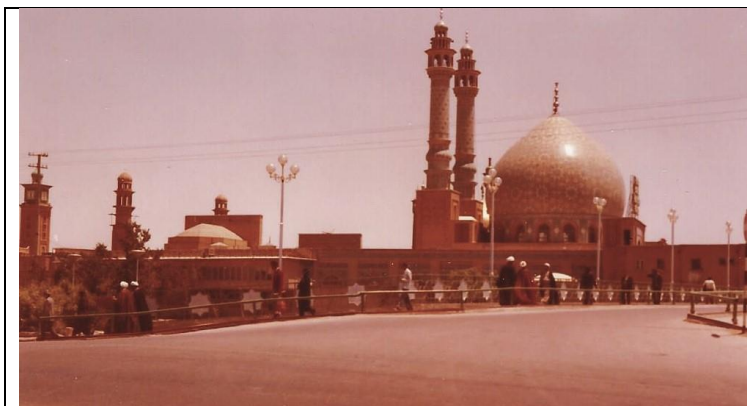
I did not reach the top, LL: The mules could not go through the too high snow, so I also pulled a trekking bag up the mountain with the mountain guide. Even if you are very strong. *Never overexert yourself on the mountain !!*

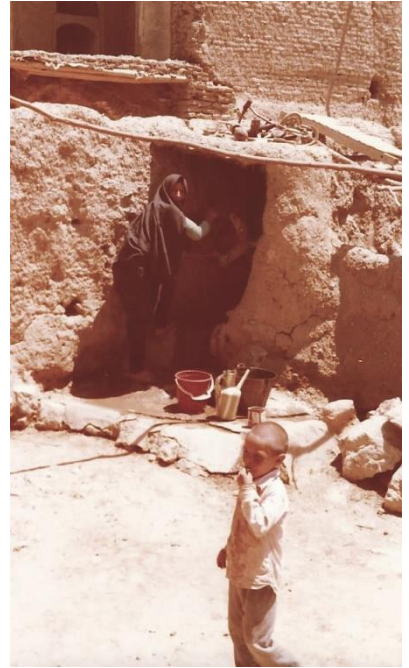


Birthplace of the tulips, red & yellow



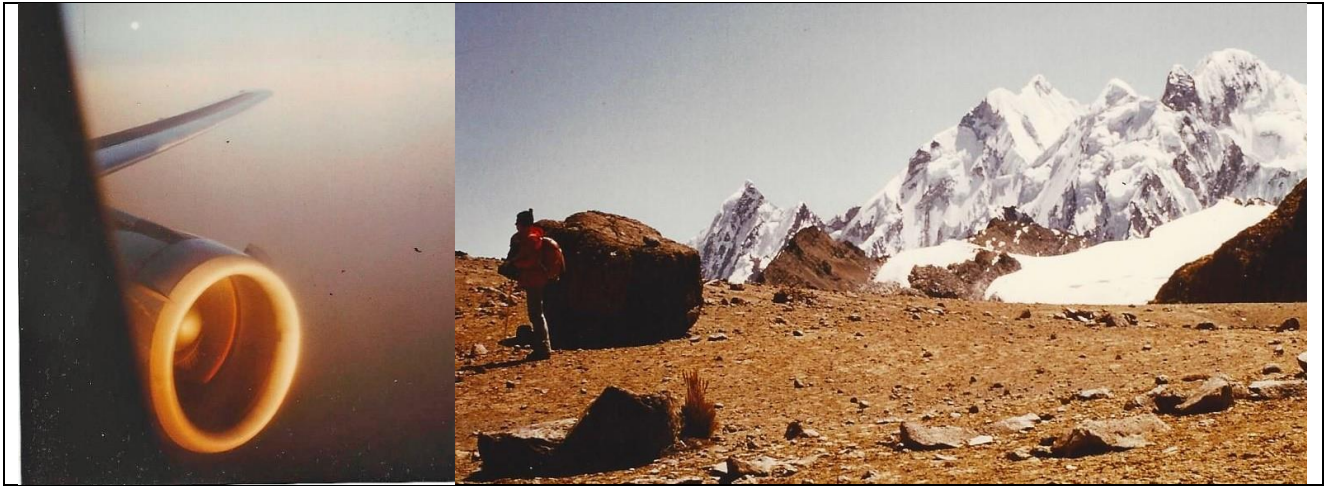
From Caspean Sea back through the mountains and then to Isfahan

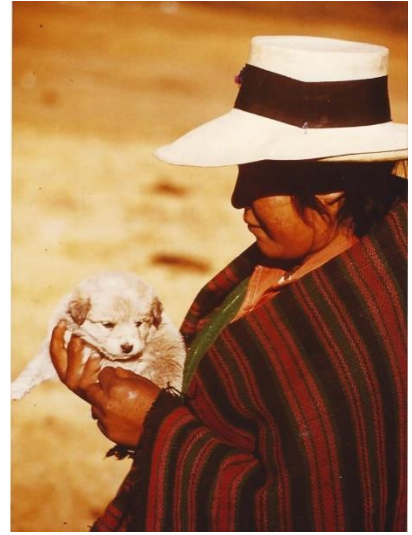
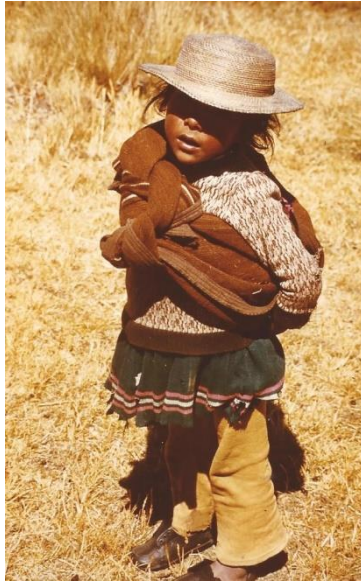
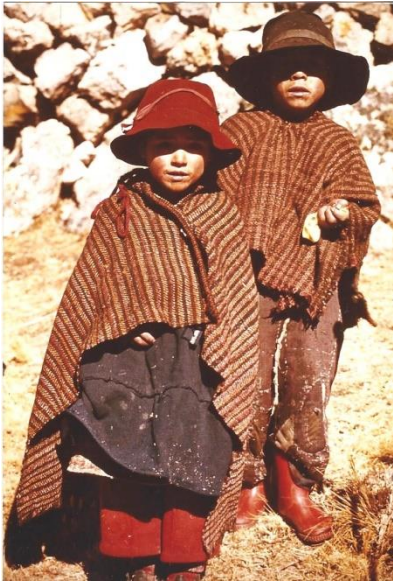




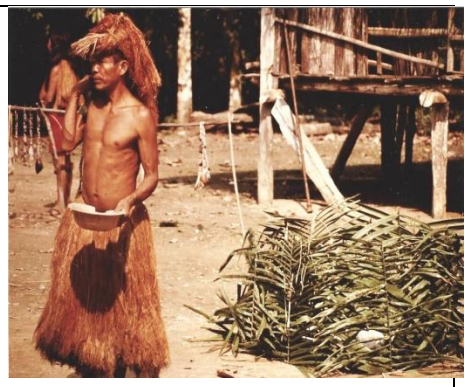
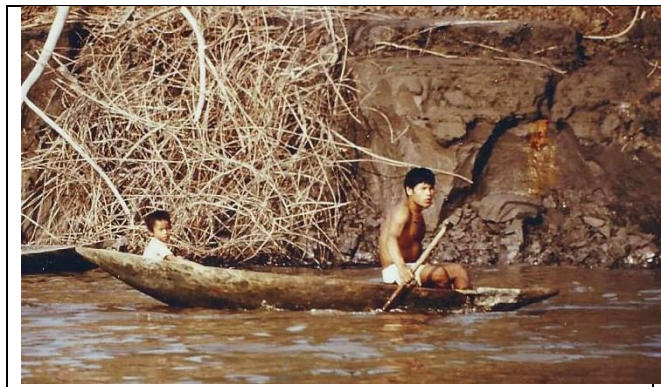
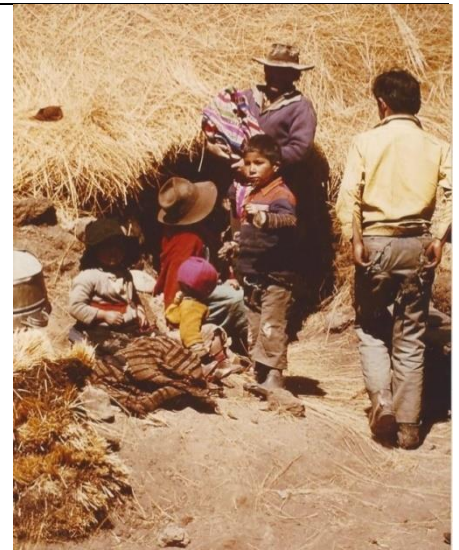
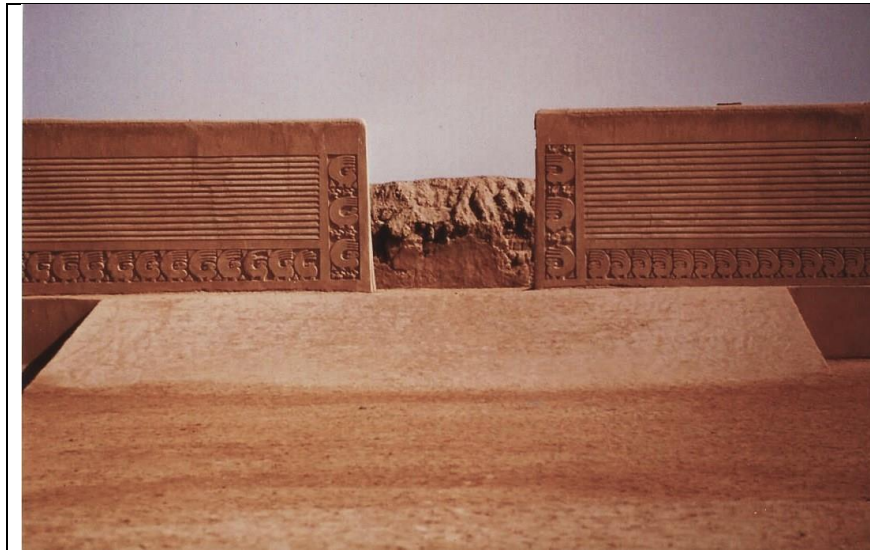
Swinging Minarett

Peru 1979





| | |
|---|---|
| A photograph of a man in a blue and white checkered shirt climbing a tree. He is holding a long wooden pole and looking up at the tree. | <p data-bbox="771 808 1421 850">St. Mauritius: Le Morne, holy mountain</p> A photograph of a large, rocky mountain with a red vine-covered tree in the foreground. The mountain is covered in green vegetation and has a white building at its base. |
|---|---|

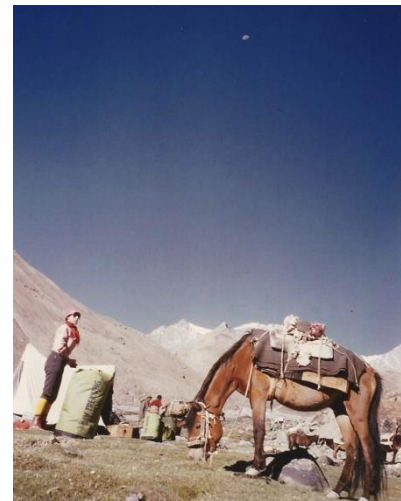
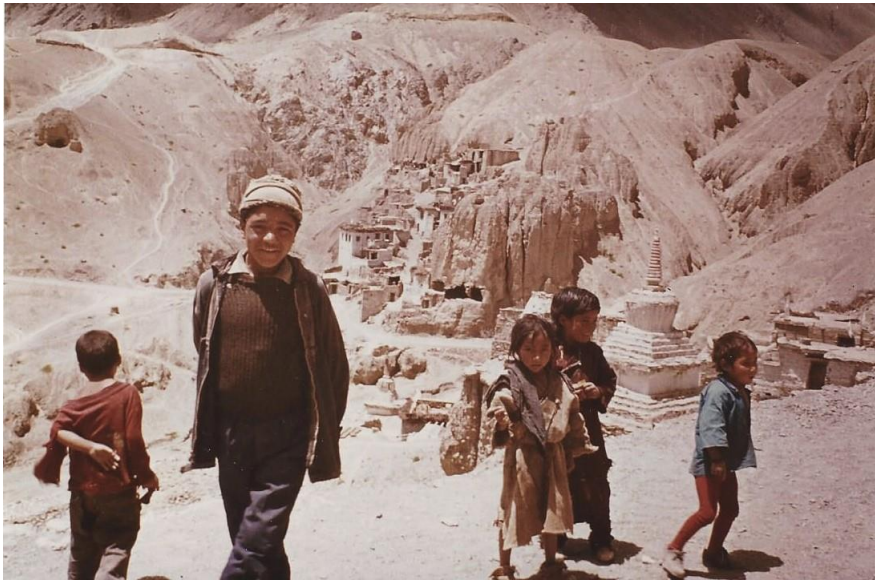


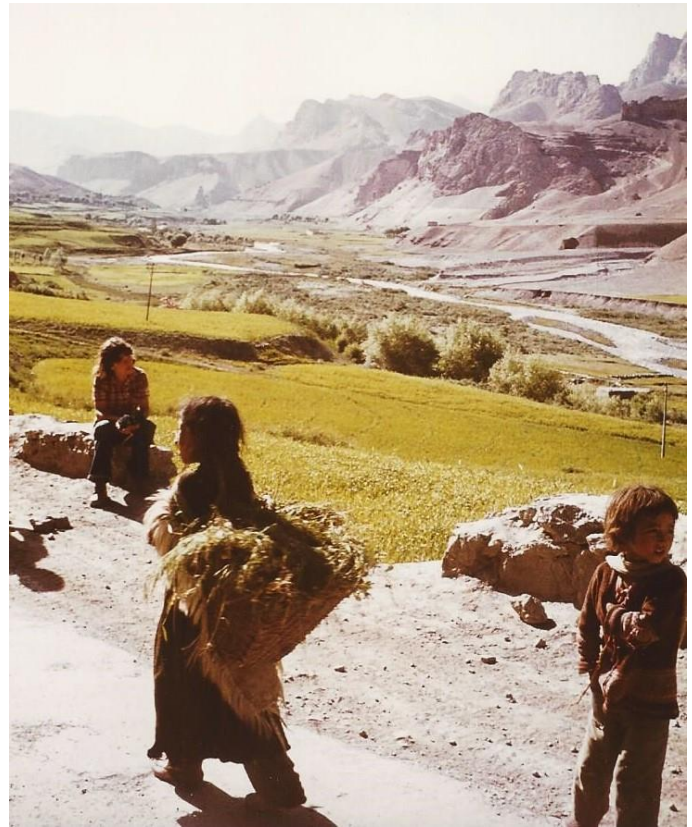
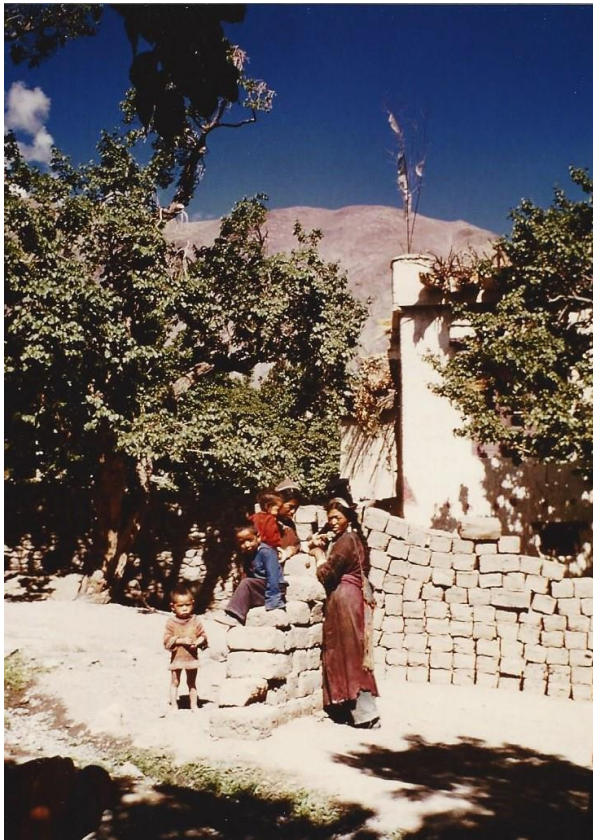
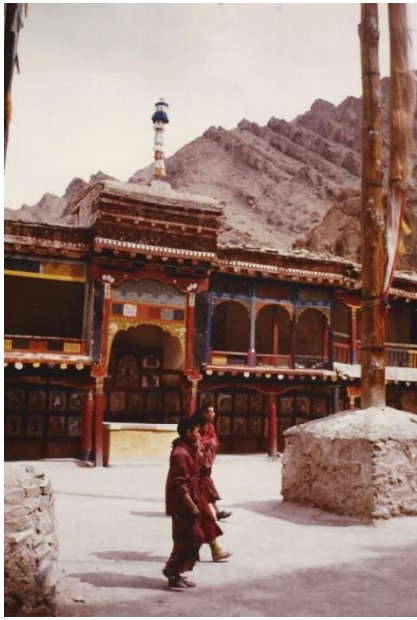
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Ladakh 1981









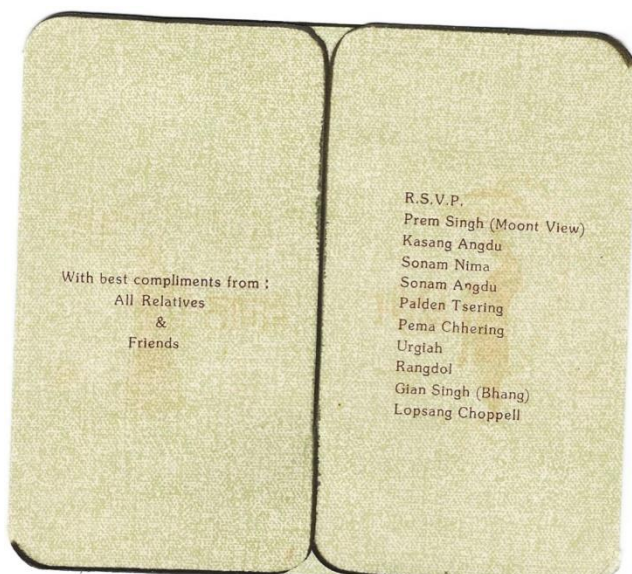
The unimaginable value of 5 cm pasture grass.

As you can see from the photos above, the grass is very short and therefore about 5 cm large pasture grass is extremely valuable for the mountain farmers:

The path led our trekking group into the valley of the neighboring community, which then led to a serious threat situation for us by the mountain farmers living there. Our problem was quickly solved the next day, when a judge was ‘called’, who arrived on

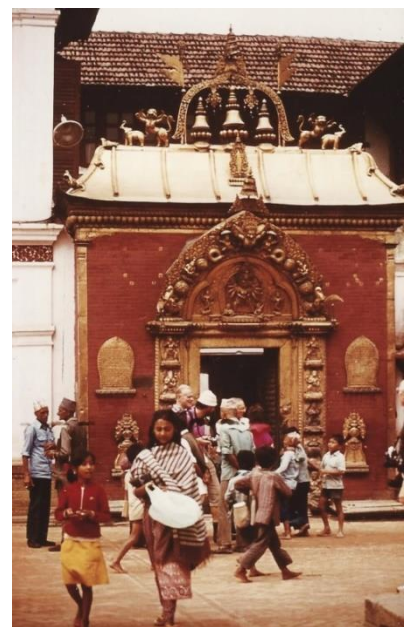
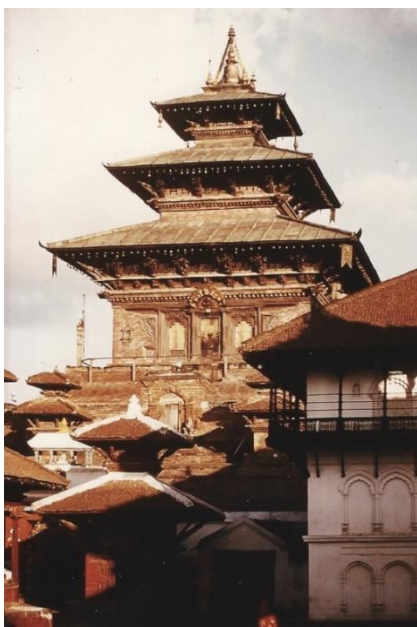
foot - walking under a parasol of the assistant - after many hours of walking. He immediately convicted the leader in a court hearing at an altitude of 4500 m and immediately had him taken to prison. When we descended again a few days later, the prisoner's relatives asked us to help them get him released. A major reason for this was that the relatives had to bring food to the detainee. Living 10 km away and having to take care of it *on foot* was a very big problem for them.

An invitation from a dealer in Leh, with whom I had also talked a lot:



Himalayan Trekking, Nepal 1983

Text





Difficult Himalayan high altitude tour,
31 days.

Destinations: Westcol 6153 m, Naulekh 6363, Mera Peak 6461m. Mera La Pass 5415 m. Base Camp: 4600 m at Baruntse 7220m. Over the Mingbo La Pass, 5817 m, back to Lukhla via Pangpoche, Tengpoche Khumjung



4 days waiting: Getting-up early, airport Kathmandu, back to hotel etc.
After 4 days arrival at Lukhla airport



The Mount Everest Log Book:
Messner, Kammerlander, Dacher etc.

APR 83

| Arrival Date | Name | Nationality | Home Address | Departure |
|--------------|-----------------|-------------|----------------|-------------|
| 6th Apr 83 | R. MESSNER | ITALY | Everest Summit | |
| | H. Kammerlander | ITALY | | |
| | H. Dacher | Germany | | |
| 21th Apr 83 | Carlo Schuster | Austria | | |
| 20th Apr 83 | C. MURRAY | Australia | | |
| 26 Apr 83 | Robert Linnert | Australia | | |
| 2 May 83 | Gregory Plunard | BRITISH | | 27th Apr 83 |
| 3 May 83 | Robbie Muir | BRITISH | | |
| 3-5 May 83 | GIORGIO RIVA | ITALY | | |
| 6th May 83 | Tim Butler | AMERICAN | | |
| | C.R. REENIE | BRITISH | | |
| | M. J. P. M. | | | |
| 11th May 83 | Bob Stewart | | | |

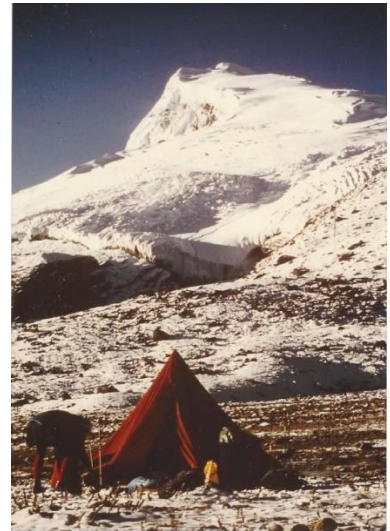
Lukhla airfield at almost 3000m:
Dangerous landing and starting

The airfield is served from Kathmandu fully depending on weather and visibility conditions. The slope of the 527 m long runway is around 12 %, and it can only be landed uphill and taken off downhill. The end of the runway then abruptly breaks off about 600 m deep to the Dudh Kosi River. *After landing, we noticed the good fencing of the former airfield garden. As we marched past: The fence was constructed from over 280 propeller blades.*



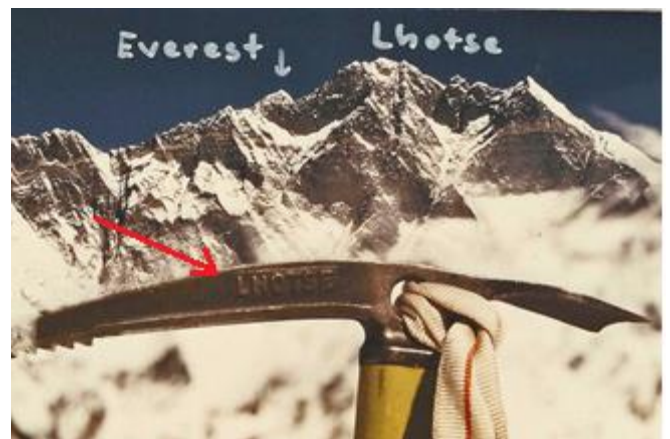
Mountain experience

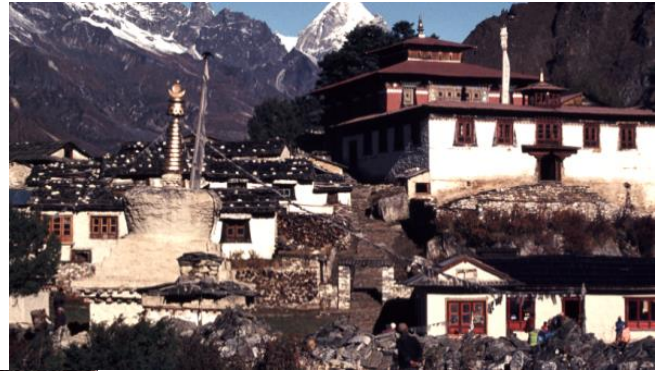
After waking up in the morning:
opening the 4 zippers inner
sleeping bag, outer sleeping bag,
inner tent, outer tent. Great view:
Half a meter of fresh snow.
Ascent of Mera La too strenuous!



left:

A yak manure place keeps warm when having breakfast.





left: Who will stop ???

below: the beautiful Ama Dablam



At the end, gifts are distributed to the capable trekking helpers





Abseiling from the Mingbo La pass, 5800 m, impressive life event:

Arrival 9 a.m. Abseiling together with a fearful sherpani (with rubber boots in the ice!) to take over the intermediate station at 10 o'clock.

Then for 8 hours let down the trekking participants, equipment and Sherpas descending in the ice channel secured to the rope and then pull this heavy rope up again and again. From 4 p.m. sunlight gone and freezing cold. I had the to go down the ice channel with the last person, our cook – tied up in front of me. When we were downstairs, he untied himself and then disappeared downstairs with a last cry in the dark.

Now my exhausted colleague from the high station slowly came down with his headlamp. I gave him my ice pick, because I still had an ice axe and then we crossed the steep slope extremely carefully.

After about an hour, someone came towards us. It was the guide who told us that he had met the cook on the ascent. Great joy.

For me, the only thing left was to drink, wrap myself in the two sleeping bags and sleep on a mat between boulders.

*Notes: *Sherpas can carry twice as much of us at great height, but could not help pull the heavy rope. *All the pictures my colleague took of this event from above were developed incorrectly. Grief*

Alaska 1986

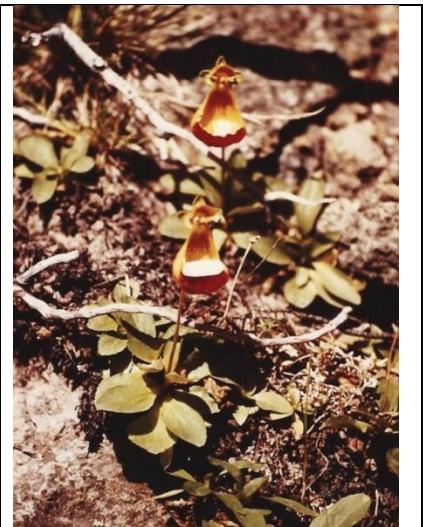
Alaska trips, later





Argentina, Patagonia, 1987)

Fitz Roy hiking, Patagonia,



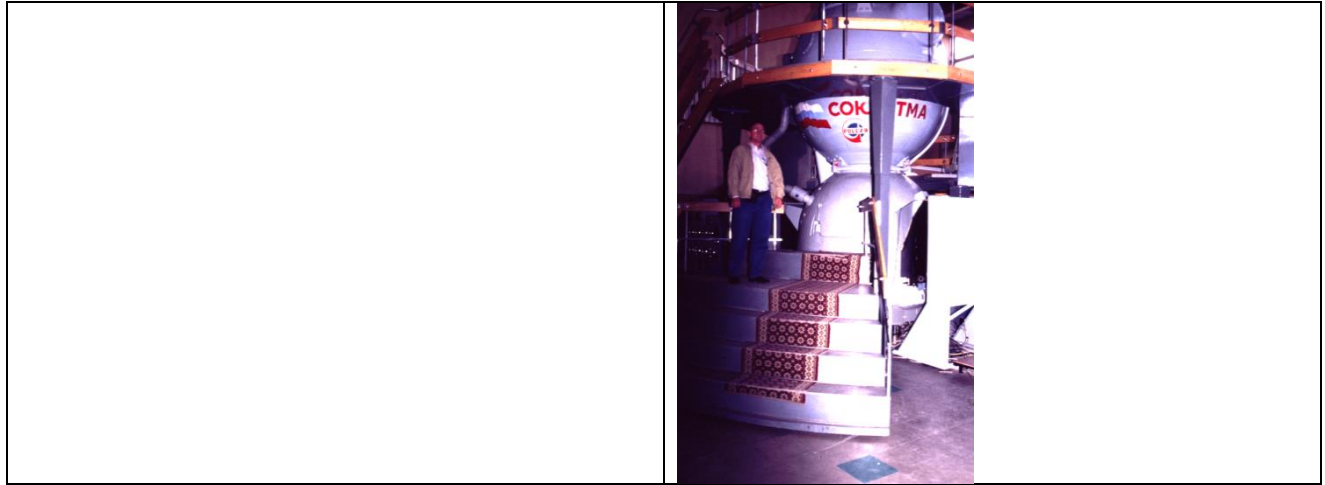


Lago Argentino





Bolivia, Chile, 19??

Moscow with Star City



Myanmar, Thailand, Laos, Cambodia, Saigon 199??

Australia 1997

| | | | |
|---|--|--|--|
|  | |  | |
| | | | |

Some Private Trips

Tunesia 1972

| | | |
|--|--|--|
| | | |
|--|--|--|

New Zealand 1997

Crossing China from Kazakhstan 2005









Chinese family on the Great Wall, 2000

Syria, Lebanon, Jordan 2005??



Usbekistan 2007

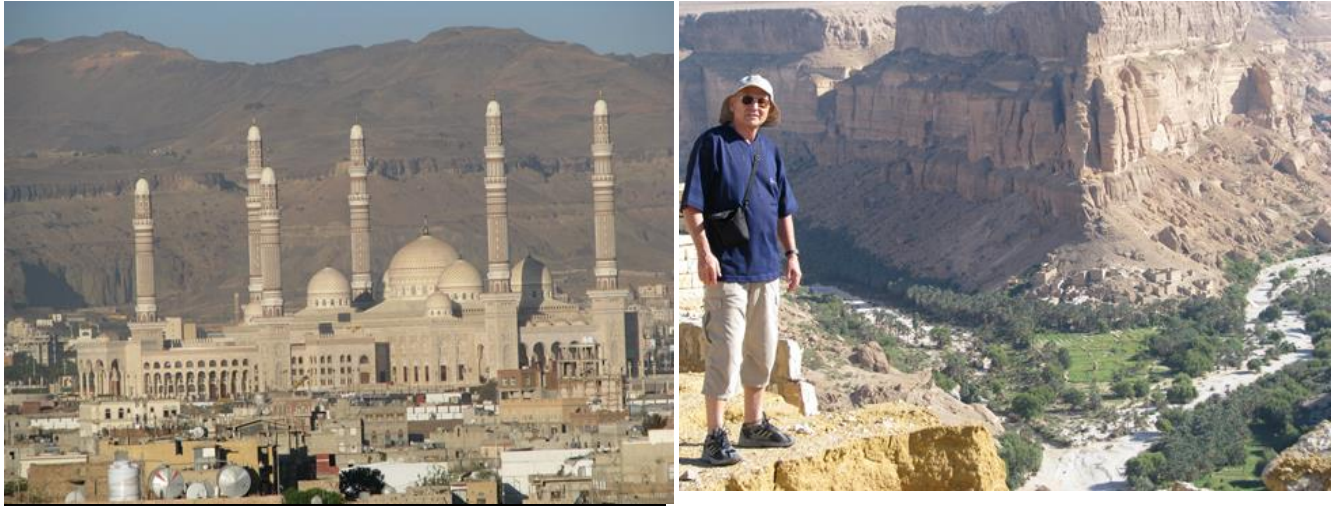


Chiwa, Usbekistan: Invited into a home



Middle America 2008

Yemen, 2009



Beautiful Yemen, Sanaa and ????



Yemen: visiting a family

Ethiopia



Mountain luggage

Ras Dashen, East Africa, 4550 m

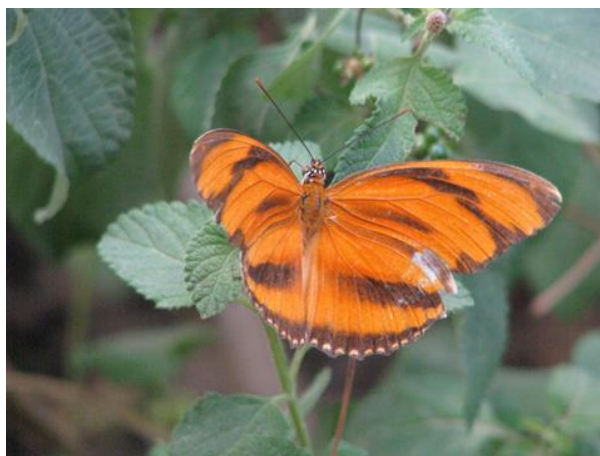
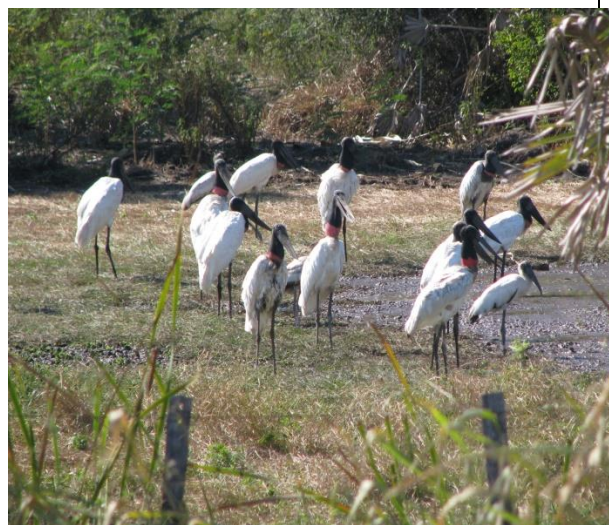


Cut out of the mountain rock, Aethiopia



Lilibela

Paraguay, 2009

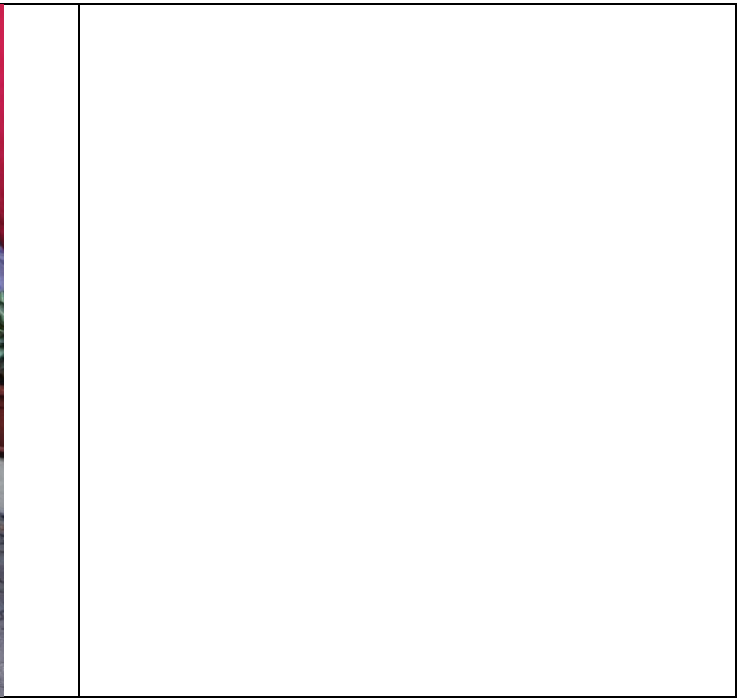




Indonesia



Feeding an Oran-Utan in Indonesia



Largest moth in the world

Smallest ape, Makaki



Crossing Africa 2011



Kikuyu ladies in South Africa



Leopard, elephant

Crossing India , 2012

Bolivia,

Philippines,



Pinatubo, Renatured after about 15 years, nature repairs

Columbia





Friend or lazy beneficiary



hyacinth,

Papua-Newguinee,



Wig maker in Eastern Irian

North and South Korea



*Grown-up at the German nation separating border, now visiting the internal Korean border.
Room of Space peace treaty with the USA. Looking to South Korea*

Bhutan, 2012

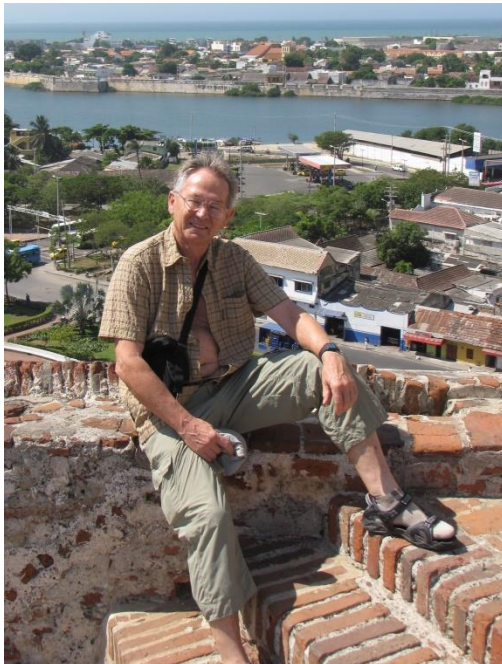
Venezuela , 2013



*Venezuela
Me and the guide*

Venezuela

**“Challenging
tasks
strengthen!”**



Marocco, 2013

Scandinavia,

Namibia 20



Island, 2020

Oman , 2023

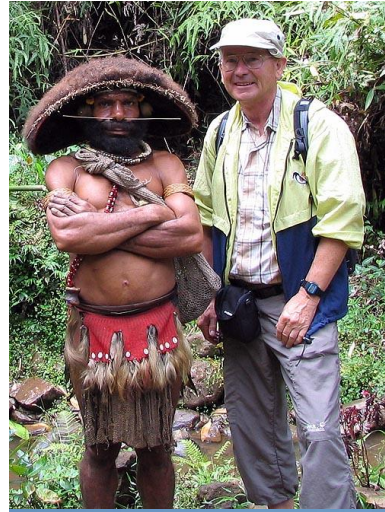


Pictures from at home



Some special Photographies

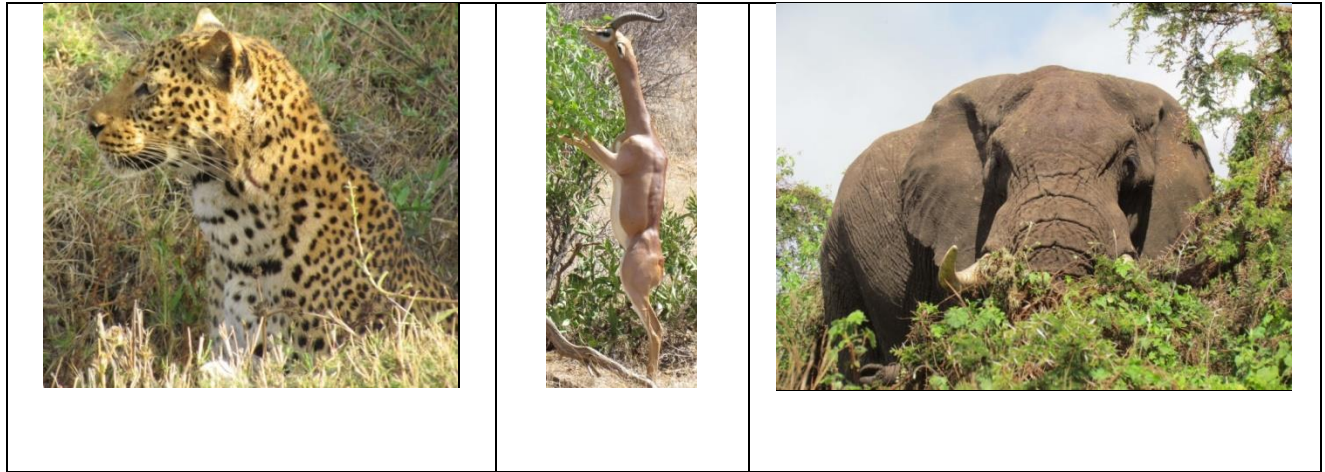


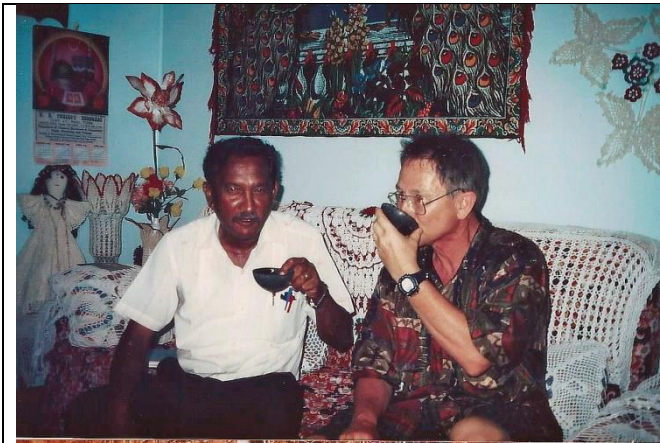




Tarsier, smallest ape





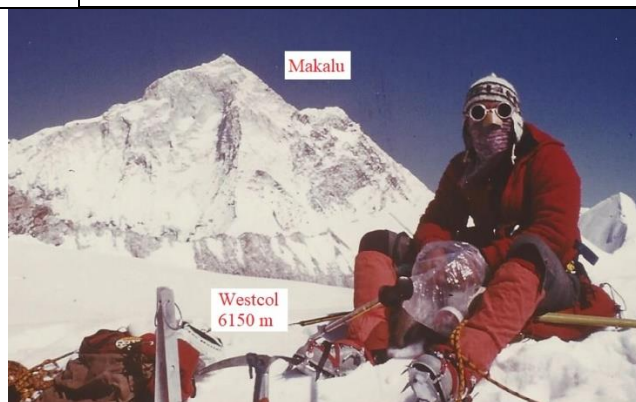


At Fiji:

In the house of a retired Hindhu
sugar engineer

The world is small.

*The look is like that one I
remember which I had when I
walked with my Grandma for
spinning and crocheting in
1944.*



Above:

Since the porters with the equipment were missing,
we had to take spare clothes.

Consequence: Take what you have.

In this context for the scientific part readers:

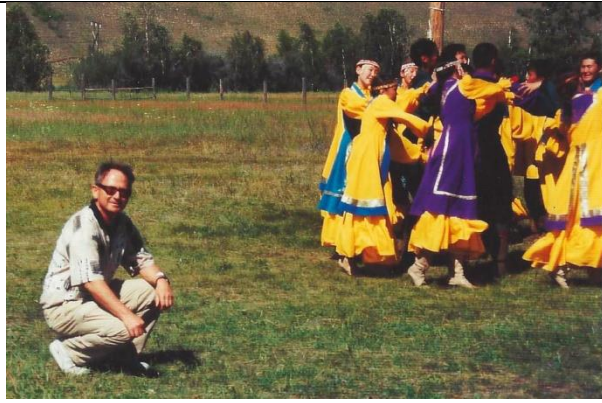
In the WWFE, the author also had no FE program
available. But, much is possible if a goal is pursued
and this was also the case with having some
fruitful ideas when investigating some special
WWFE-II test cases.

Lessons Learned (LL) from Mountaineering



Pinatubo, renatured after about 15 years.

Nature repairs



Albtraum einer Brückenüberfahrt

Cuntze: Sikkim, 2012



The caiman mother Maria observes limit the “No trespassing (No pase!)”. That was very good for the personal health of my friend Eddi (he unfortunately fell in front of her snout while running away). We learn: Structural engineers should always observe the limits set by the structural specifications.(2013)



